

# Stability of river bed forms cont'd

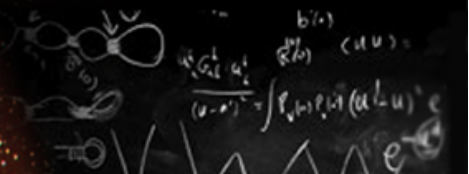
**Marco Colombini**

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**University of Genova, Italy**



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University of California, Santa Barbara



# Ripples and Dunes



Ripples in the Hunter River, New South Wales, Australia

Image courtesy of M.C. Rygel



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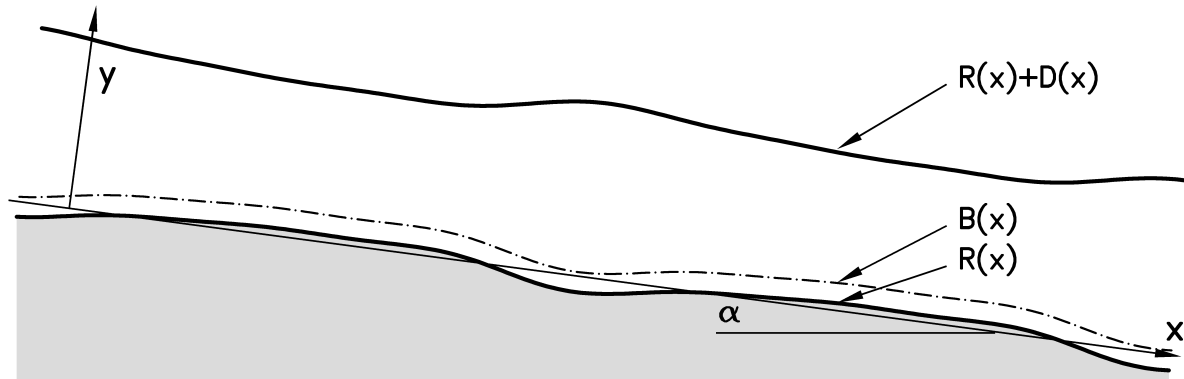
# Linear Stability Analyses

Authors	Flow Regime	Flow model	Results
Richards 1980 <i>J. Fluid Mech.</i>	rough	Rotational Effect of the flow depth included	Dunes Ripples
Engelund & Fredsoe, 1982, <i>Ann. Rev. Fluid Mech.</i>	Ripples associated to smooth flow regime		
Sumer & Bakioglu, 1984 <i>J. Fluid Mech.</i>	smooth	Rotational Boundary layer Infinite flow depth	Ripples
Fourriere et al, 2010 <i>J. Fluid Mech.</i>	rough	Rotational Effect of flow depth included	Ripples
Colombini & Stocchino, 2011, <i>J. Fluid Mech.</i>	rough transitional smooth	Rotational Effect of flow depth included	Dunes Ripples

**All models consider bed load only**

Previous Studies





$$\eta = \frac{y - R}{D}$$

## 2D REYNOLDS EQUATIONS + CONTINUITY

$$u_{,t} + uu_{,x} + vv_{,y} + p_{,x} - S / Fr^2 - T_{xx, x} - T_{xy, y} = 0$$

$$v_{,t} + uv_{,x} + vv_{,y} + p_{,y} + 1 / Fr^2 - T_{xy, x} - T_{yy, y} = 0$$

$$u_{,x} + v_{,y} = 0$$

## BOUSSINESQ's TURBULENCE CLOSURE

$$T_{xx} = 2\nu_T u_{,x} \quad T_{yy} = 2\nu_T v_{,y} \quad T_{xy} = \nu_T (u_{,y} + v_{,x})$$

## EDDY VISCOSITY

$$\nu_T = l^2 \sqrt{(u_{,x} - v_{,y})^2 + (u_{,y} + v_{,x})^2}$$





## CONDUCTANCE COEFFICIENT (CHEZY)

$$C = \sqrt{\frac{8}{f}} = \frac{1}{\kappa} \ln\left(\frac{11.09}{2.5d}\right)$$

**Rough regime**

(ASCE Task Committee, 1963)

$$C = \sqrt{\frac{8}{f}} = \frac{1}{\kappa} \ln\left(\frac{\text{Re}\sqrt{f}}{3.41}\right)$$

**Smooth regime**



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$$C = \sqrt{\frac{8}{f}} = \frac{1}{\kappa} \ln\left[\left(\frac{11.09}{2.5d}\right)^{1-\beta} \left(\frac{\text{Re}\sqrt{f}}{3.41}\right)^\beta\right]$$

**Transitional regime**

(Chang, 2008)

$$\beta = \exp\left[-0.11(\ln \text{Re}_k)^{\frac{5}{2}}\right]$$

$$\text{Re}_k = \frac{u_f^* k_S^*}{\nu}$$

$$\text{Re} = \frac{4U^* D^*}{\nu}$$



$$\text{Re} = \frac{4C}{2.5d} \text{Re}_k$$

**Roughness**

**Reynolds number**

**Flow**

**Reynolds number**



## MIXING LENGTH – Rough regime

$$l = \kappa D (\eta + \eta_0) (1 - \eta)^{1/2}$$

**Rule of the thumb:** the roughness height is about one thirtieth of the actual nondimensional roughness.

This result can be formally obtained by integrating the log-law along depth, relating the roughness height to the nondimensional Chezy coefficient)

$$1 = \int_0^1 u d\eta = \frac{1}{\kappa C} \int_0^1 \ln\left(\frac{\eta + \eta_0}{\eta_0}\right) d\eta \cong -\frac{1}{\kappa C} [1 + \ln(\eta_0)]$$

and then substituting the empirical relationship

$$C = \sqrt{\frac{8}{f}} = \frac{1}{\kappa} \ln\left(\frac{11.09}{2.5d}\right)$$

to obtain

$$\eta_0 = \exp(-\kappa C - 1) = \frac{d}{12} = \frac{2.5d}{30} = \frac{y_R}{30} = R_0$$



## MIXING LENGTH – Smooth & Transitional regimes

$$l = \kappa D (\eta + \Delta \eta) \left[ 1 - \exp \left( - \frac{\eta + \Delta \eta}{A} D \frac{\text{Re}_k}{2.5d} \right) \right] (1 - \eta)^{1/2}$$



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**Rotta (1962)** – The velocity profiles for rough and smooth regimes can be similar provided a suitable displacement is introduced.





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**Van Driest (1956)** – The exponential correction accounts for the existence of a viscous and buffer layer in the smooth and transitional regimes.

