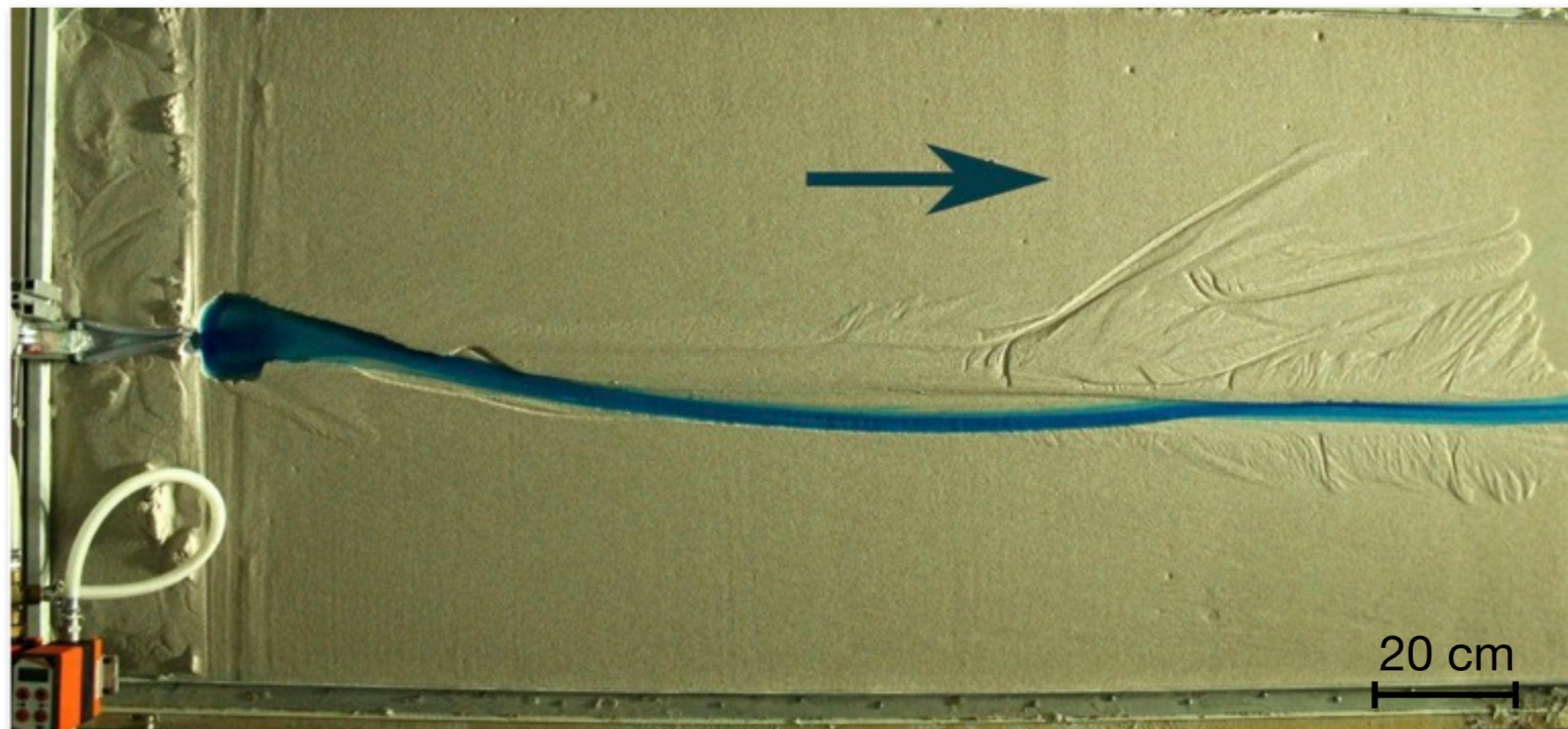


Equilibrium morphology of rivers : Insights from small scale experiments

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1. Institut de Physique du Globe de Paris

2. Earth and Environmental Science dpt, University of Pennsylvania



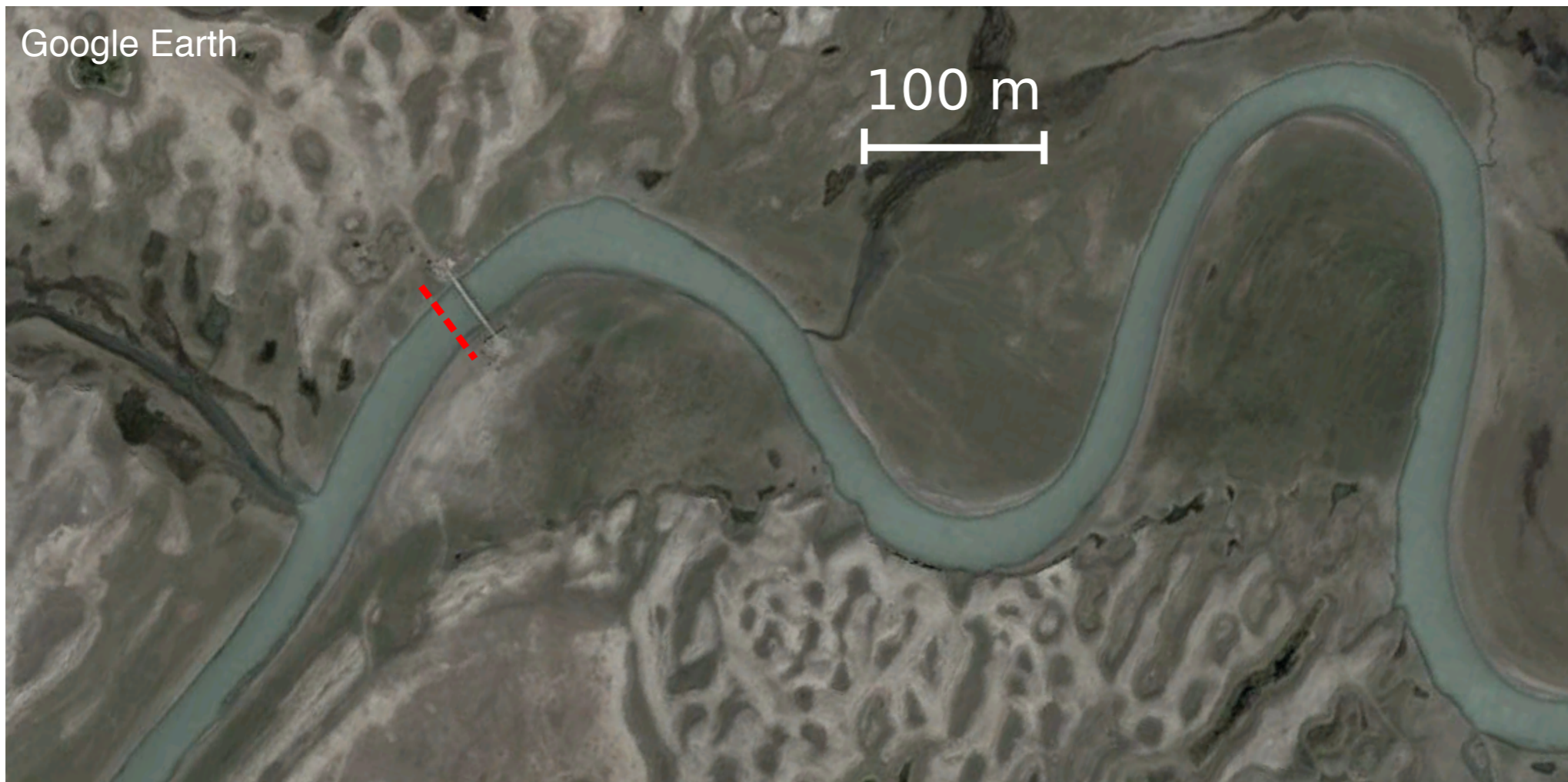
Kaidu river,
chinese Tian-Shan

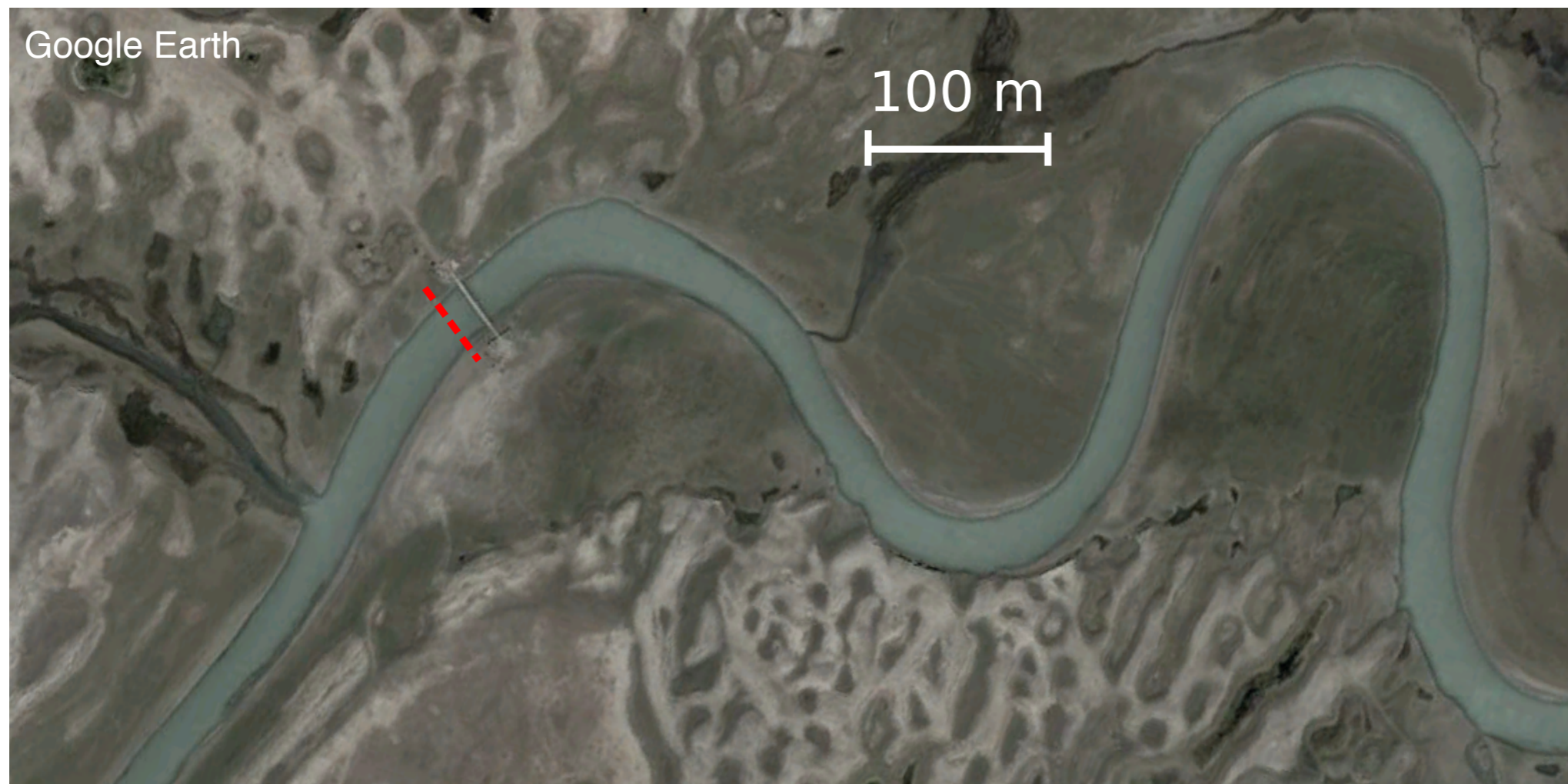
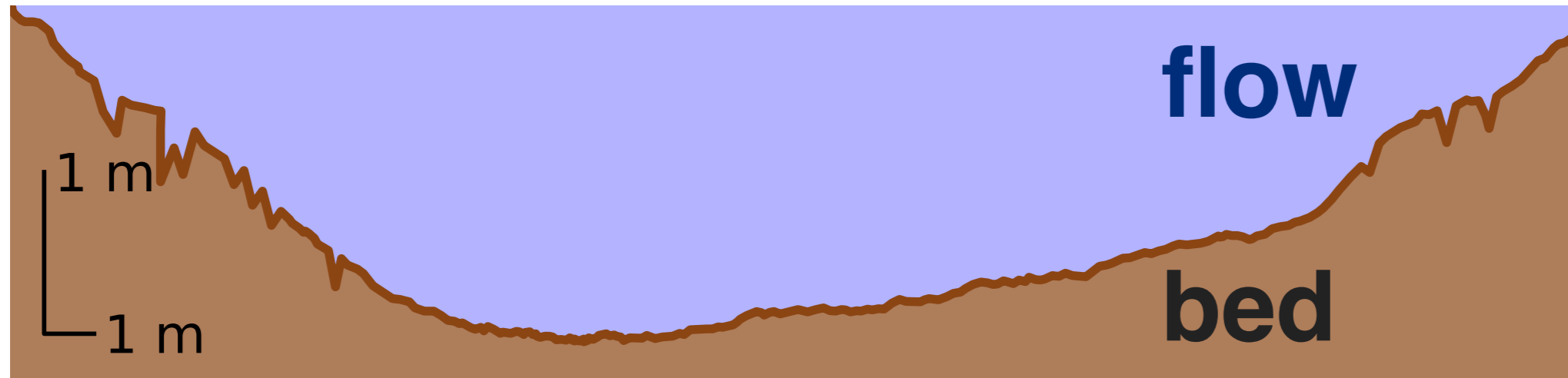


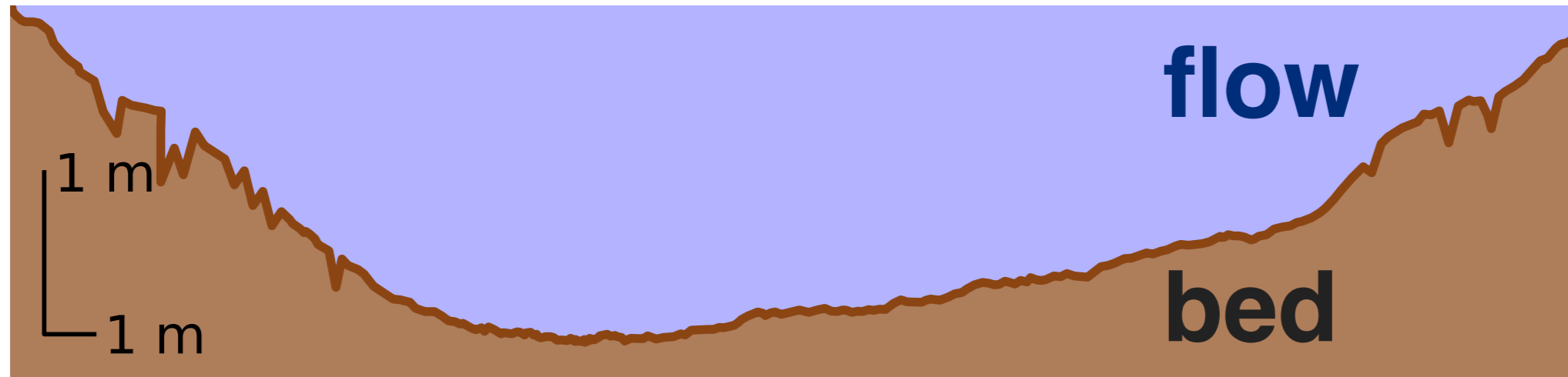
Kaidu river,
chinese Tian-Shan



Google Earth

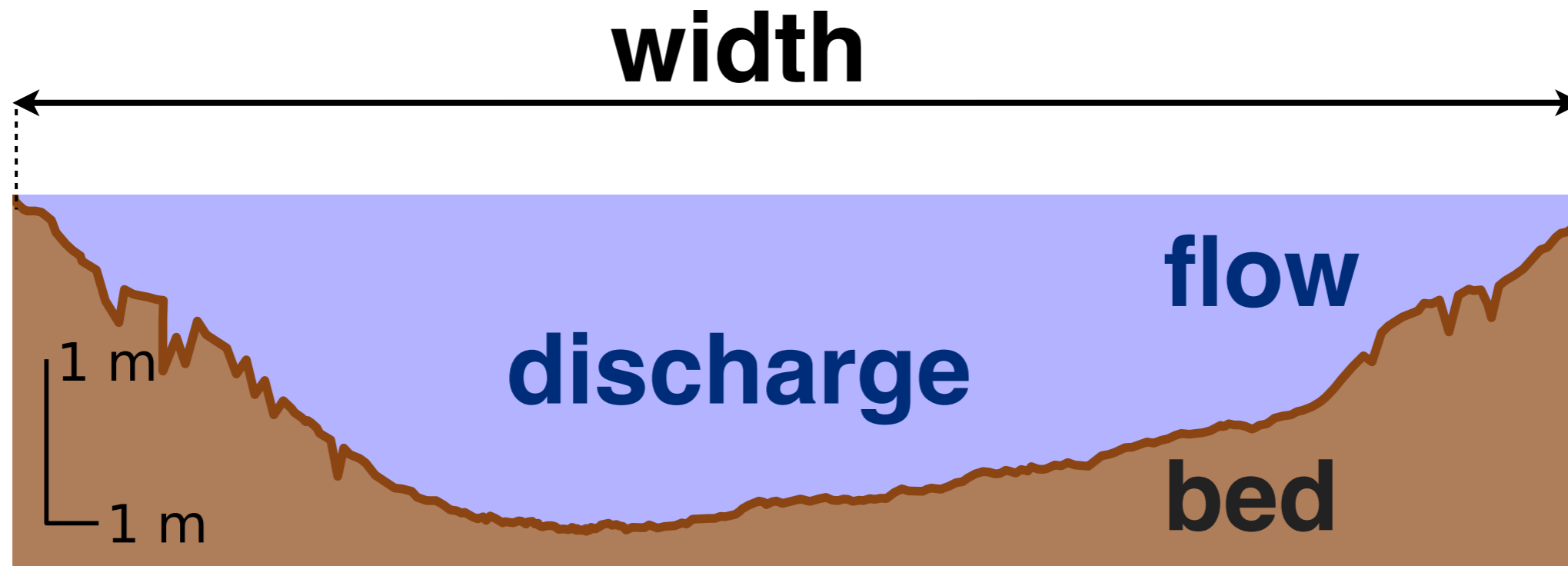






- **What selects the shape of the cross-section ?**
- **What parameters control its size ?**

Parker [1978], Vigilar & Diplas [1997], Cao & Knight [1998], Eaton & Millar [2004], ...

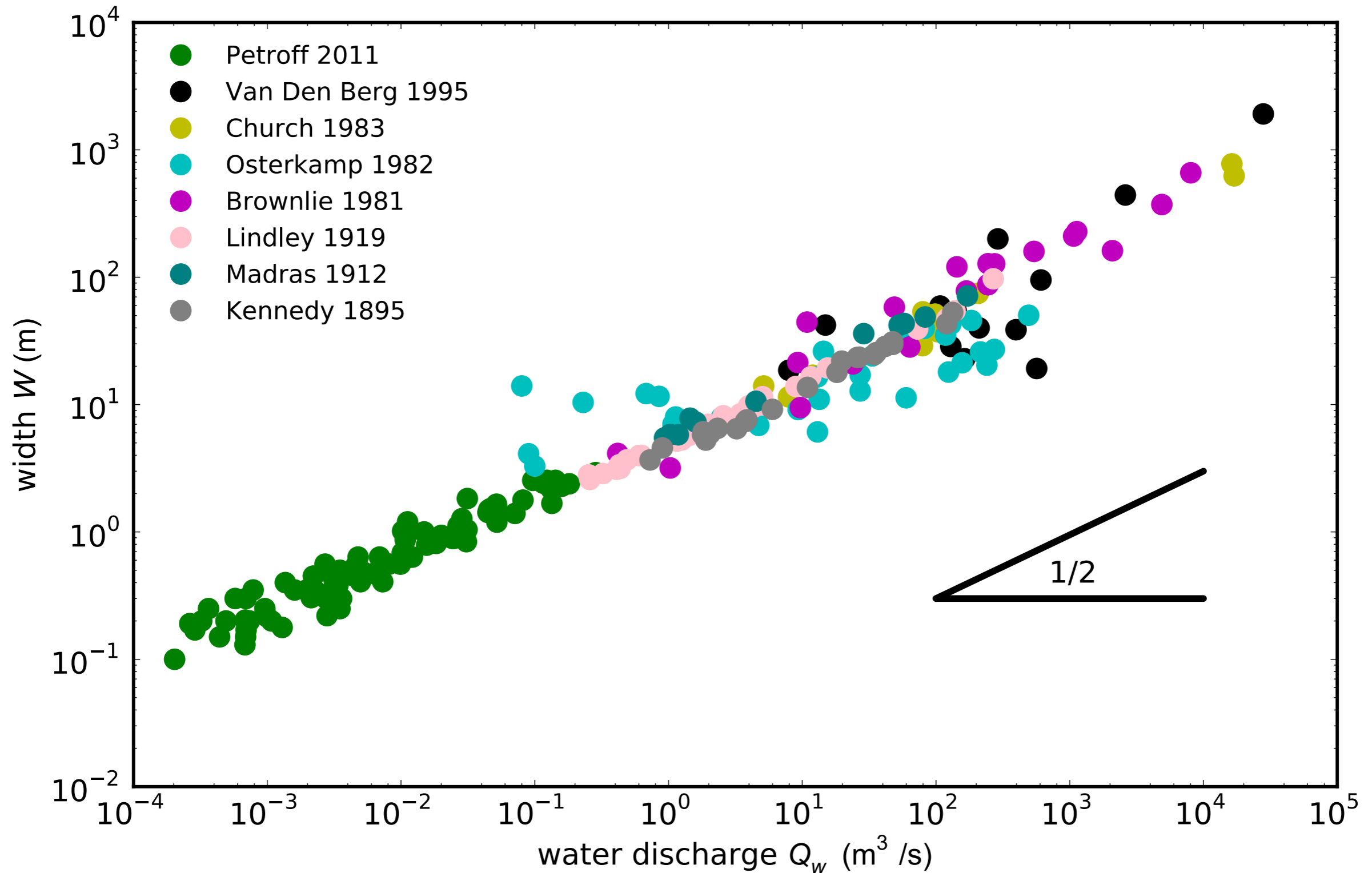


- **What selects the shape of the cross-section ?**
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Parker [1978], Vigilar & Diplas [1997], Cao & Knight [1998], Eaton & Millar [2004], ...

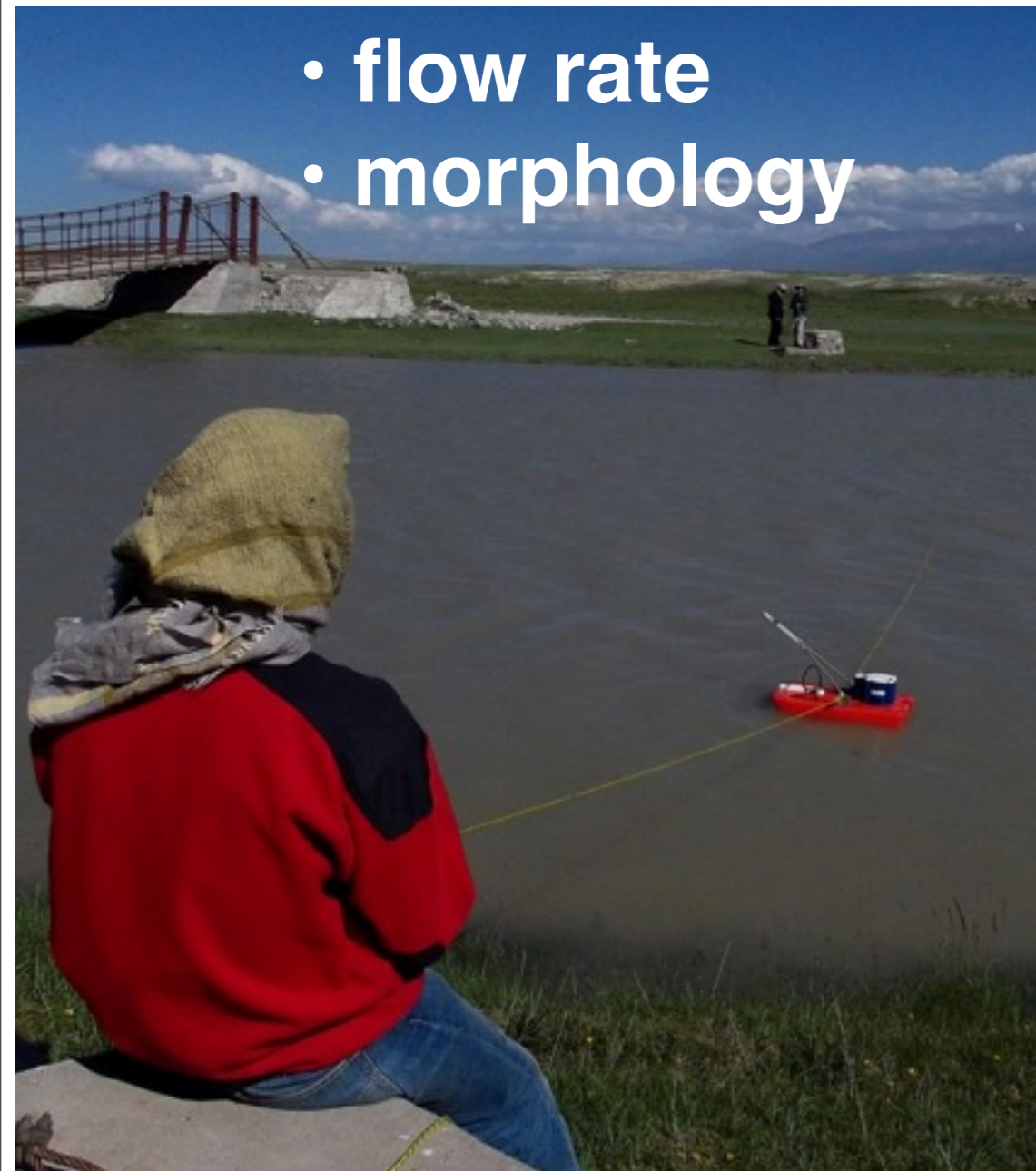
Lacey's law - sandy, single thread rivers

data from Brownlie [1981], Church & Rood [1983], Osterkamp & Hedman [1982], Vand den Berg [1995], Devauchelle et al [2010]



Field measurements

- flow rate
- morphology



- sediment transport



- granulometry

Field measurements

- flow rate
- morphology

- sediment transport

But ...

- 1. direct observation of the physical processes is difficult,**
- 2. parameters (discharge, grain size, ...) cannot be varied independantly.**