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$$\Delta\eta^+ = \frac{\text{Re}_k}{30}$$

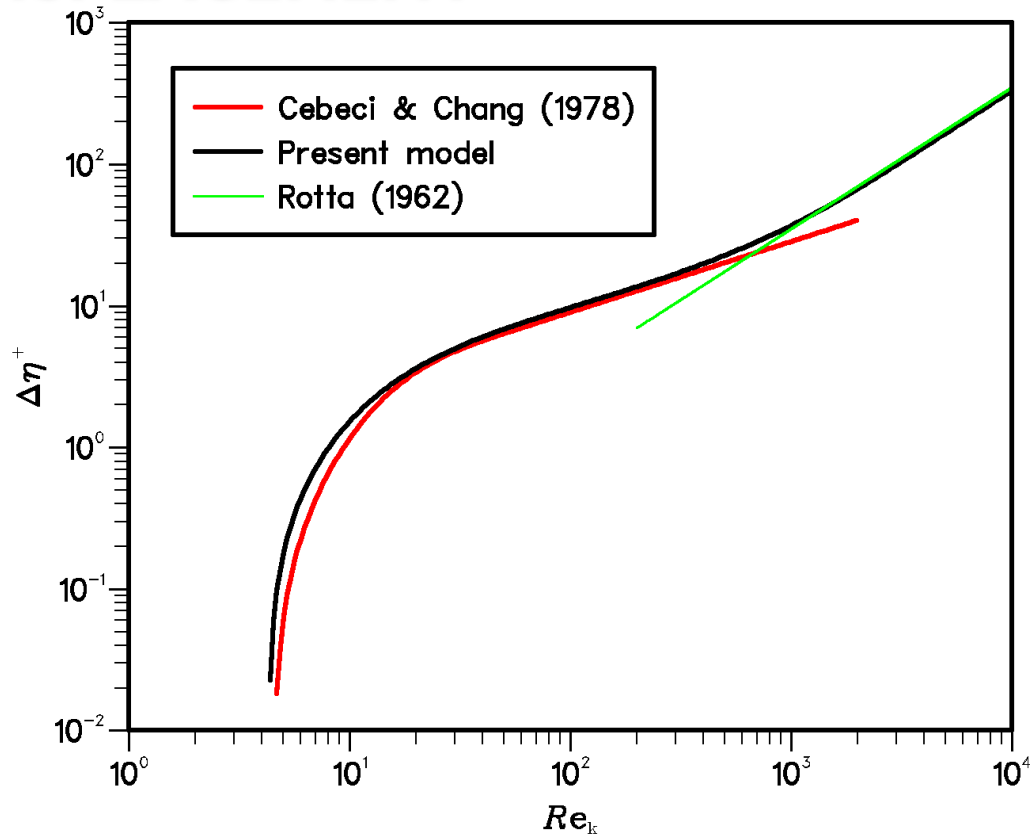
**Rotta (1962)**  
Rough regime

$$\Delta\eta^+ = 0.9 \left[ \sqrt{\text{Re}_k} - \text{Re}_k \exp\left(-\frac{\text{Re}_k}{6}\right) \right]$$

**Cebeci & Chang (1978)**  
Smooth & Transitional regime



## DISPLACEMENT

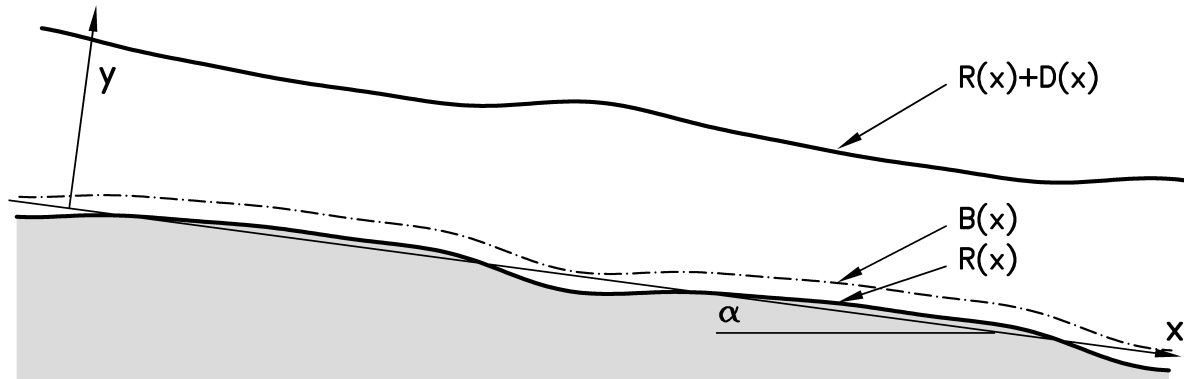


$$C = \frac{1}{\kappa} \ln \left[ \left( \frac{11.09}{2.5d} \right)^{1-\beta} \left( \frac{\text{Re} \sqrt{f}}{3.41} \right)^\beta \right]$$

$$\beta = \exp \left[ -0.11 (\ln \text{Re}_k)^{\frac{5}{2}} \right]$$



$$\Delta \eta^+ = f(\text{Re}_k)$$



### SEDIMENT CONTINUITY EQUATION (EXNER)

$$R_t + Q_S \Phi_{,x} = 0 \quad Q_S = \frac{1}{F} \frac{\sqrt{(s-1)d^3}}{1-p} \ll 1$$

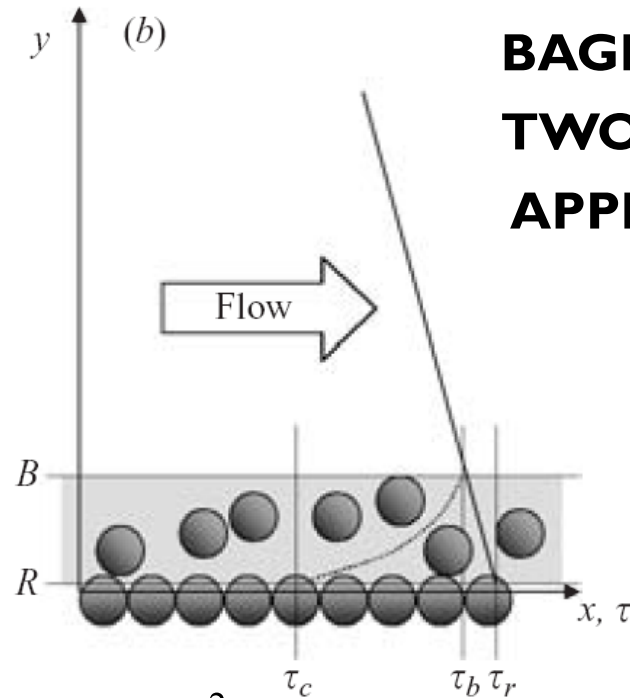
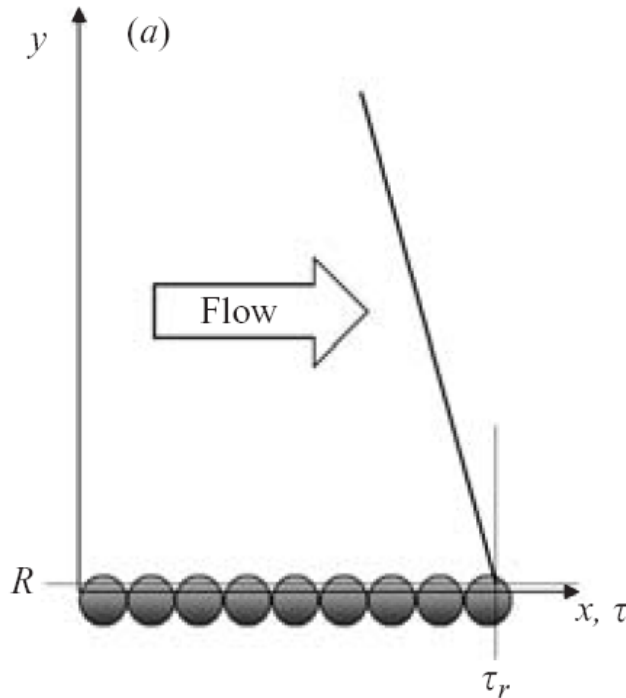
### BEDLOAD FUNCTION (MPM after Wong & Parker (2006))

$$\Phi = 3.97 (\vartheta_B - \vartheta_C)^{3/2}$$

### BEDLOAD LAYER THICKNESS (scaled with grain size)

$$l_B = 1 + 1.3 \left( \frac{\vartheta_0 - \vartheta_{c0}}{\vartheta_{c0}} \right)^{0.55}$$