- granulometry

Field measurements

mosquitoes



Field measurements

mosquitoes



dust storms



Field measurements

mosquitoes



dust storms



swimming cows



Laboratory rivers



water + sugar or glycerol

$$\rho = 1.2 \cdot 10^3 \text{ kg m}^{-3}$$

$$\nu = 15 \cdot 10^{-6} \text{ m}^2 \text{ s}^{-1}$$

$$\text{Re} \approx 10$$

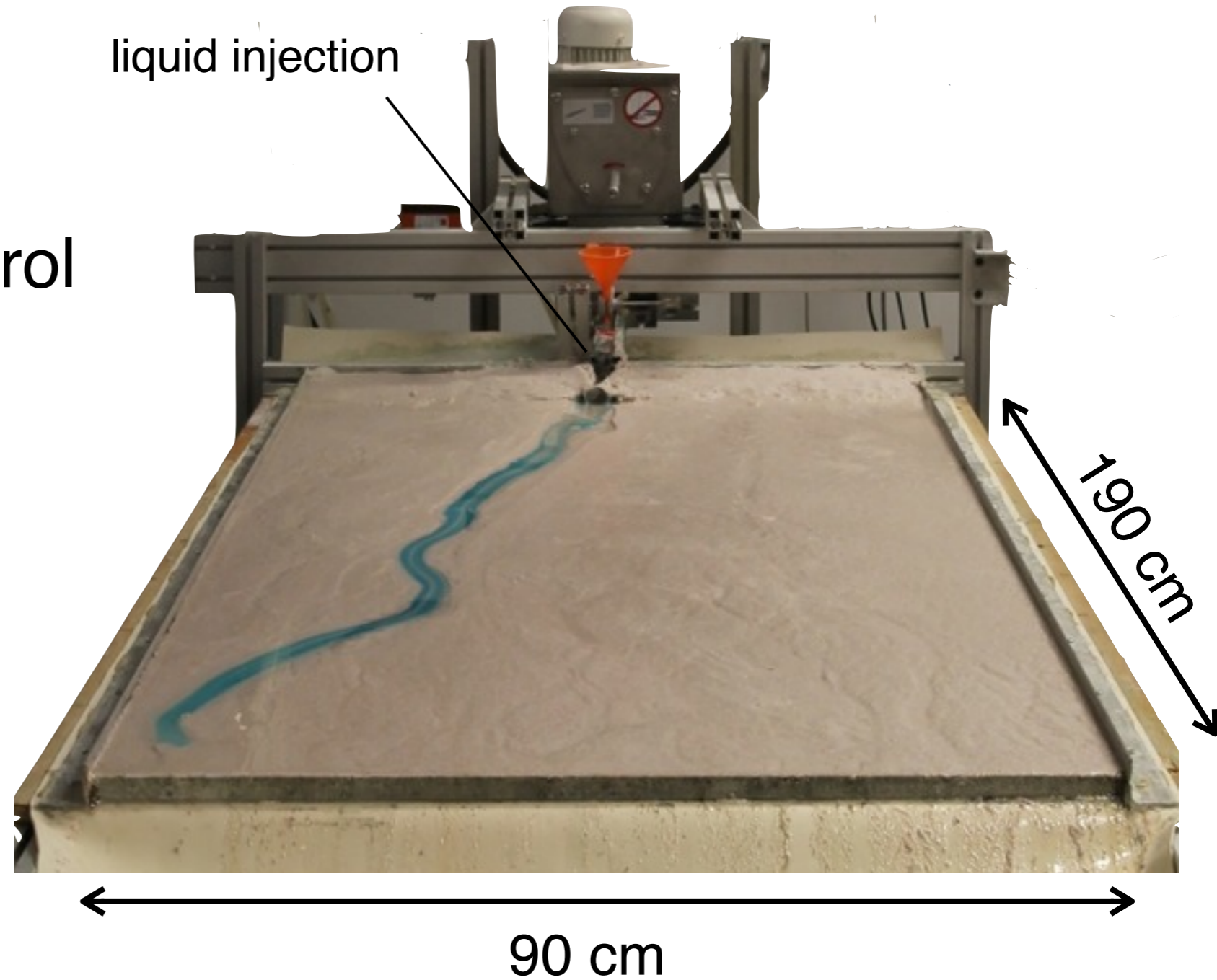


plastic sand

$$d_s = 250 \text{ mm}$$

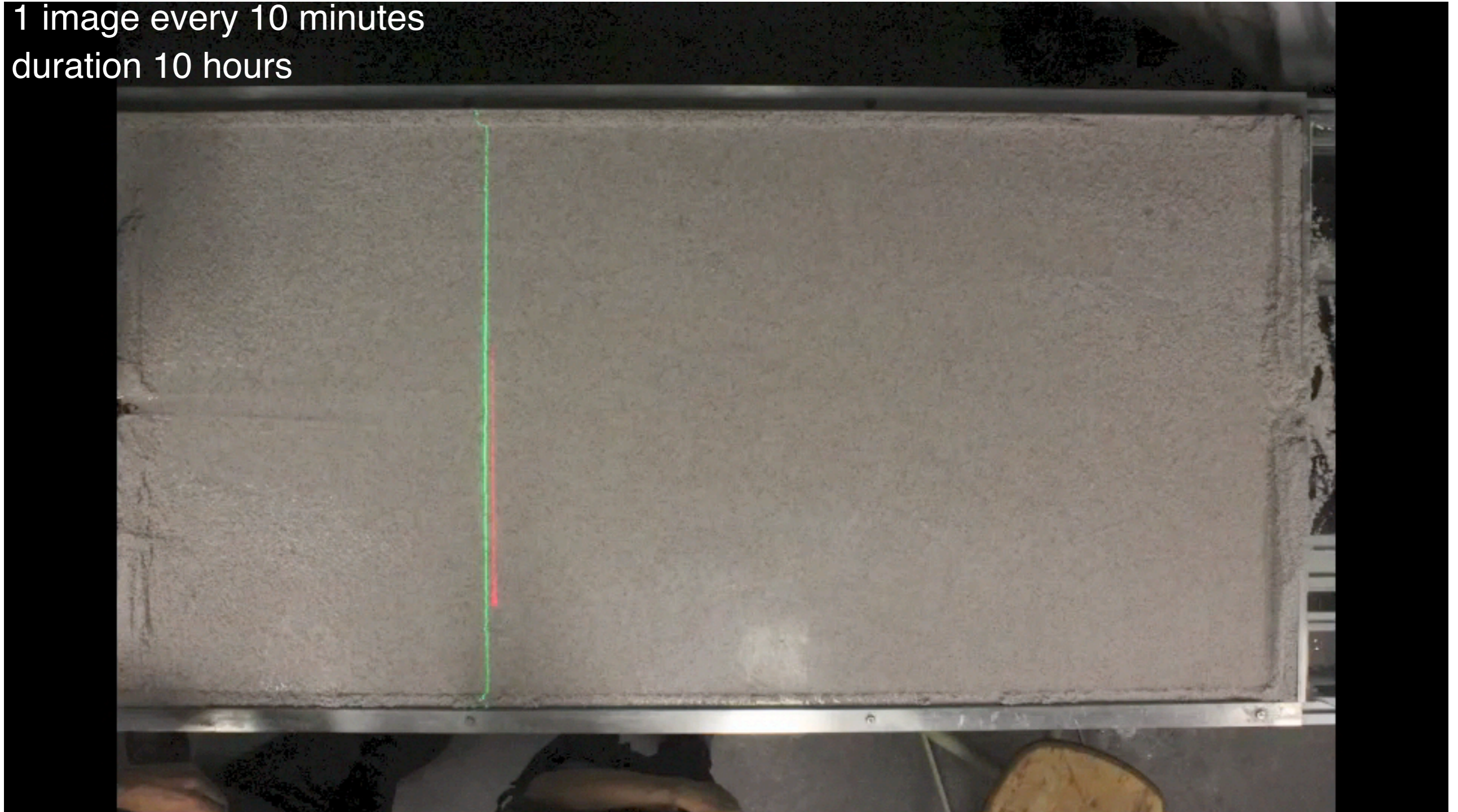
$$\rho_s = 1.5 \cdot 10^3 \text{ kg m}^{-3}$$

$$\alpha_a = 35^\circ$$



Experimental procedure

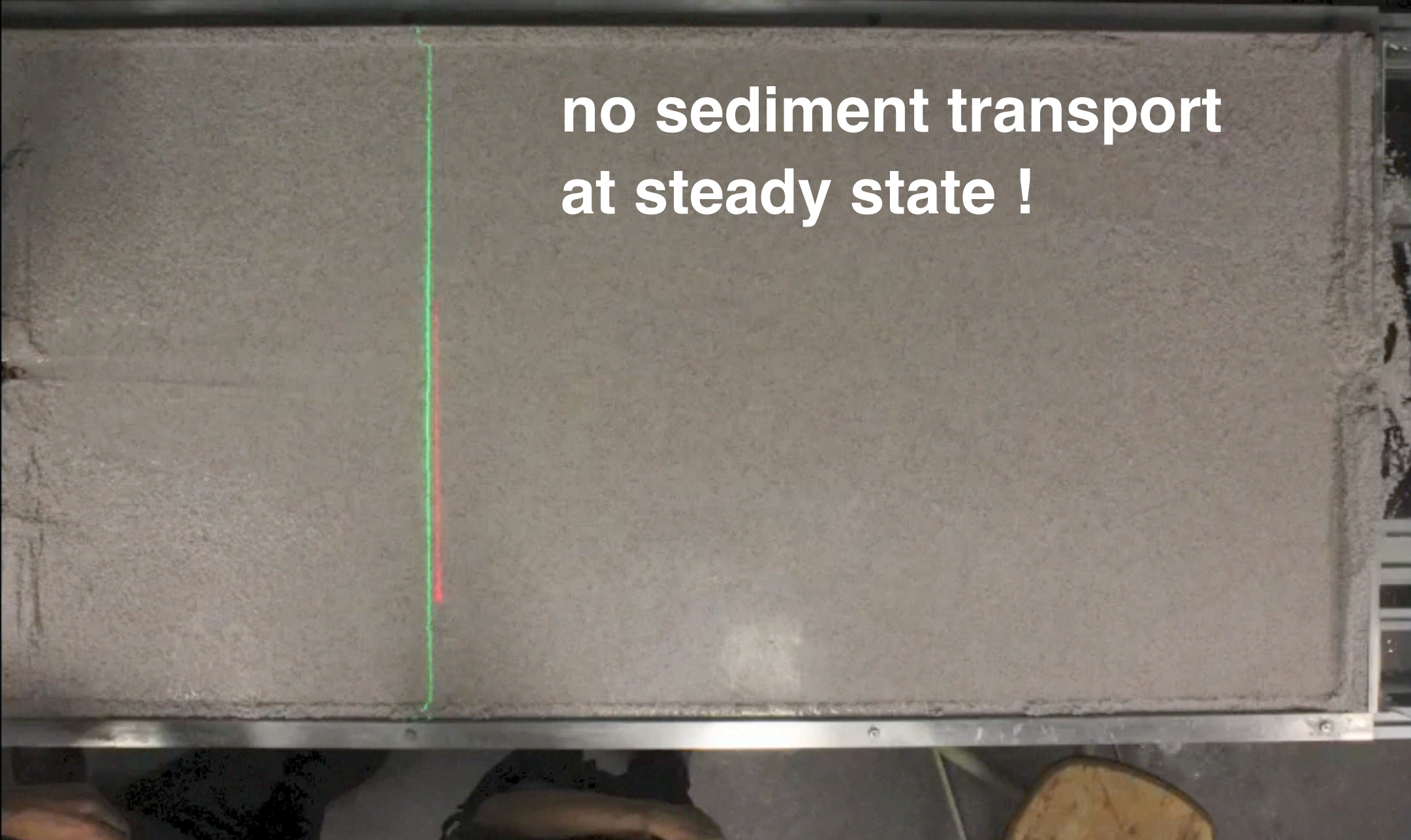
1 image every 10 minutes
duration 10 hours



constant discharge
no sediment input

Experimental procedure

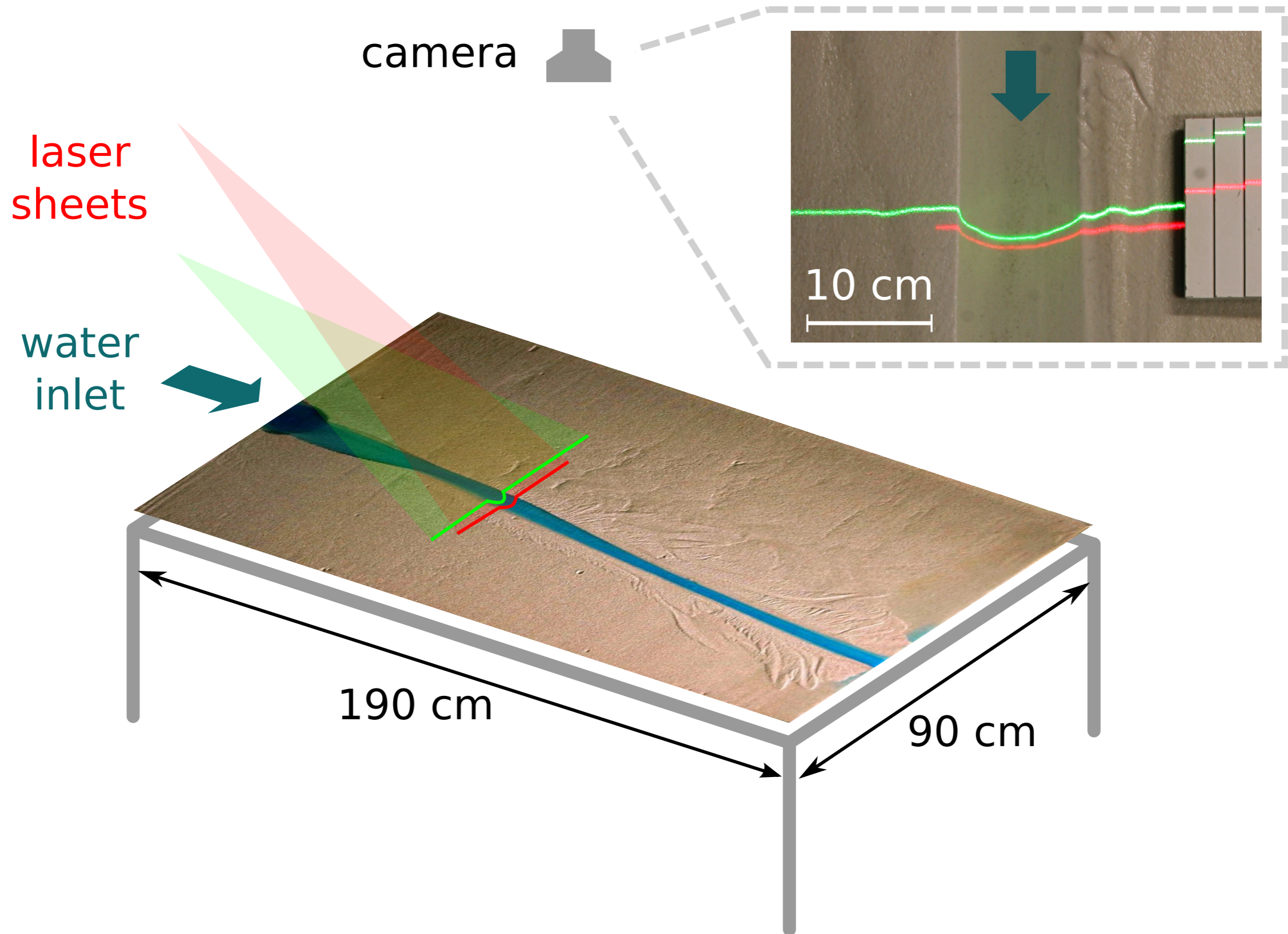
1 image every 10 minutes
duration 10 hours



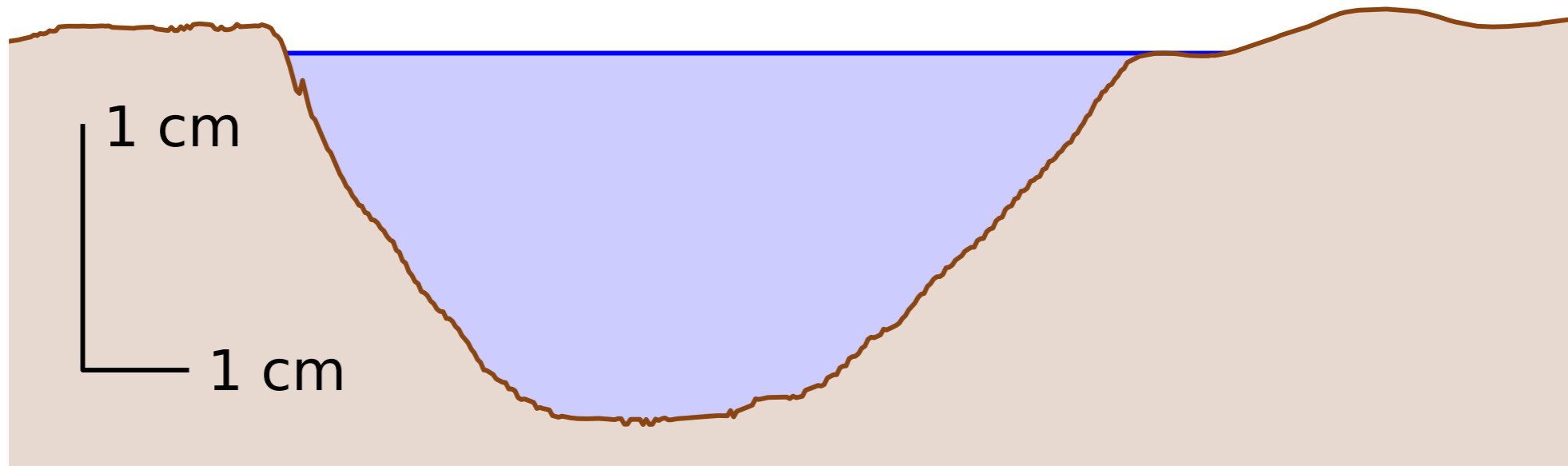
no sediment transport
at steady state !