**BAGNOLD'S  
TWO-LAYER  
APPROACH**

$$\vartheta_B = \vartheta_0 T_B = T_{xy} \Big|_{y=B}$$

$$\vartheta_0 = \frac{F^2}{C^2 (s-1) d}$$

$$B = R + \eta_B D$$

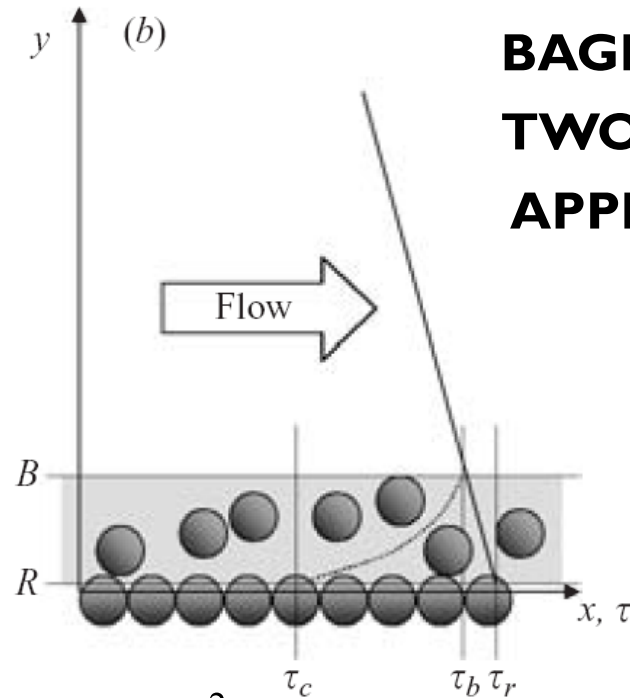
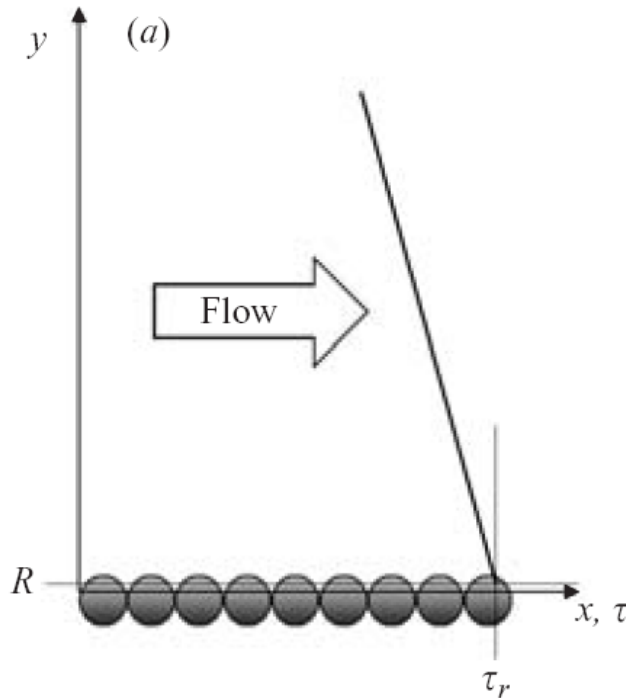
$$\eta_B = l_B d$$

**CORRECTION FOR SEDIMENT WEIGHT**

$$\vartheta_C = \vartheta_{CH} - \mu_x (S - R_{,x})$$

$$\mu_x = 0.1$$

**Fredsøe (1974)**



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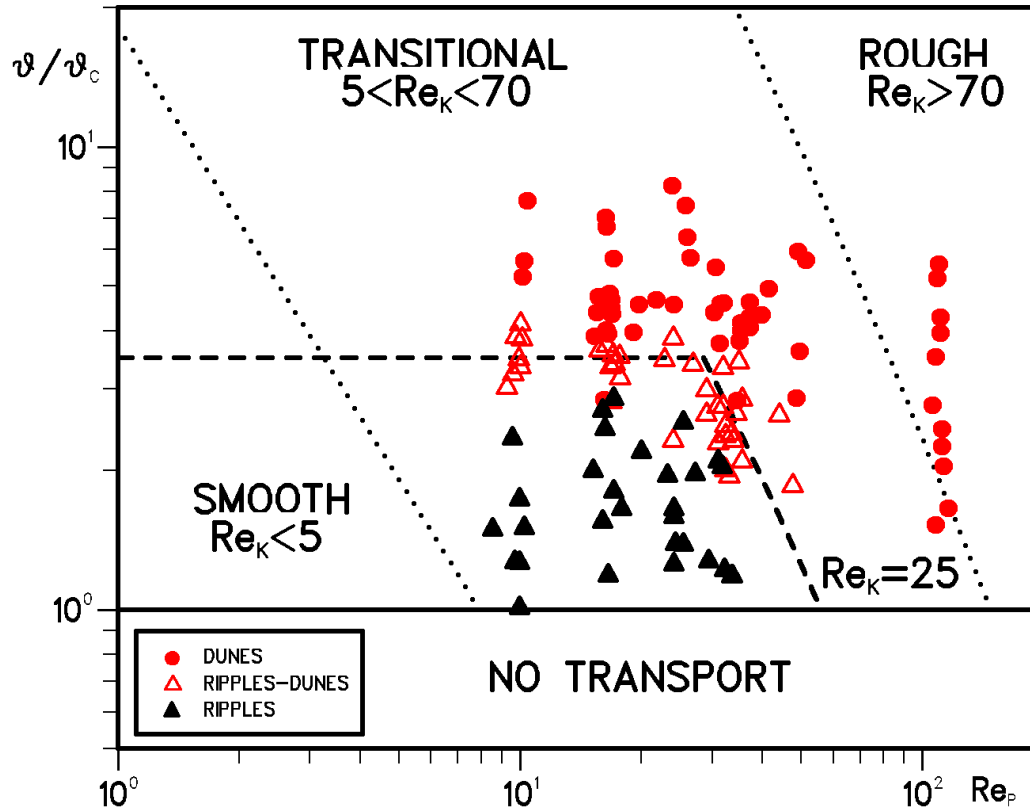
$$\vartheta_{CH} = 0.22 \text{Re}_p^{-0.6} + 0.06 \exp(-17.73 \text{Re}_p^{-0.6}) \quad \text{Brownlie (1981)}$$