constant discharge
no sediment input

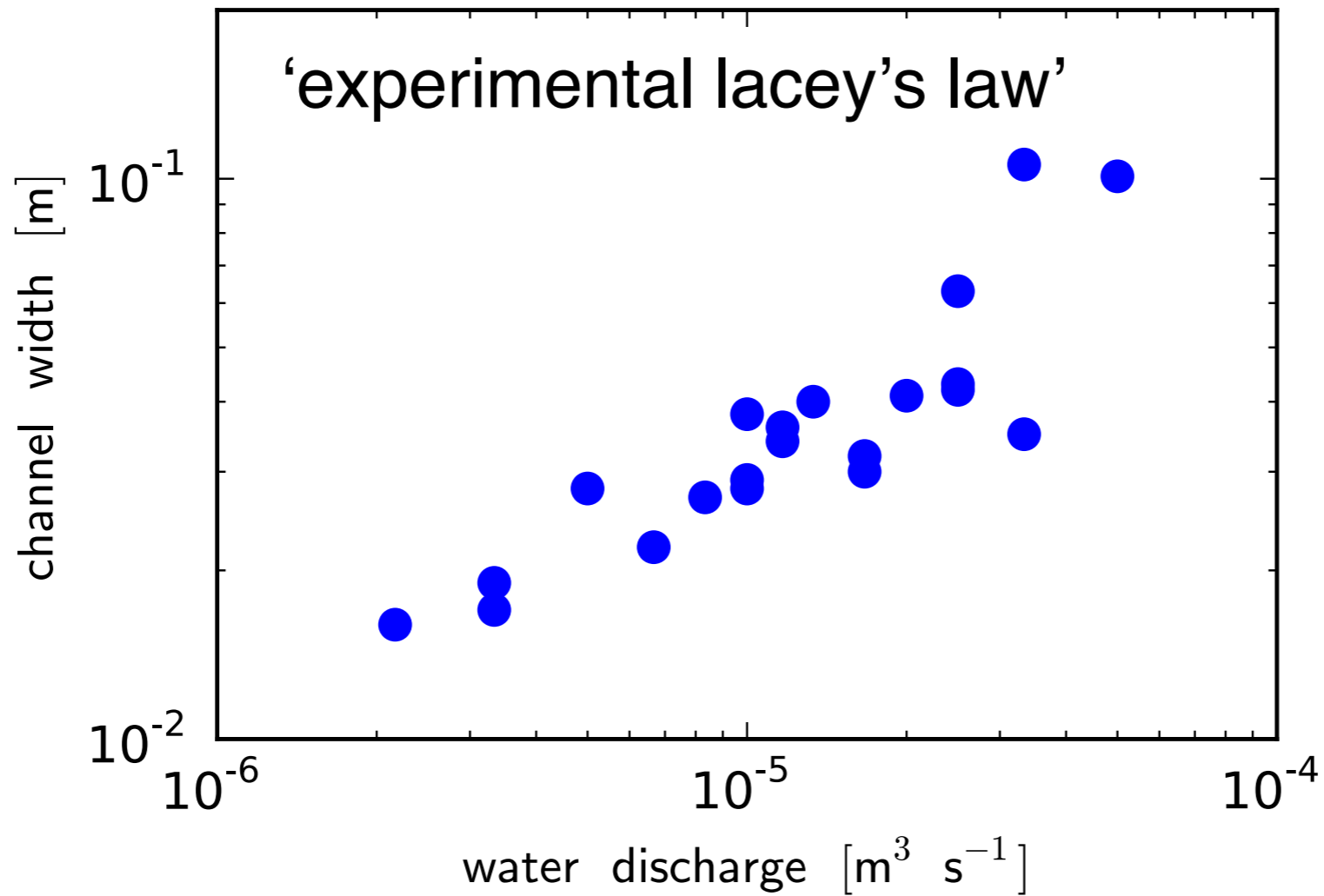
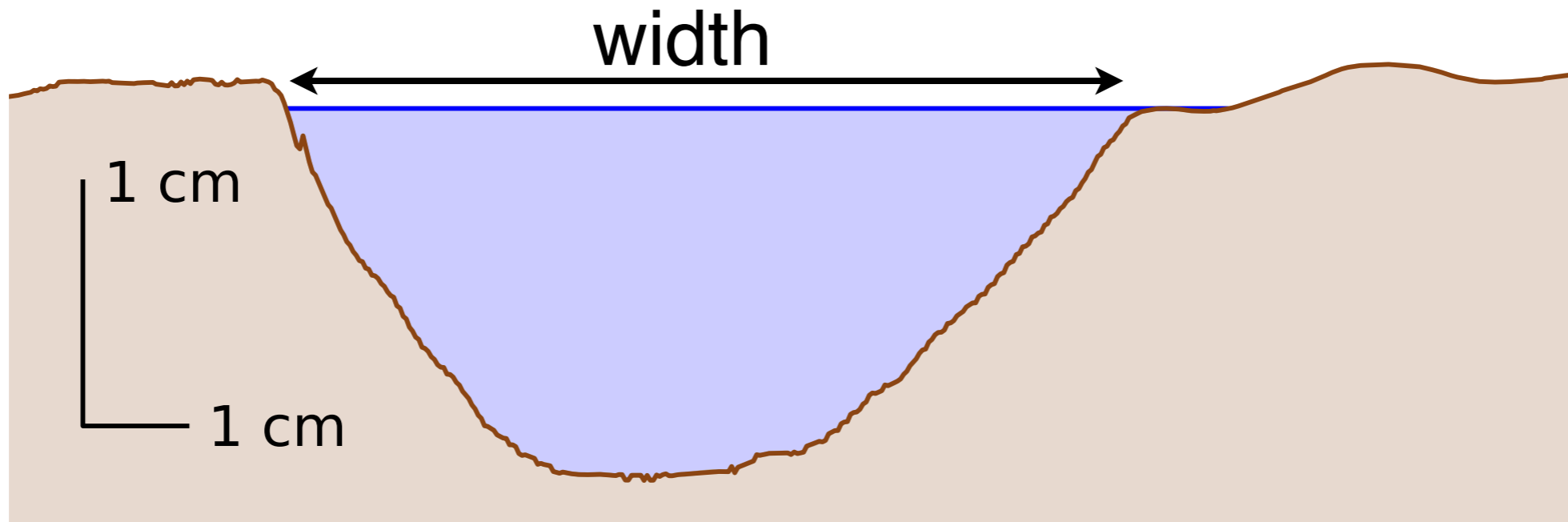
Measurements of flow depth and bed topography



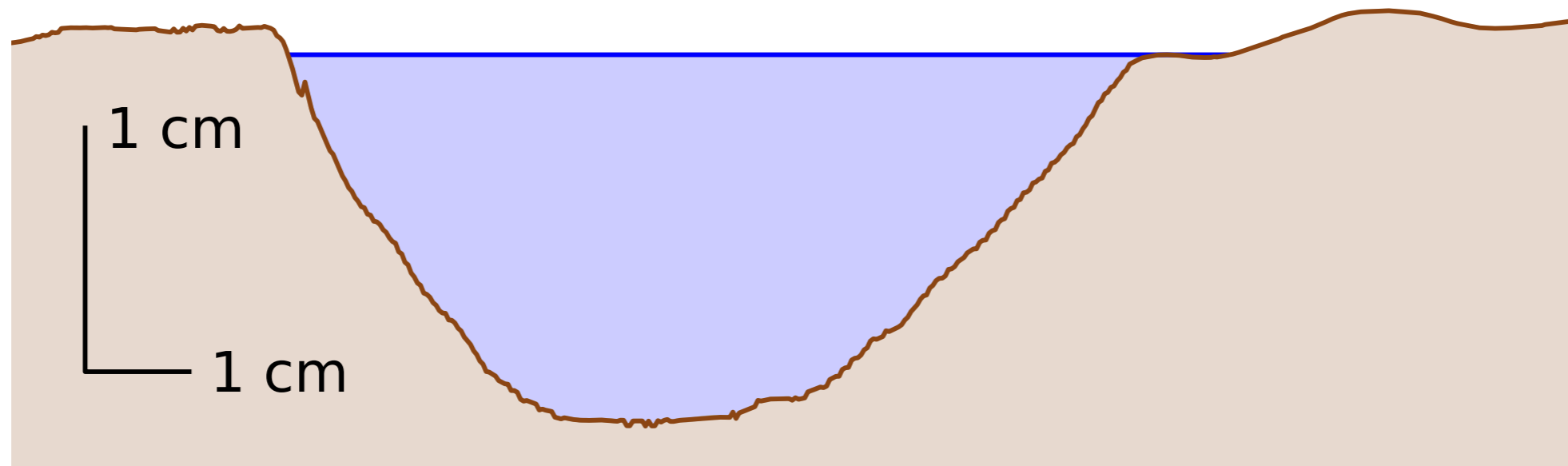
Laboratory rivers



Laboratory rivers



Zero-transport model



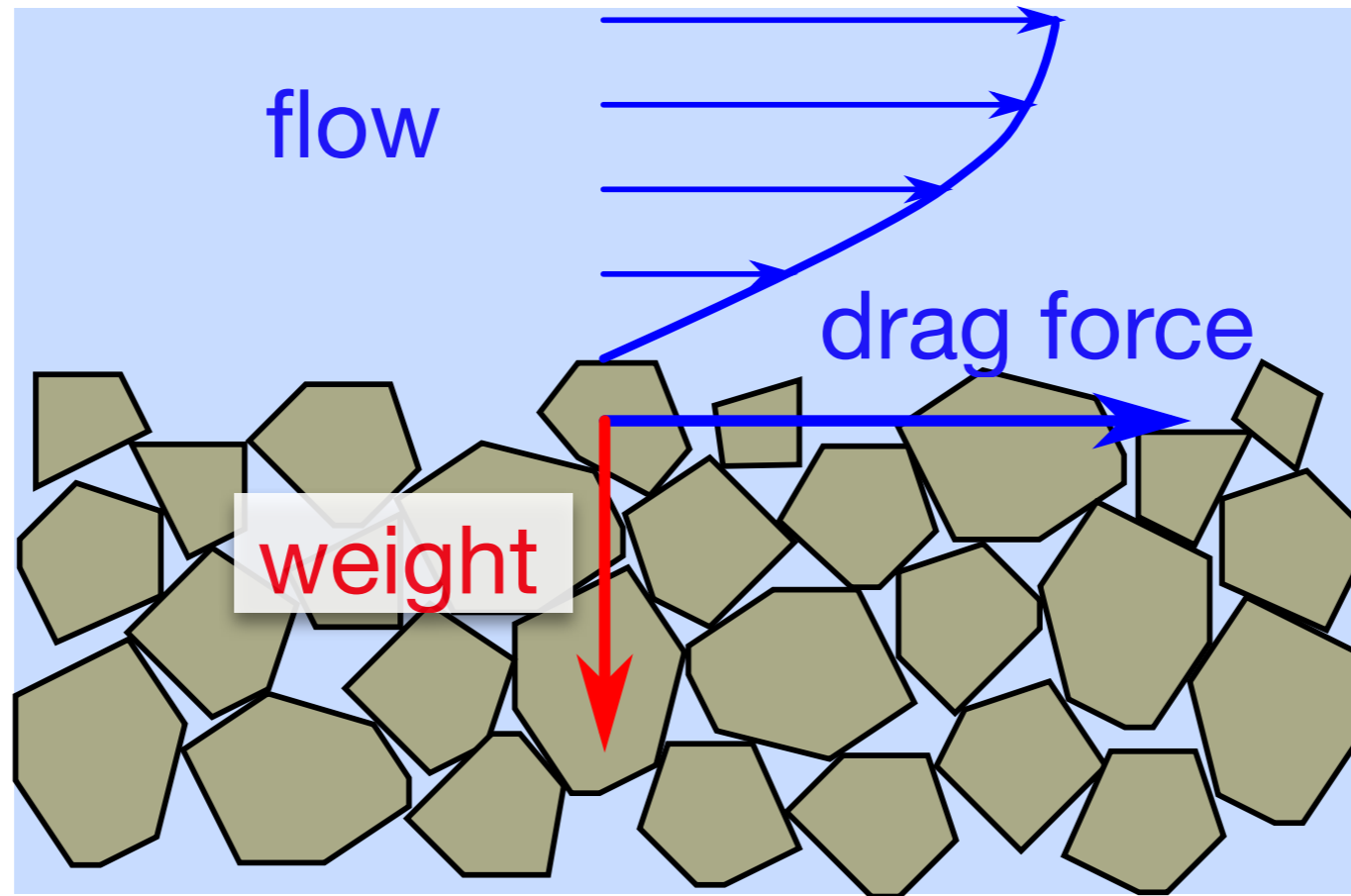
The equilibrium morphology is reached when sediment transport ceases.

At equilibrium, grains must be at the threshold of entrainment everywhere on the bed!

[Glover & Florey, 1951; Henderson, 1961]

Threshold of motion for a flat sediment bed

Coulomb's law of friction ...

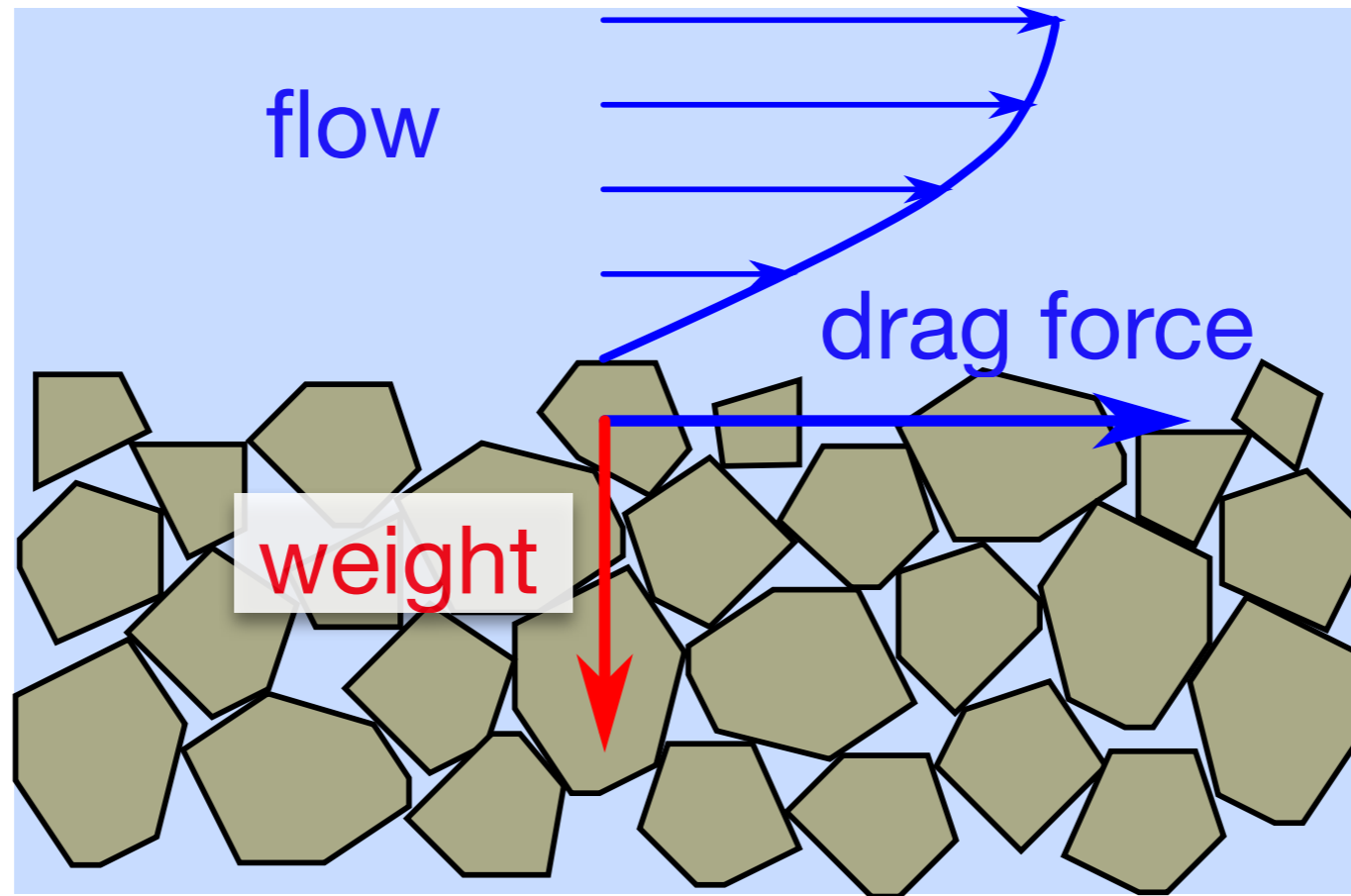


$$\frac{\text{tangeantial force}}{\text{normal force}} = \mu$$

friction coefficient

Threshold of motion for a flat sediment bed

Coulomb's law of friction ...