$$\vartheta_{CH} = 0.0495$$

$$\text{Wong \& Parker (2006)}$$



## Experiments by Guy, Simons & Richardson (1966)



$$Re_k = \frac{u_f^* k_s^*}{\nu}$$

**Roughness  
Reynolds number**

$$Re_p = \frac{\sqrt{(s-1)gd^*} d^*}{\nu}$$

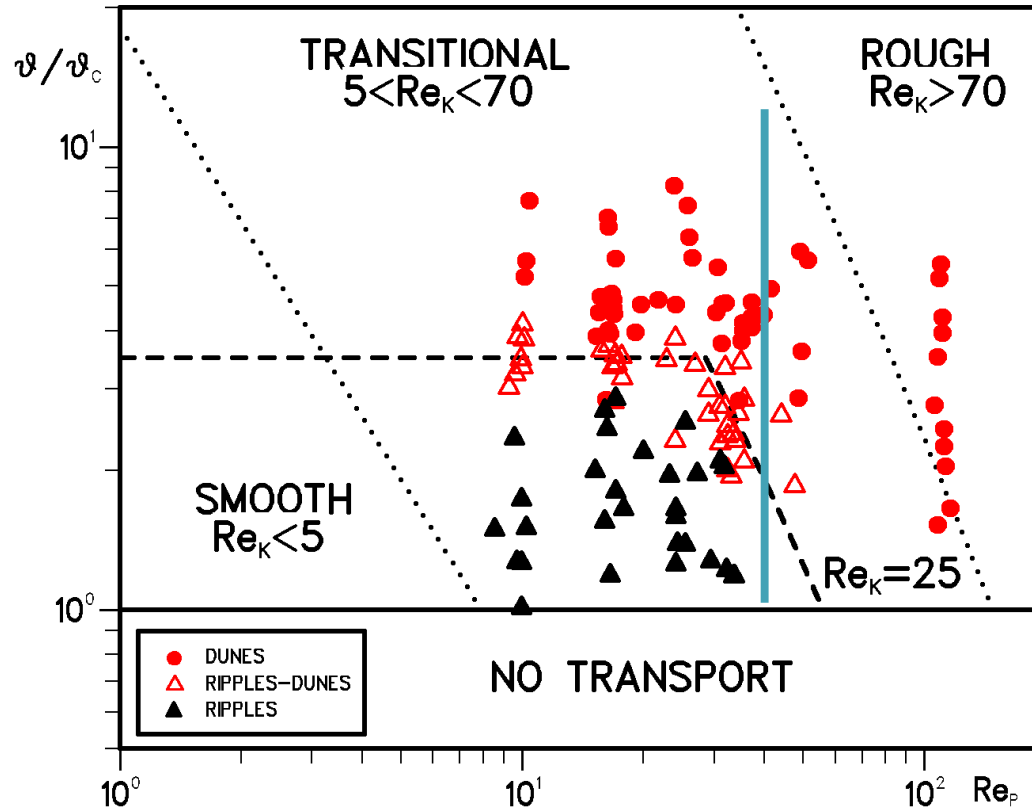
**Particle Reynolds  
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$$Re_k = \sqrt{v} Re_p^{2.5}$$



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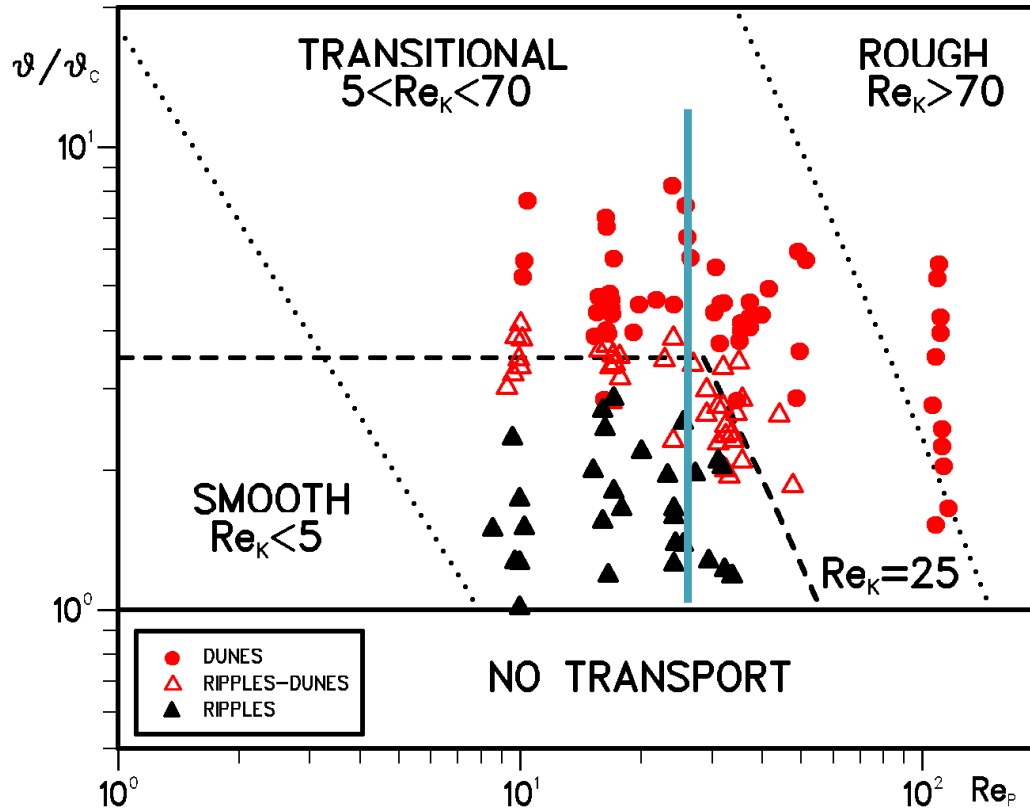