drag force

tangeantial force

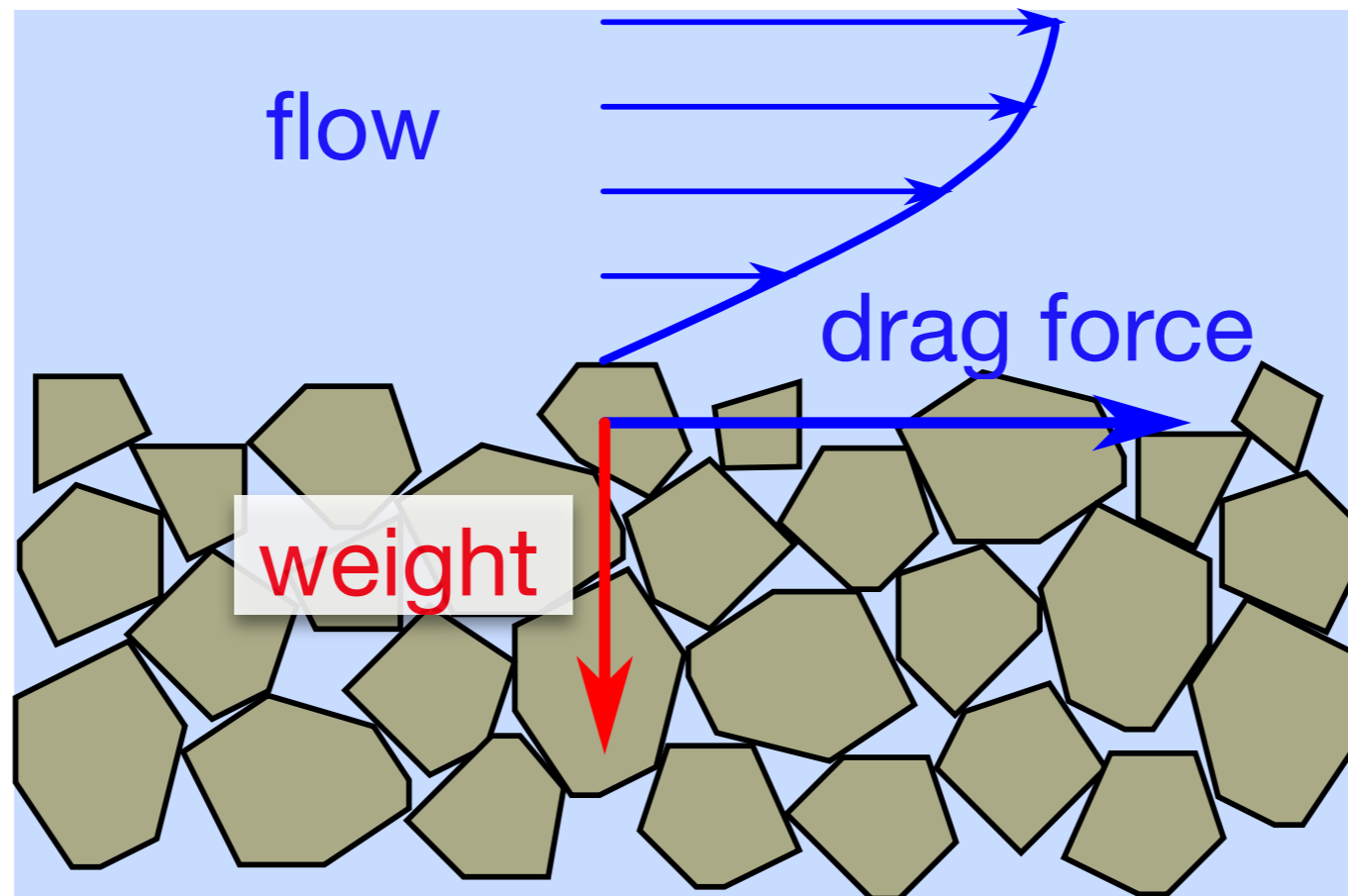
normal force

weight

= μ

friction coefficient

Threshold of motion for a flat sediment bed ... and Shields number



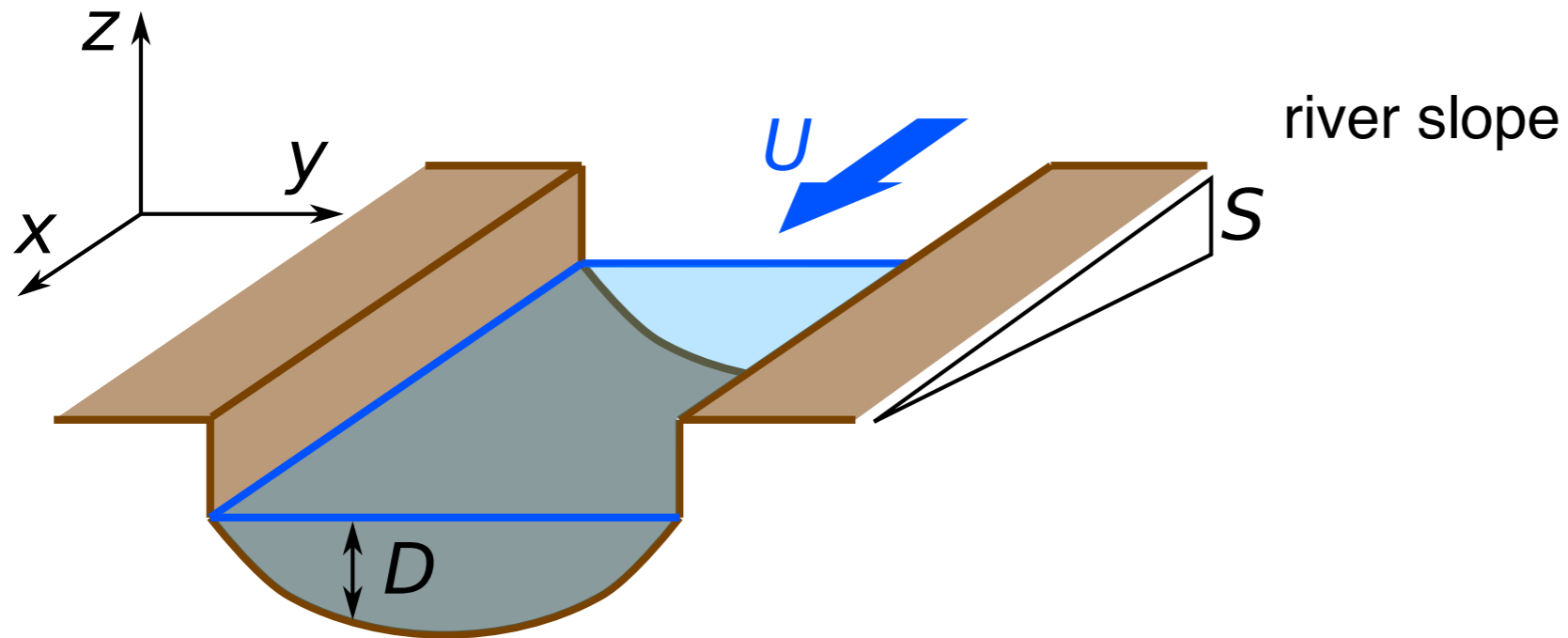
$$\frac{\tau}{(\rho_s - \rho_f)gd_s} = \theta_t$$

Shields number threshold Shields number

[Shields, 1936]

t = shear stress
 d_s = grain size
 ρ_s, ρ_f = sediment
 & fluid densities

Threshold of motion for a channel

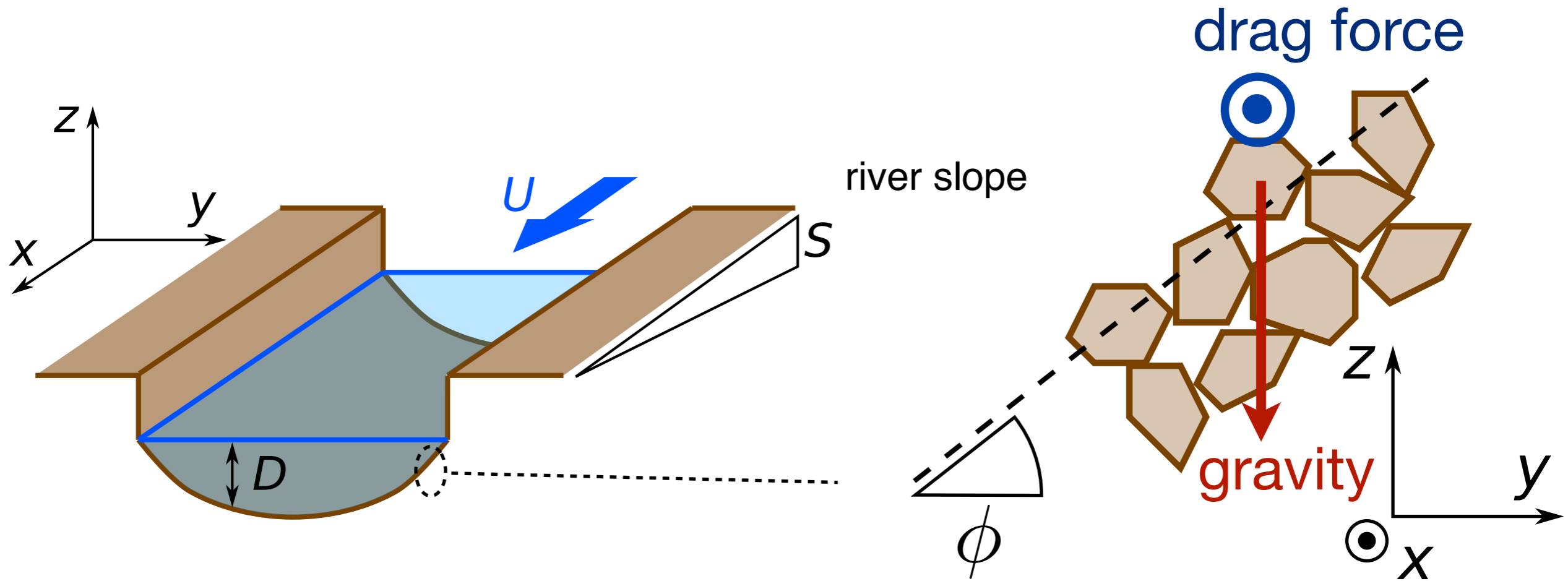


Coulomb's law of friction :

$$\frac{\text{tangeantial force}}{\text{normal force}} = \mu$$

friction coefficient

Threshold of motion for a channel

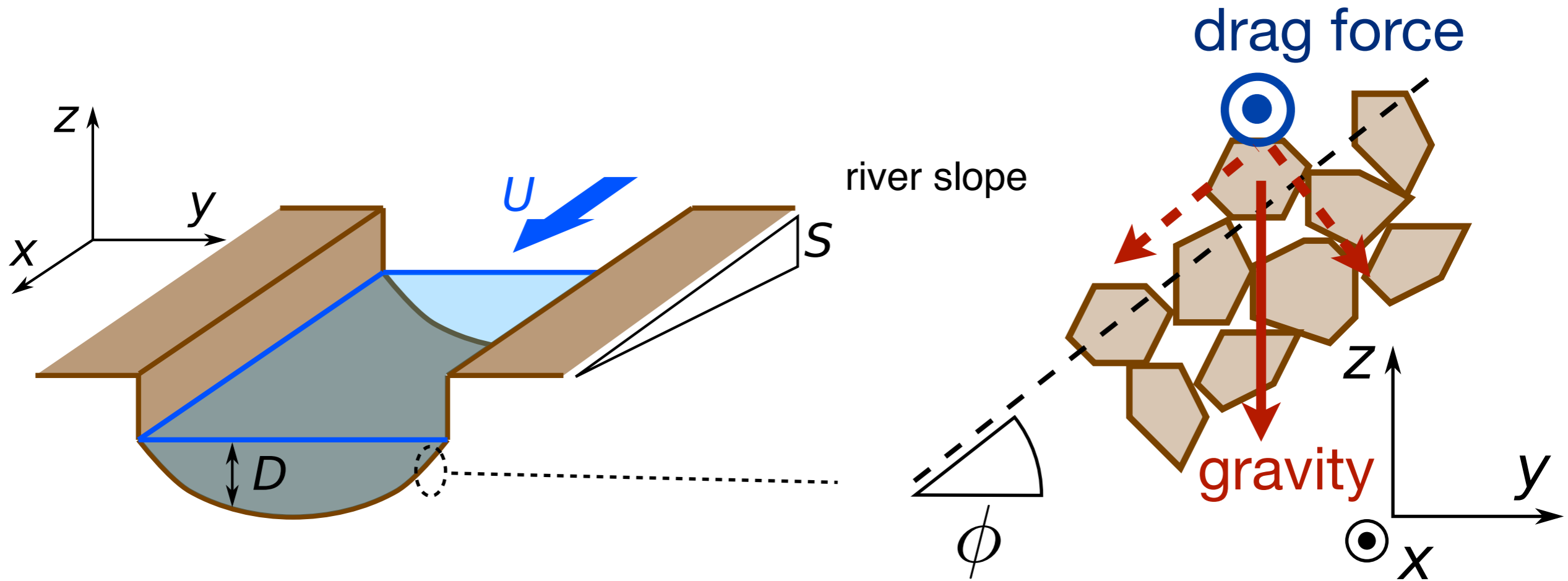


Coulomb's law of friction :

$$\frac{\text{tangeantial force}}{\text{normal force}} = \mu$$

friction coefficient

Threshold of motion for a channel

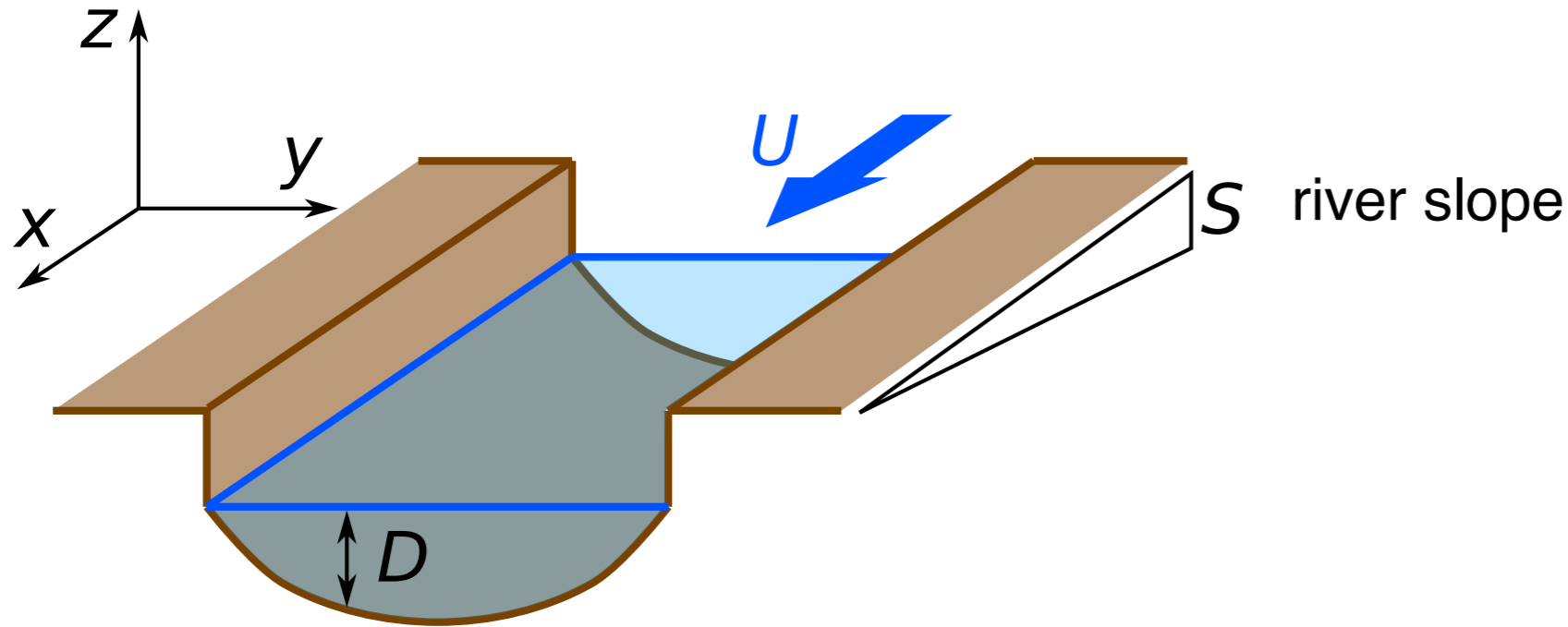


Coulomb's law of friction :

$$\frac{\text{tangeantial force}}{\text{normal force}} = \mu$$

friction coefficient

Threshold of motion for a channel



$$\left(\frac{\partial D}{\partial y}\right)^2 + \left(\frac{S D}{L}\right)^2 = \mu^2$$

streamwise slope

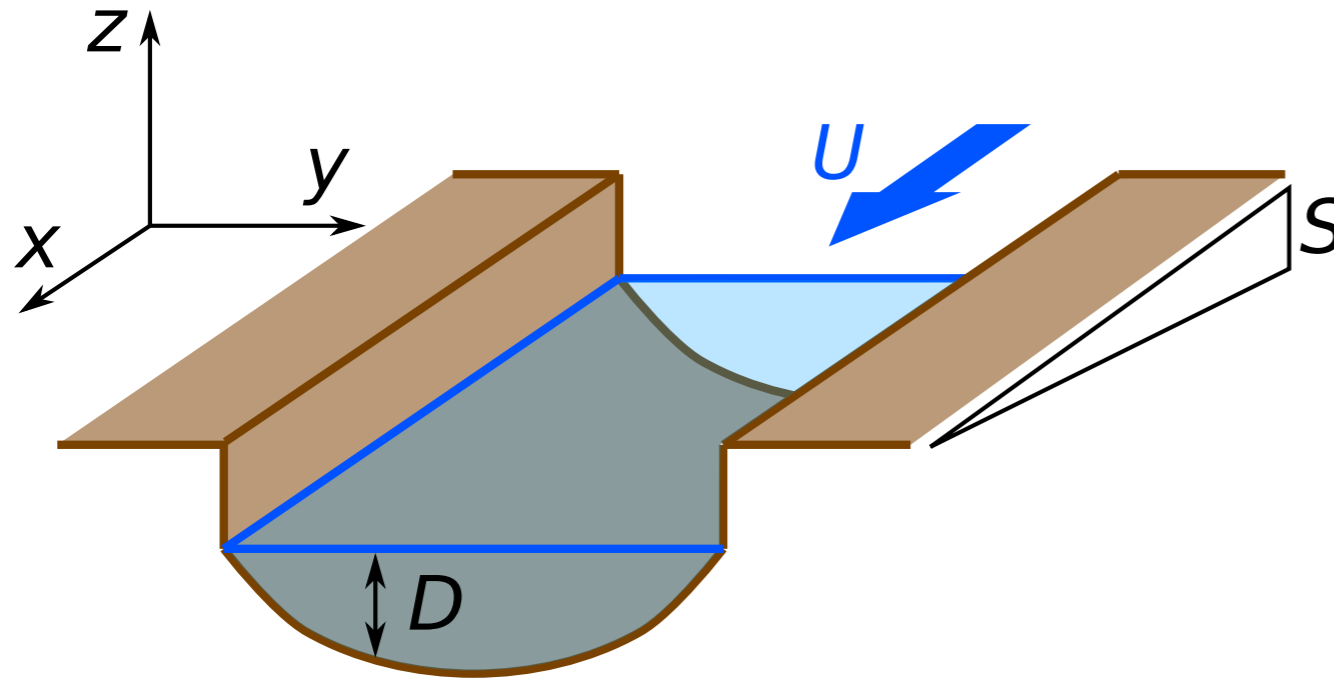
friction coefficient ≈ 0.7

characteristic length \approx grain size

weight **drag force**

[Seizilles et al., 2013]

Threshold of motion for a channel



river slope

threshold Shields stress sediment & fluid density grain size

$$L = \frac{\theta_t (\rho_s - \rho_f) d_s}{\mu \rho_f}$$

streamwise slope

friction coefficient ≈ 0.7

$$\left(\frac{\partial D}{\partial y} \right)^2 + \left(\frac{S D}{L} \right)^2 = \mu^2$$

characteristic length \approx grain size

weight

drag force

[Seizilles et al., 2013]

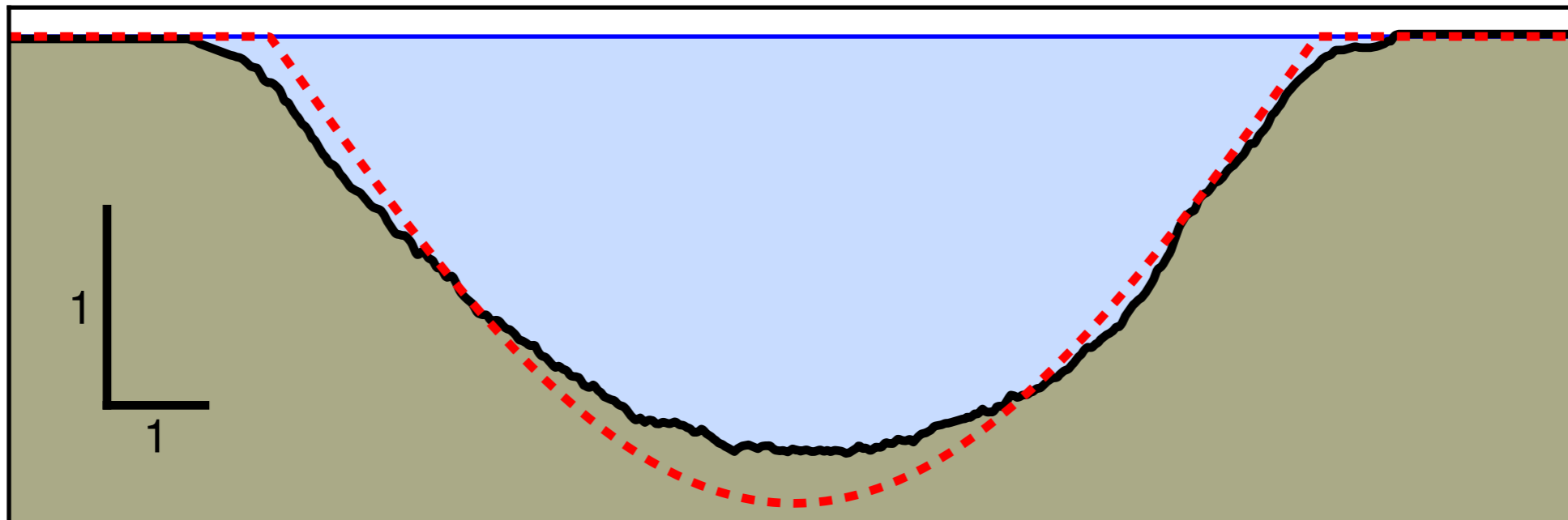
Zero-transport model

friction coefficient

longitudinal slope

$$D = \frac{\mu L}{S} \cos \left(\frac{S y}{L} \right)$$

characteristic length \approx grain size



no adjustable parameter !

Zero-transport model

friction coefficient

streamwise slope

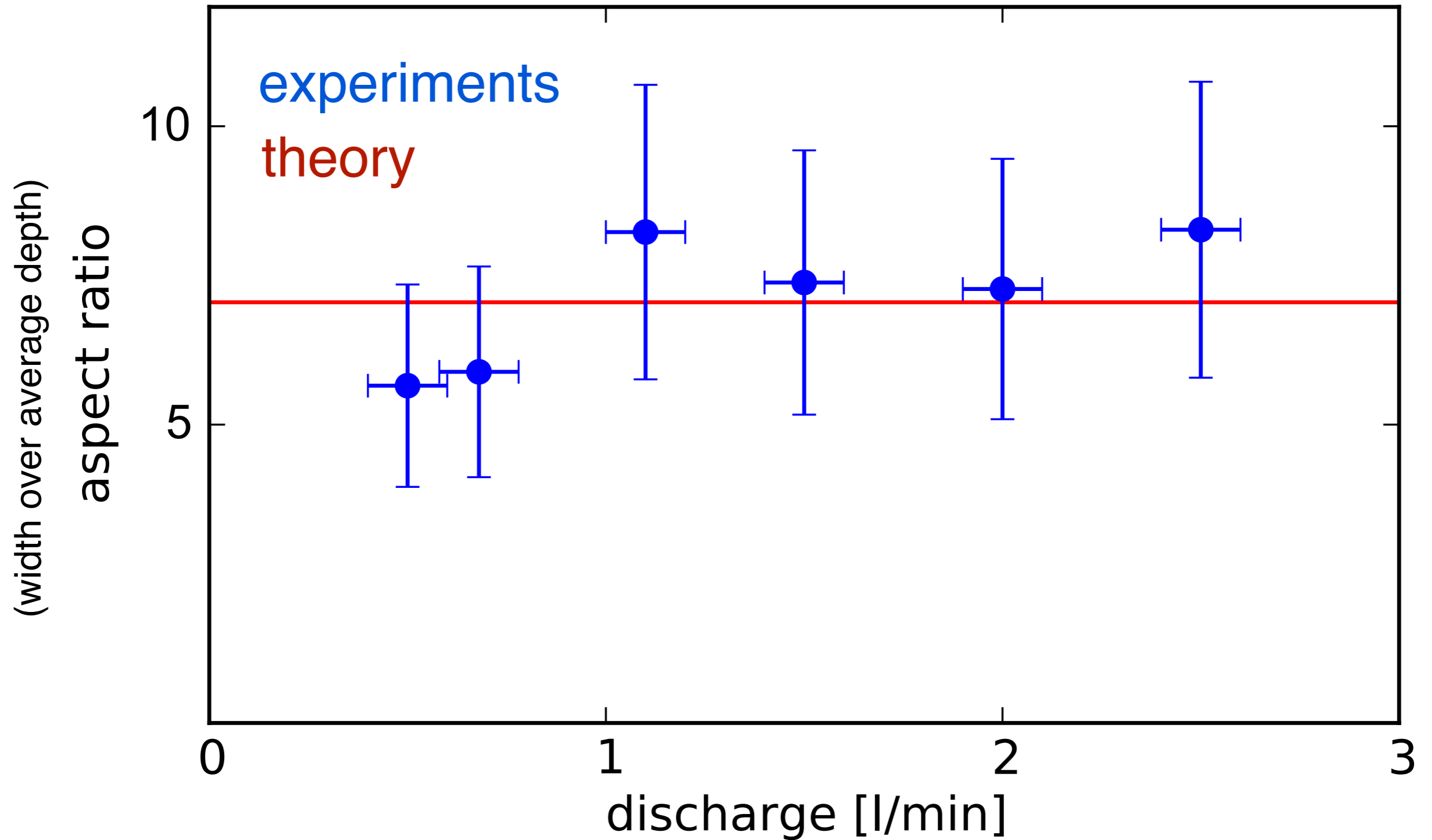
$$D = \frac{\mu L}{S} \cos \left(\frac{S y}{L} \right)$$

characteristic length \approx grain size

Channel characteristic depth and width scale as L / S !

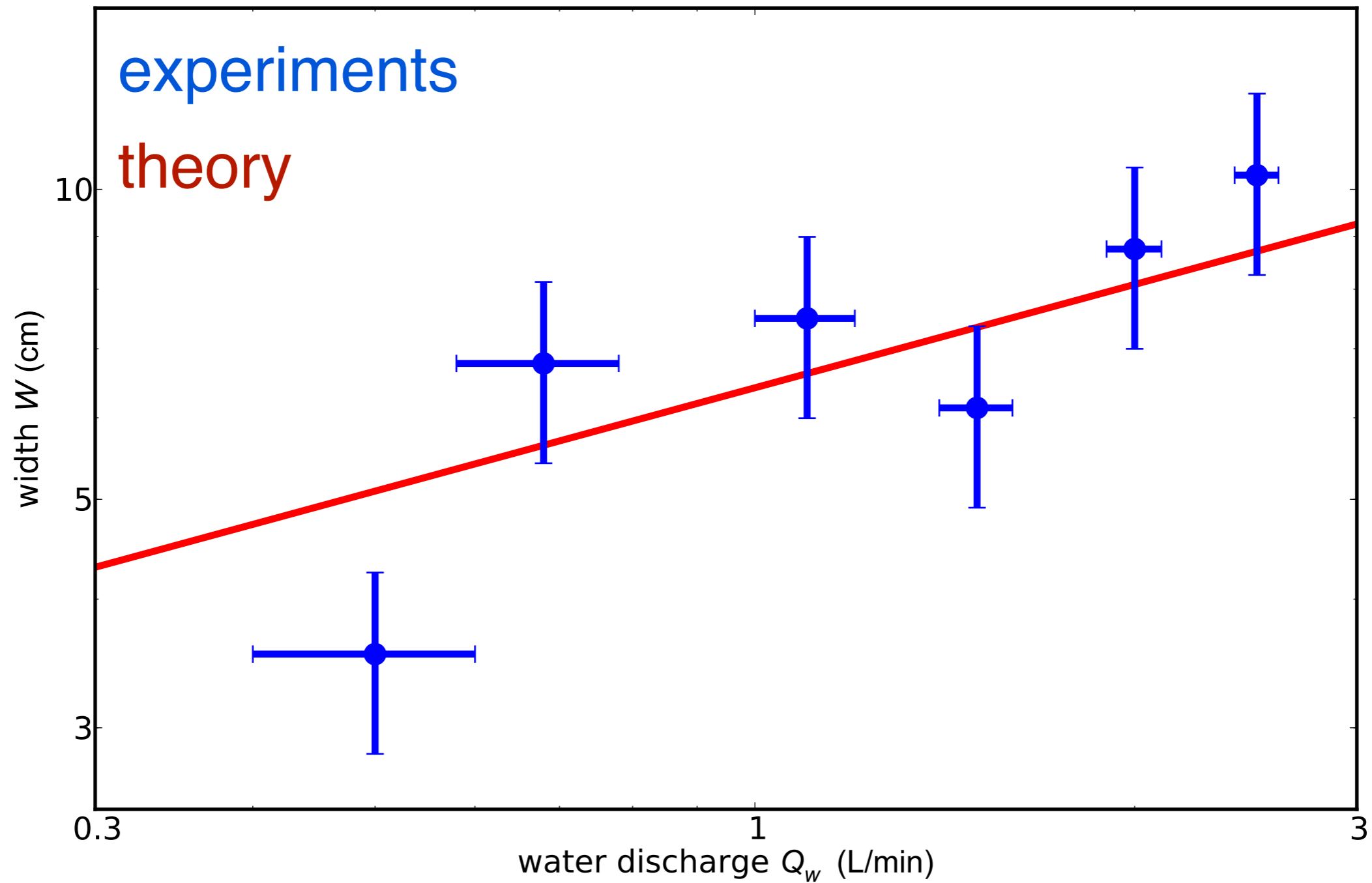
The channel aspect ratio is constant.