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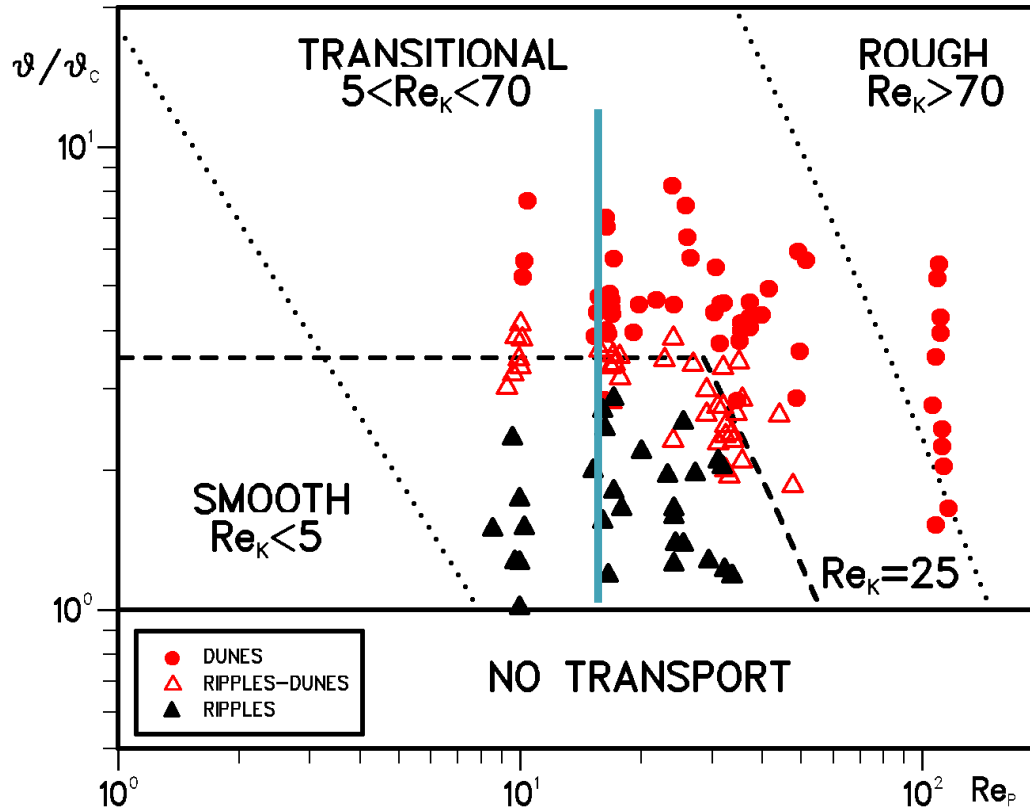
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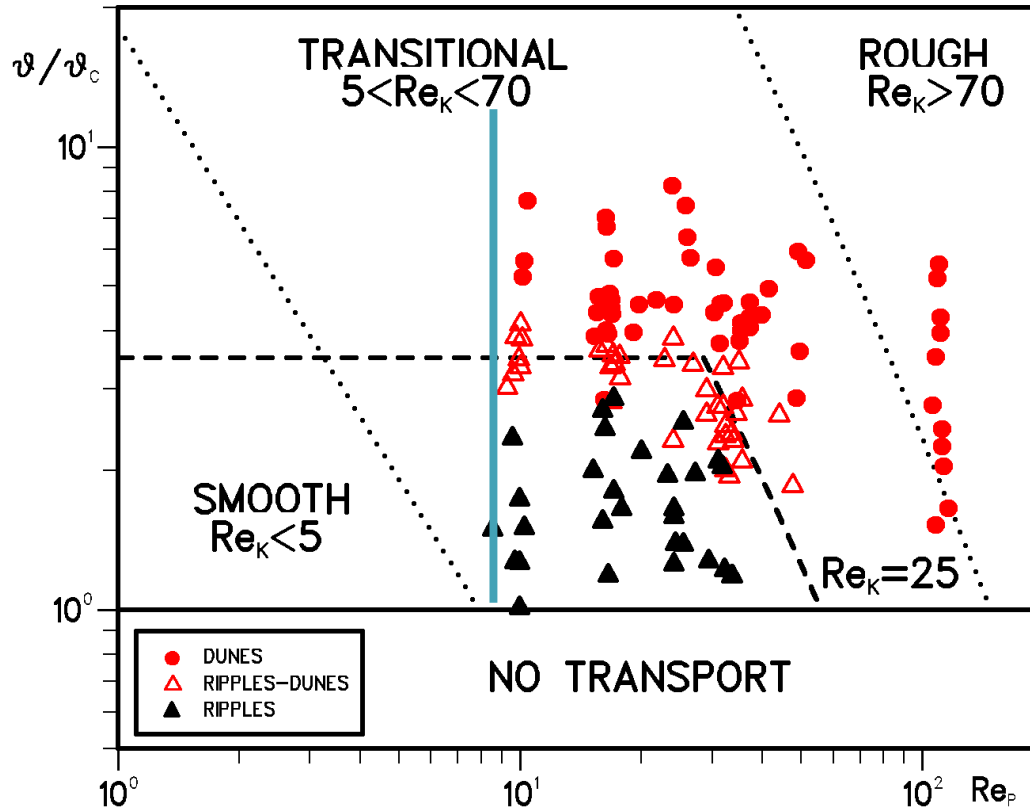
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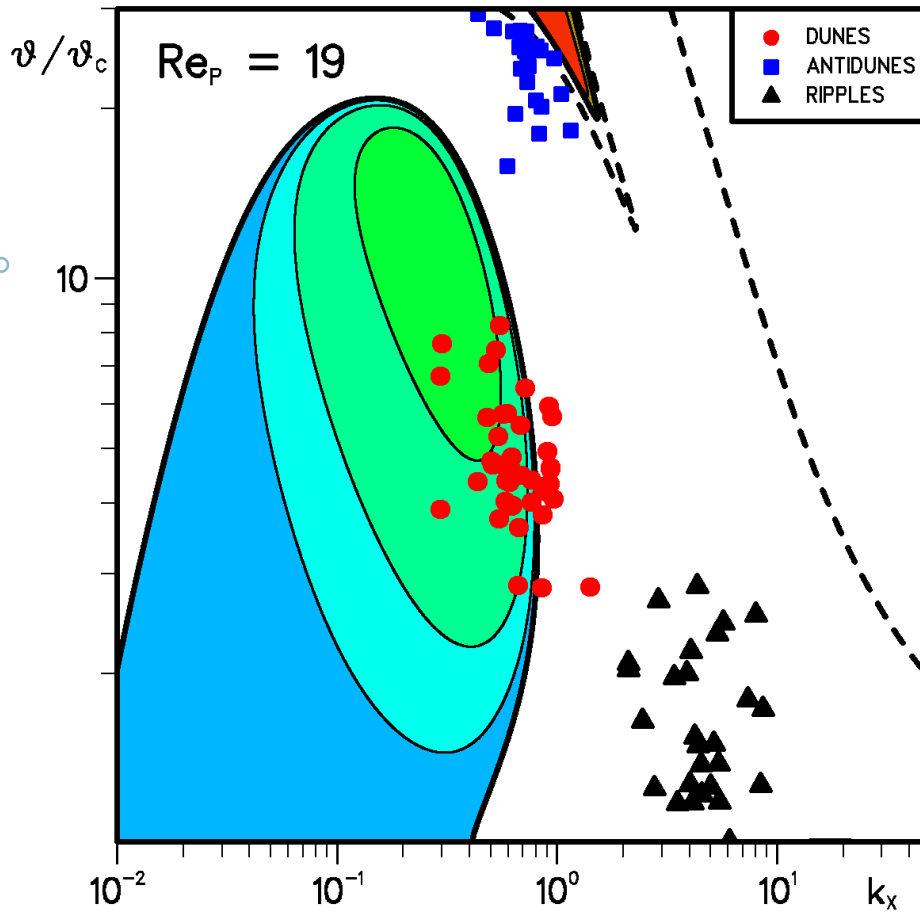
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# Dune-ripple transition