Comparison with experimental results



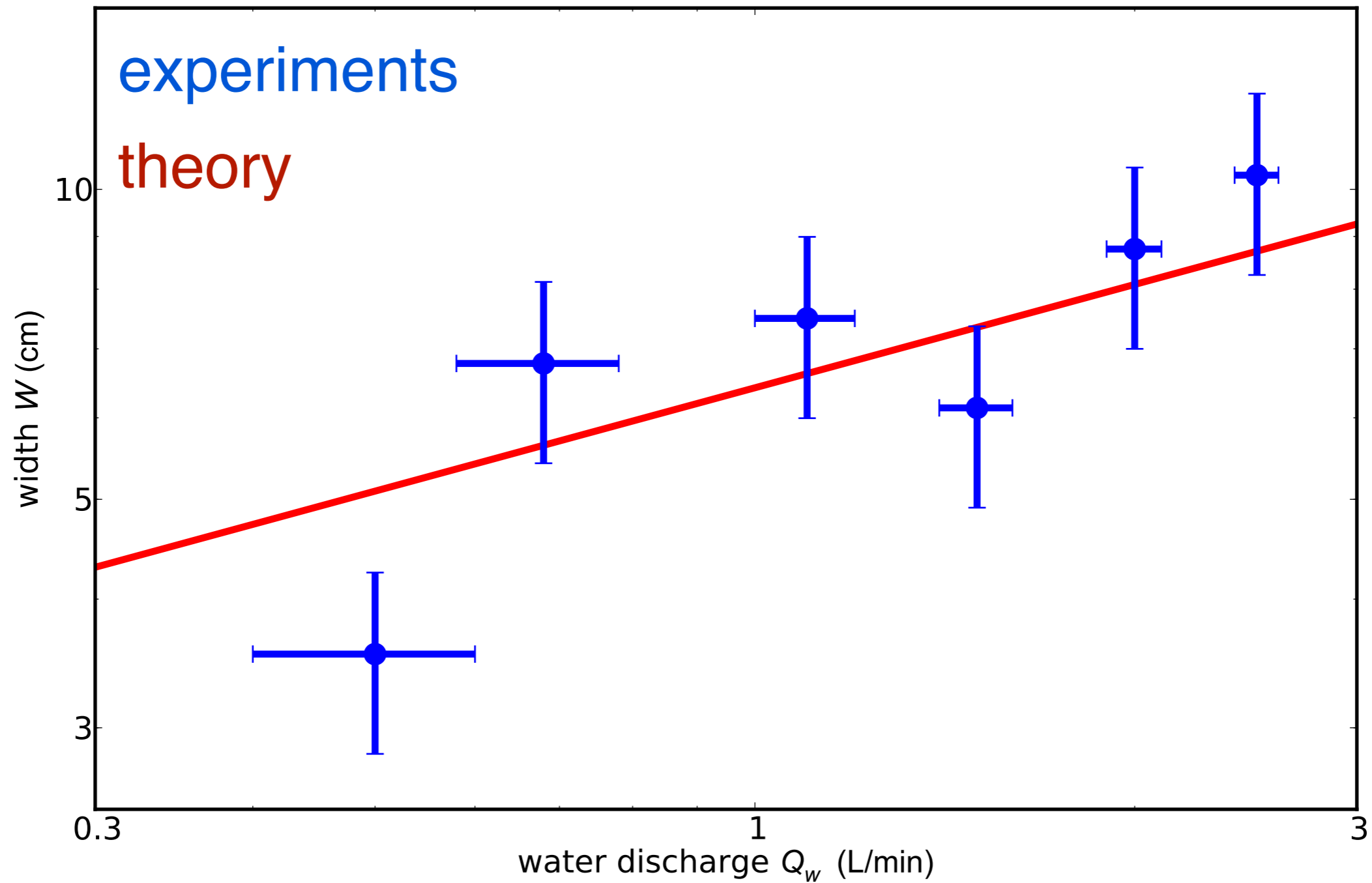
$$\frac{W}{\langle D \rangle} = \frac{\pi^2}{2\mu} \approx 7$$

Comparison with experimental results



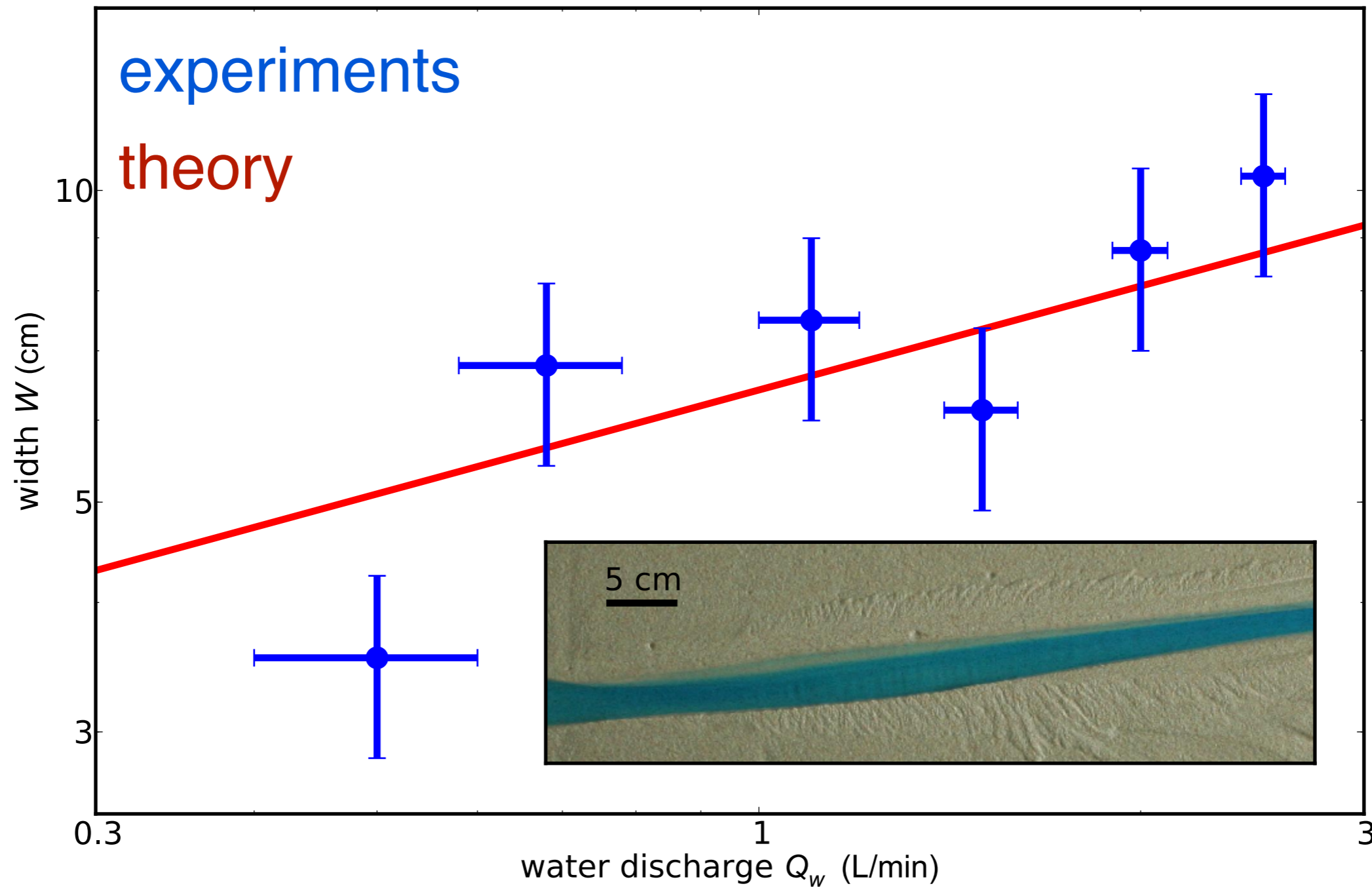
$$\text{width} = \frac{\pi}{\mu^{2/3}} \left(\frac{9 \nu \rho_f}{4 g \theta_t (\rho_s - \rho_f) d_s} \right)^{1/3} \times \text{discharge}^{1/3}$$

Comparison with experimental results



$$\text{width} = \frac{\pi}{\mu^{2/3}} \left(\frac{9 \nu \rho_f}{4 g \theta_t (\rho_s - \rho_f) d_s} \right)^{1/3} \times \text{discharge}^{1/3}$$

Comparison with experimental results



$$\text{width} = \frac{\pi}{\mu^{2/3}} \left(\frac{9 \nu \rho_f}{4 g \theta_t (\rho_s - \rho_f) d_s} \right)^{1/3} \times \text{discharge}^{1/3}$$

Comparison with field sandy rivers

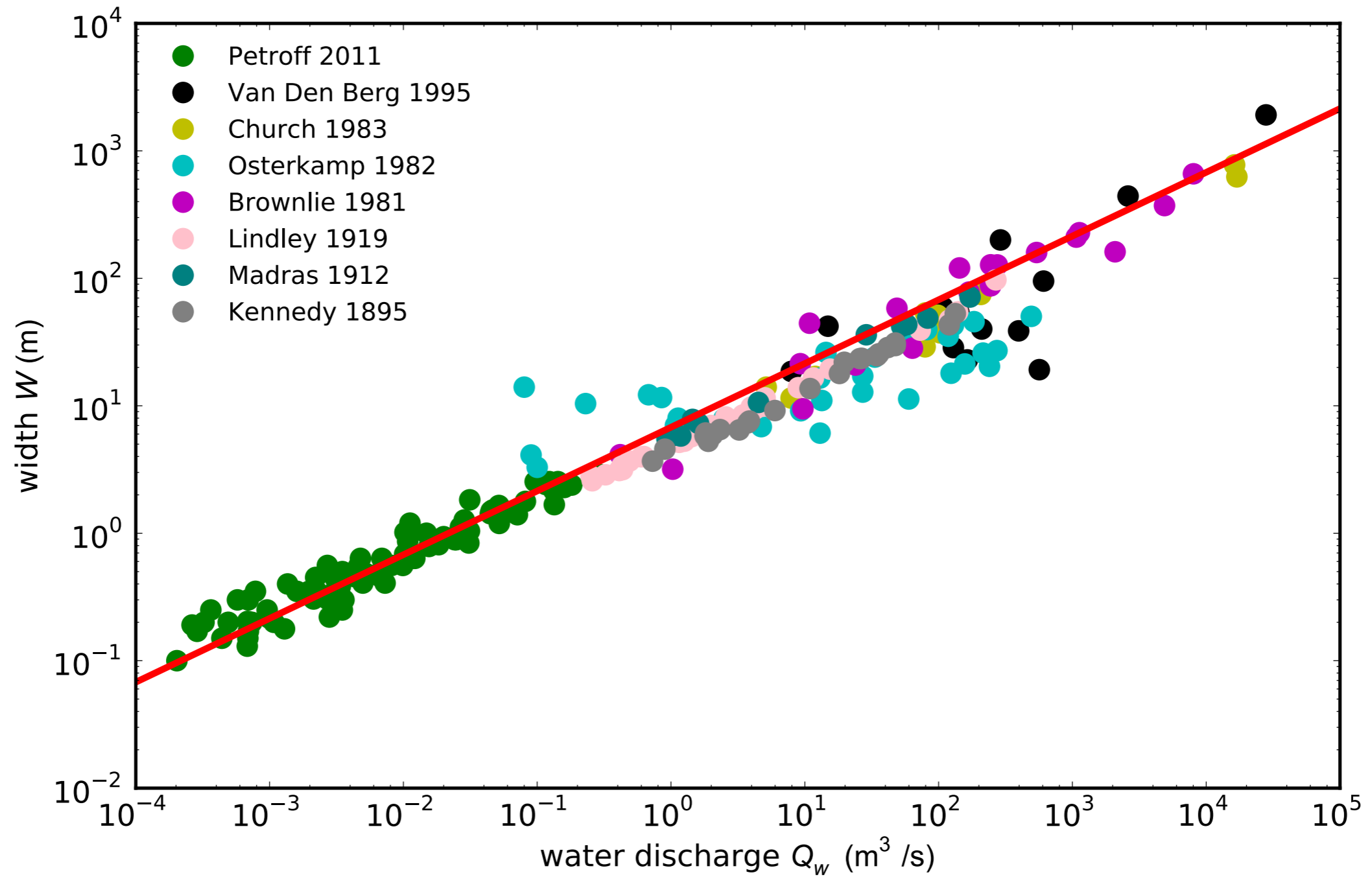
$$\text{width} = \frac{\pi}{1.75} \left(\frac{\mu^2 C_f}{gL} \right)^{1/4} \times \text{discharge}^{1/2}$$

turbulent friction coef.

turbulent flow

[Glover & Florey, 1951]