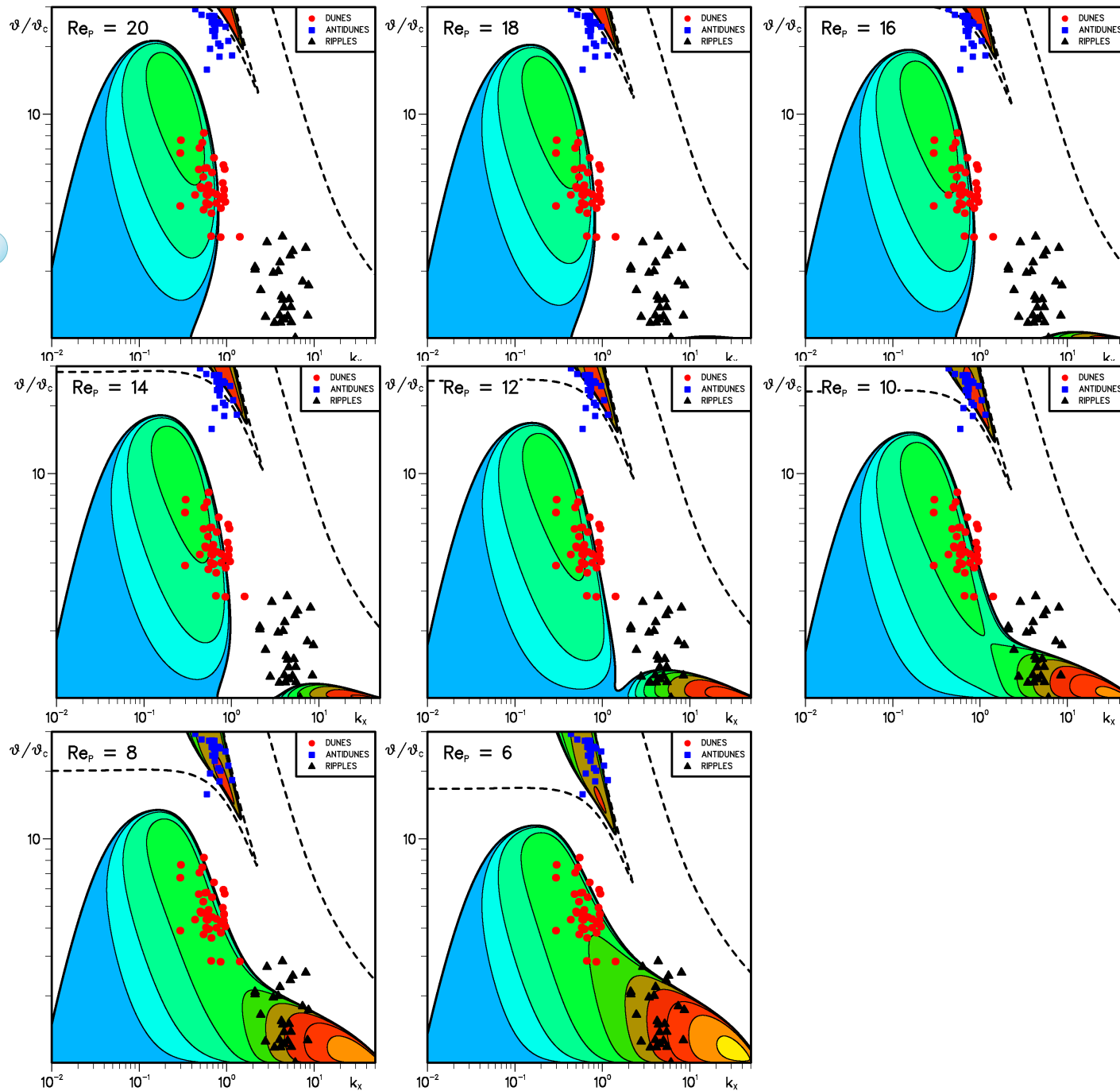
$$Re_p = \frac{\sqrt{(s-1)gd^* d^*}}{\nu}$$

$Re_p$  is the parameter that controls the transition between ripples and dunes

$Re_p$  decreasing  $\Rightarrow d^*$  decreasing  $\Rightarrow$  finer sediment

$d$  constant,  $d^*$  decreasing  $\Rightarrow$  shallower flow

# Dune-ripple transition





- Kennedy (1963,1969) - Irrotational D A R
- Engelund (1970), Fredsøe (1974) - Slip velocity D A R
- Parker (1975) - Irrotational D A R
  
- Richards (1980) - Rotational D A R
- Sumer & Backioglu (1984) - Rotational D A R
- Coleman & Fenton (2000) - Irrotational D A R
- Colombini (2004) - Rotational D A R
- Fourrière, Claudin & Andreotti (2010) - Rotational D A R
- Colombini & Stocchino (2011) - Rotational D A R





- **Kennedy (1963,1969) - Irrotational** **D A R**
- **Engelund (1970), Fredsøe (1974) - Slip velocity** **D A R**
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- **Fourrière, Claudin & Andreotti (2010) - Rotational** **D A R**
- **Colombini & Stocchino (2011) - Rotational** **D A R**
  
- **Charru, Andreotti & Claudin (2013) - Sand ripples and dunes. Ann. Rev. Fluid Mech.** **D A R**
- **Andreotti & Claudin (2013) - Aeolian and subaqueous bedforms in shear flows. Phil. Trans. R. Soc. A** **D A R**

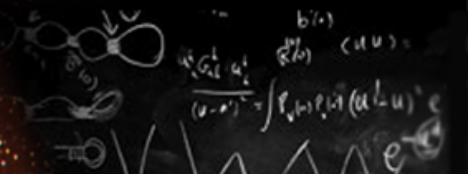
# Linear Coarsening

**Marco Colombini**  
on behalf of

**L. Ridolfi, C. Camporeale, R. Vesipa**  
Dipartimento di Ingegneria dell'Ambiente, del Territorio  
e delle Infrastrutture  
Politecnico di Torino, Italy



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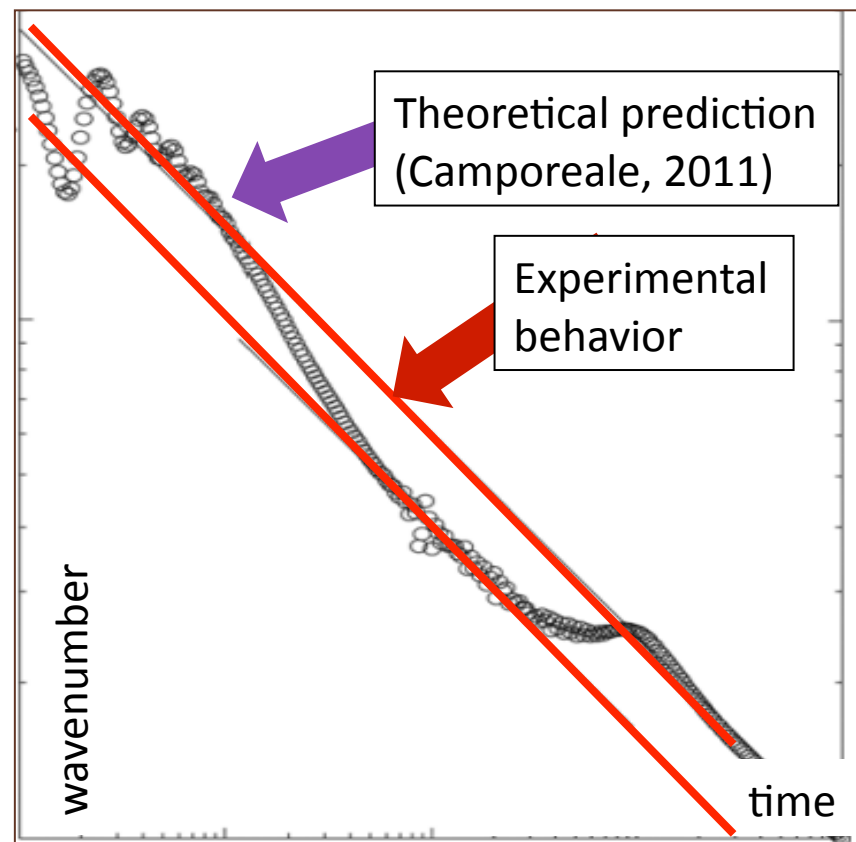
# Nonmodal analysis and river morphodynamics

**Nonmodal analysis has been proposed for different morphological systems:**

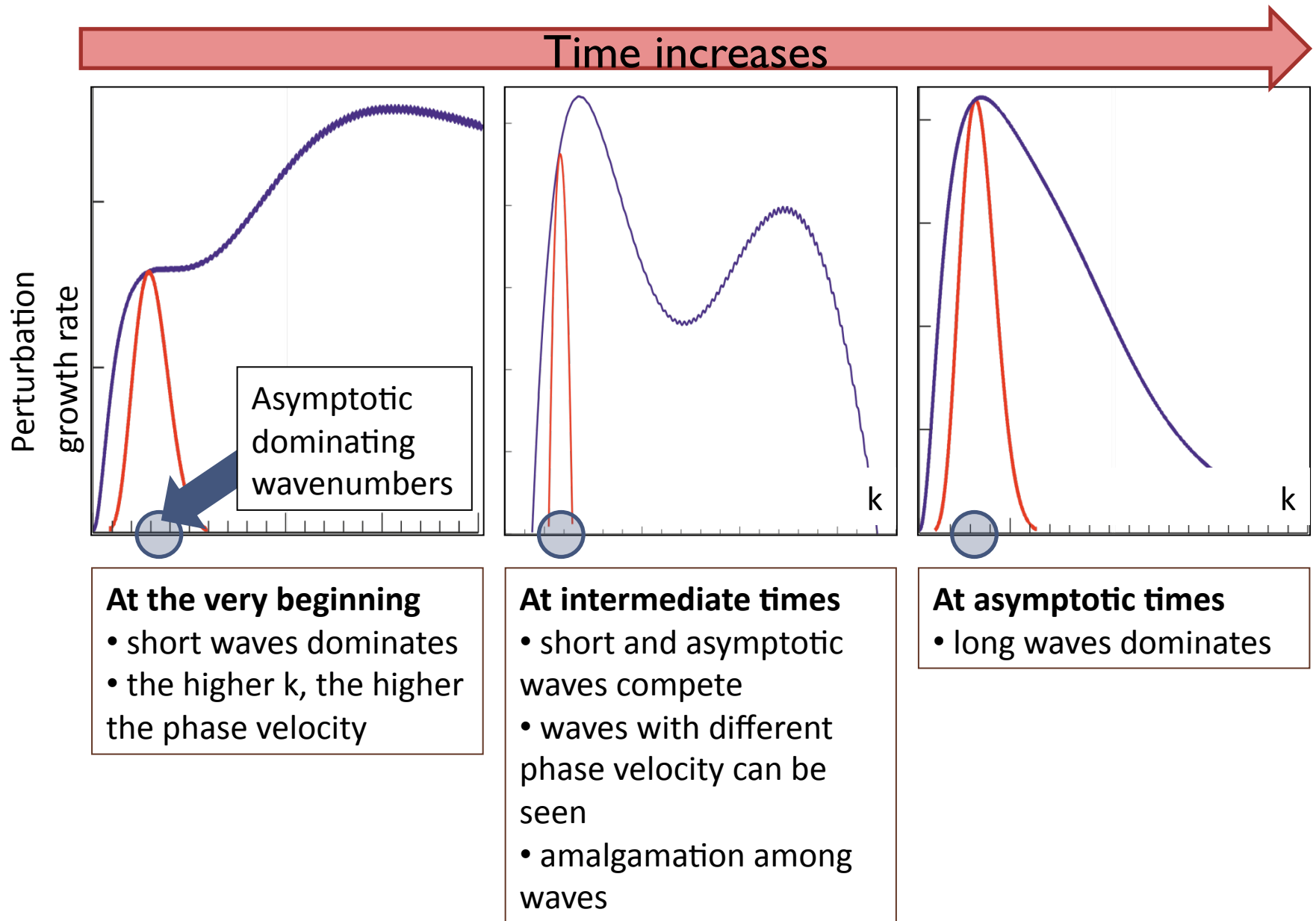
- Long sediment waves and bars (Camporeale, 2011; Vesipa 2012)
- Dunes and antidunes (Camporeale, 2011)

**Nonmodal analysis can possibly explain a number of issues:**

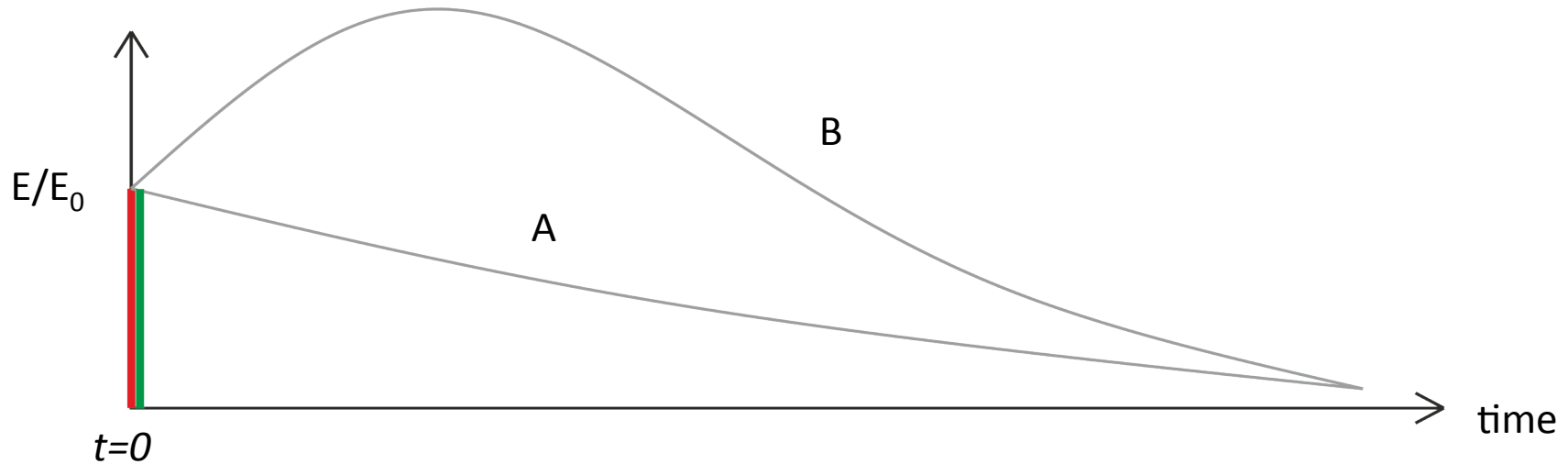
- Wavelength at initial times are shorter than wavelength at asymptotic times (linear transient excitation of stable modes)
- Sediment wave amalgamation is induced by linear mechanisms (excitation of modes with different phase velocity)



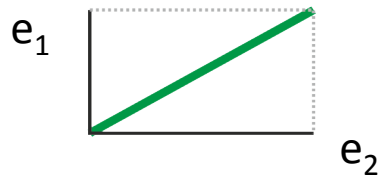
# Non modal analysis and bed form wavelengths



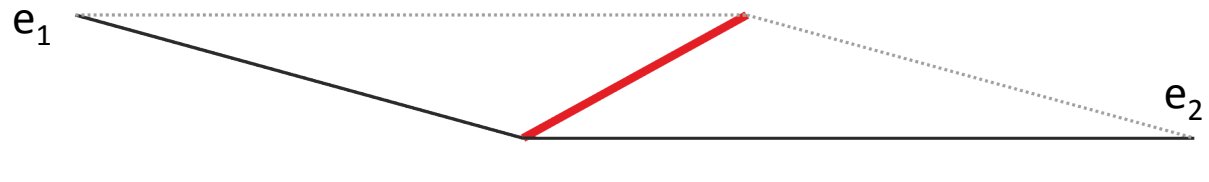
# Energy exchanges among (stable) modes



**$t=0$**



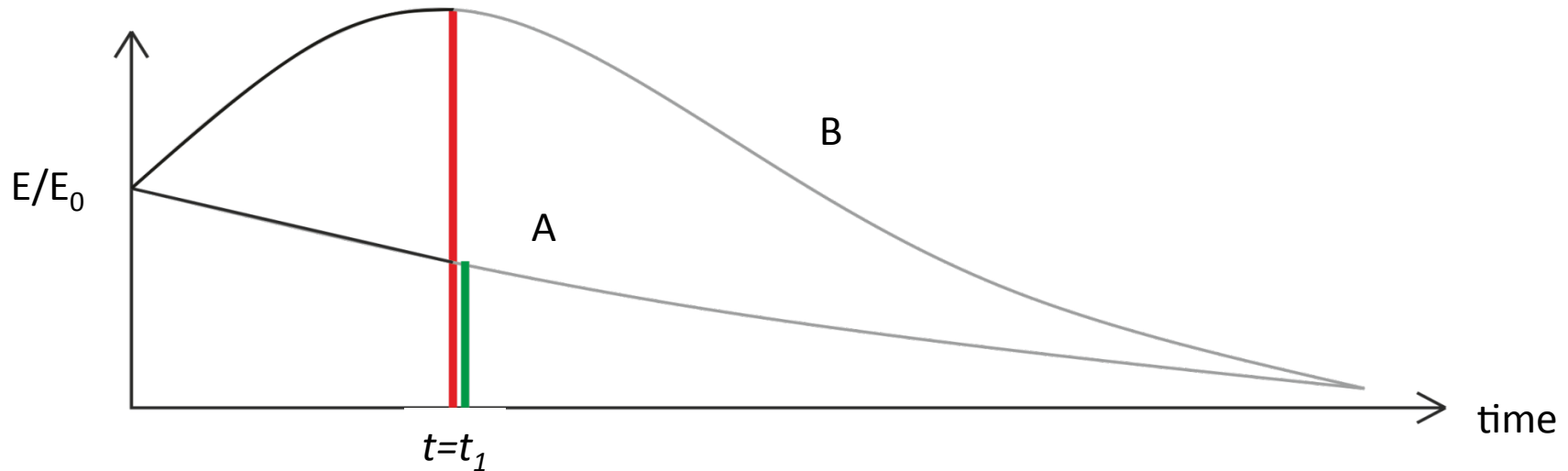
A- Eigenvectors ( $e_1, e_2$ ) are orthogonal



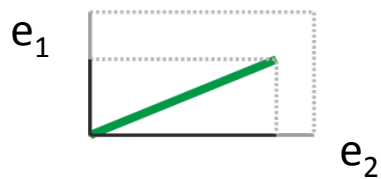
B- Eigenvectors ( $e_1, e_2$ ) are not orthogonal



# Energy exchanges among (stable) modes



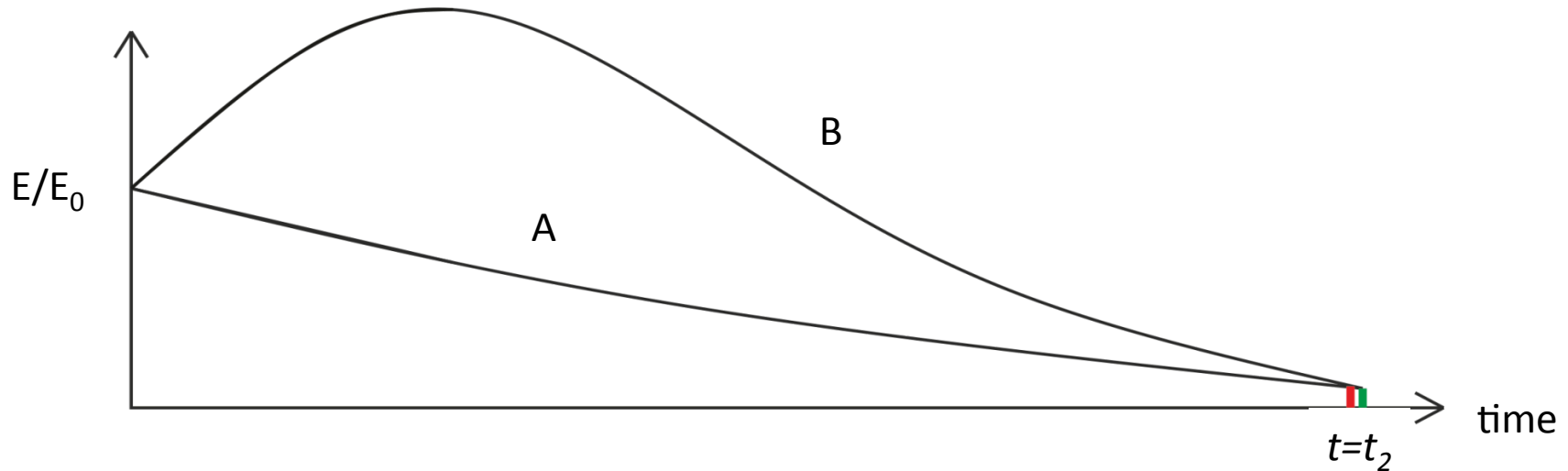
$t=t_1$



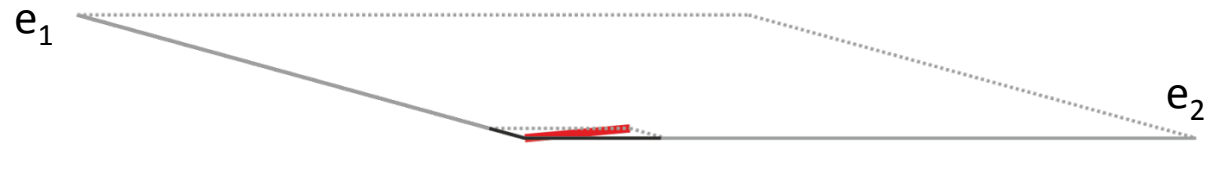
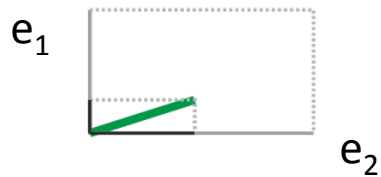
A- Each energetic components of the pert. reduces, the total energy reduces

B- Each energetic components of the pert. reduces, **but the total energy increases**

# Energy exchanges among (stable) modes



$$t=t_2 \approx \infty$$



A- For **asymptotic time** the perturbation energy has decayed

B- After a **transient growth**, unperturbed conditions are restored

# References

Camporeale, C. and Ridolfi, L. 2009 **Nonnormality and transient behavior of the de Saint-Venant-Exner equations** *Water Resources Research* 45 W08418

Camporeale, C. and Ridolfi, L. 2011 **Modal versus nonmodal linear stability analysis of river dunes** *Physics of Fluids* 23(10) 104102

Schmid P.J. and Henningson D.S. 2001 *Stability and Transition in Shear Flows*, Springer

Vesipa, R. and Camporeale, C. and Ridolfi, L. 2012 **Transient growths of stable modes in riverbed dynamics** *EPL* 100(6) 64002