Comparison with field sandy rivers



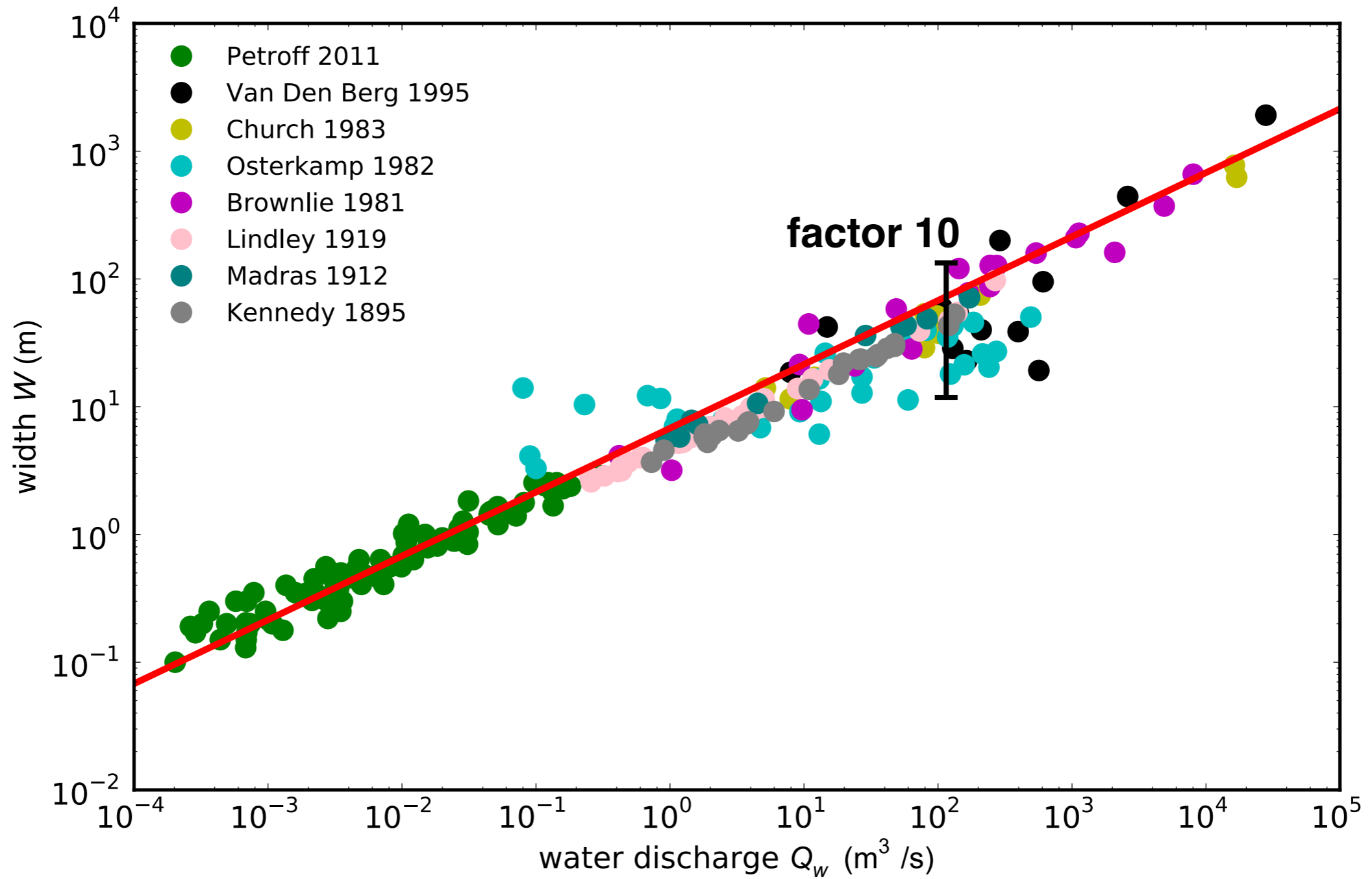
turbulent friction coef.

$$\text{width} = \frac{\pi}{1.75} \left(\frac{\mu^2 C_f}{gL} \right)^{1/4} \times \text{discharge}^{1/2}$$

turbulent flow

[Glover & Florey, 1951]

Comparison with field sandy rivers



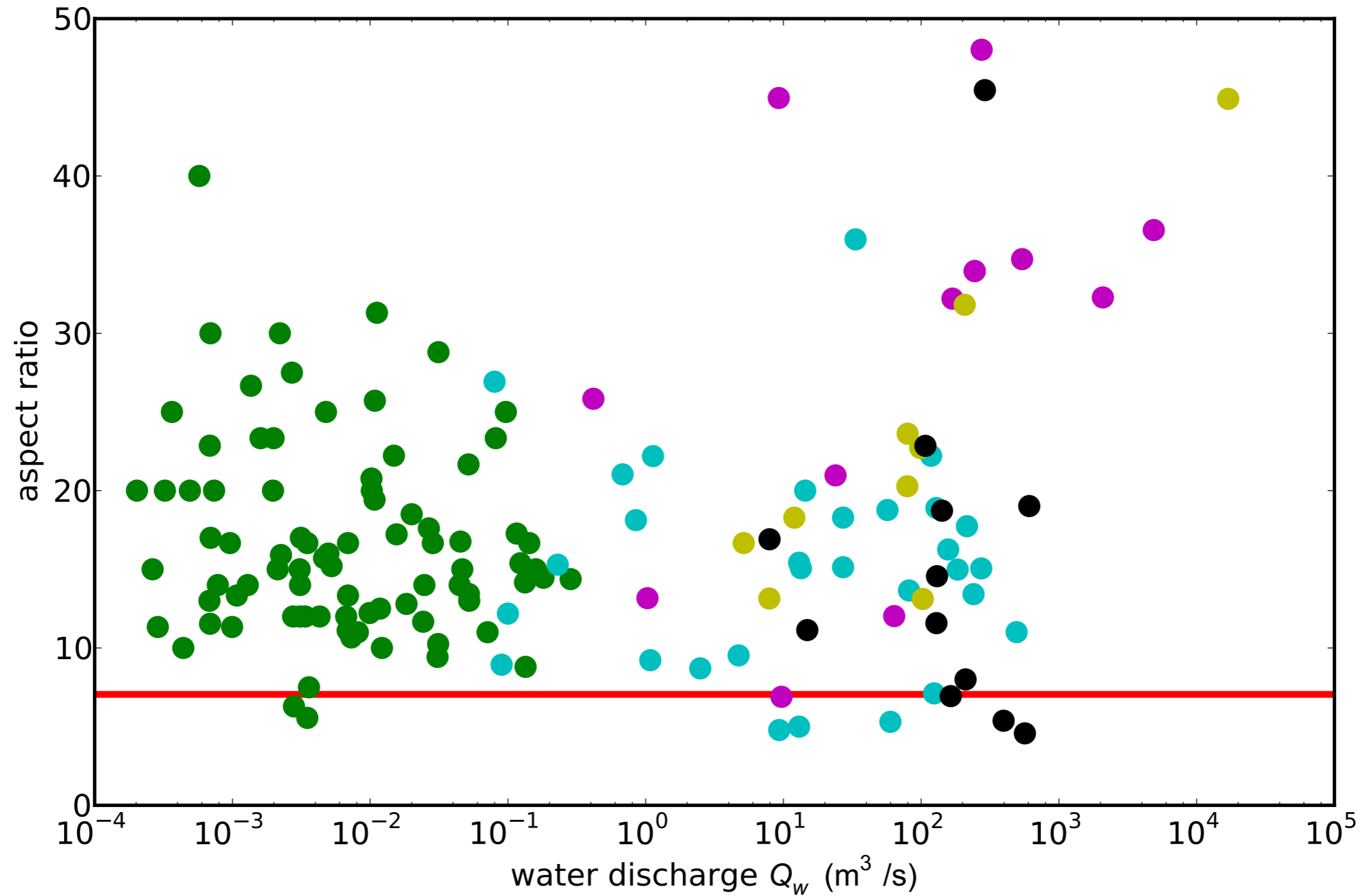
turbulent friction coef.

$$\text{width} = \frac{\pi}{1.75} \left(\frac{\mu^2 C_f}{gL} \right)^{1/4} \times \text{discharge}^{1/2}$$

turbulent flow

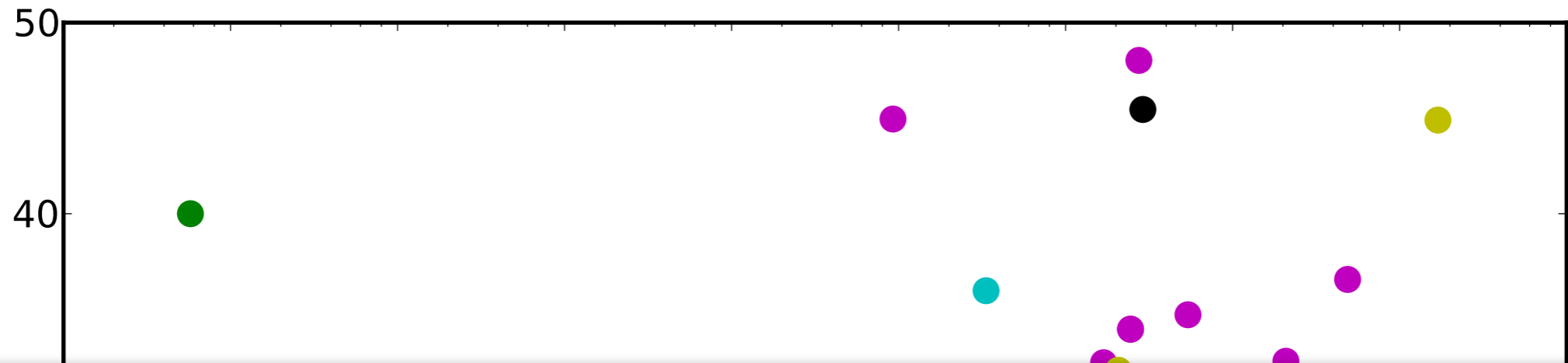
[Glover & Florey, 1951]

Comparison with field sandy rivers

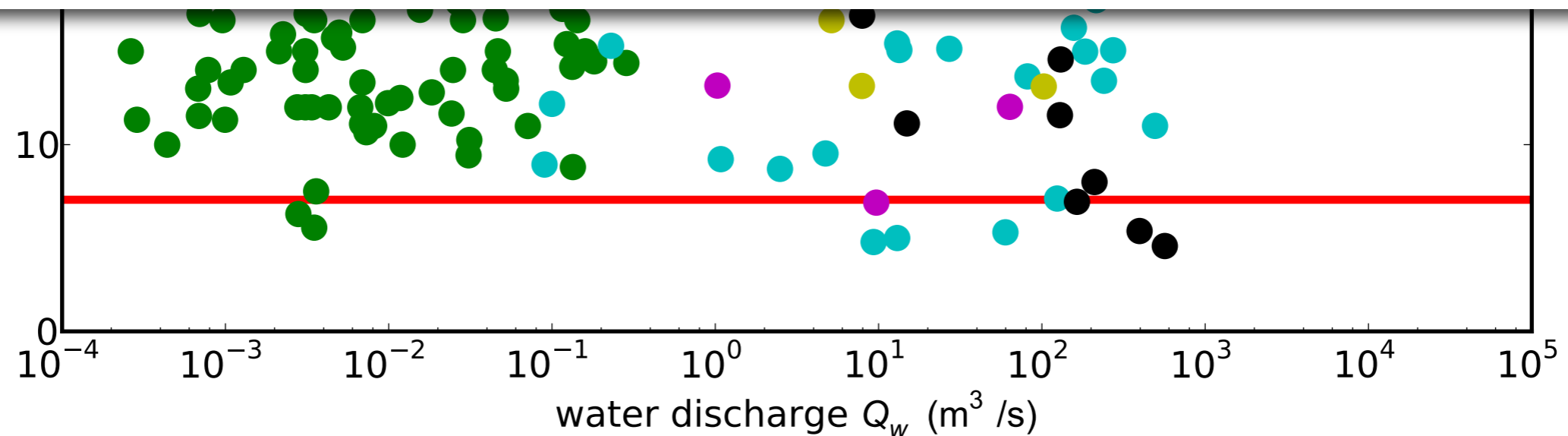


$$\frac{W}{\langle D \rangle} = \frac{\pi^2}{2\mu} \approx 7$$

Comparison with field sandy rivers



The zero transport model captures the 1/2 exponent of the width vs discharge relationship but underestimates the aspect ratio of natural rivers!

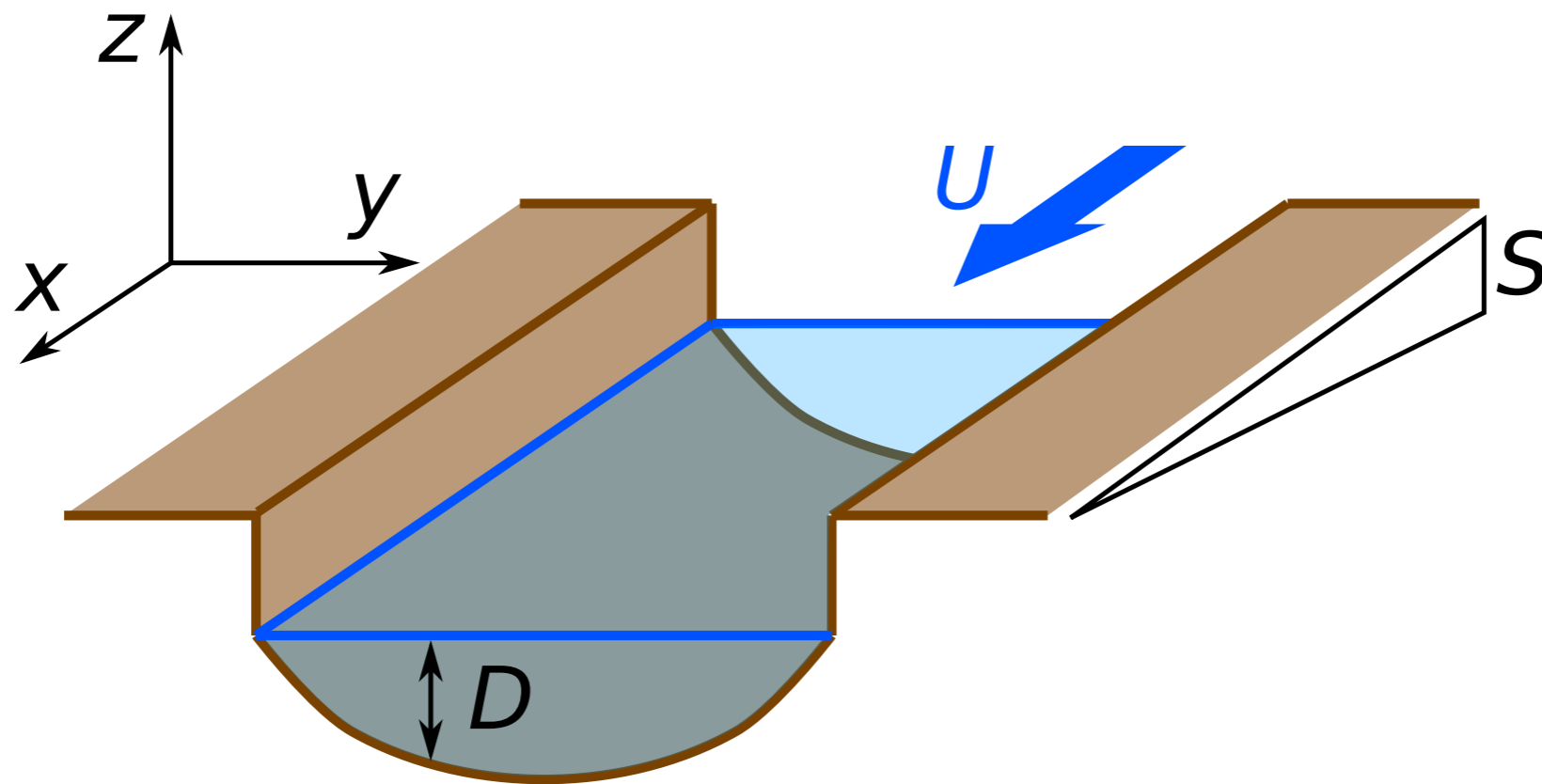


$$\frac{W}{\langle D \rangle} = \frac{\pi^2}{2\mu} \approx 7$$

Rivers do transport sediment !

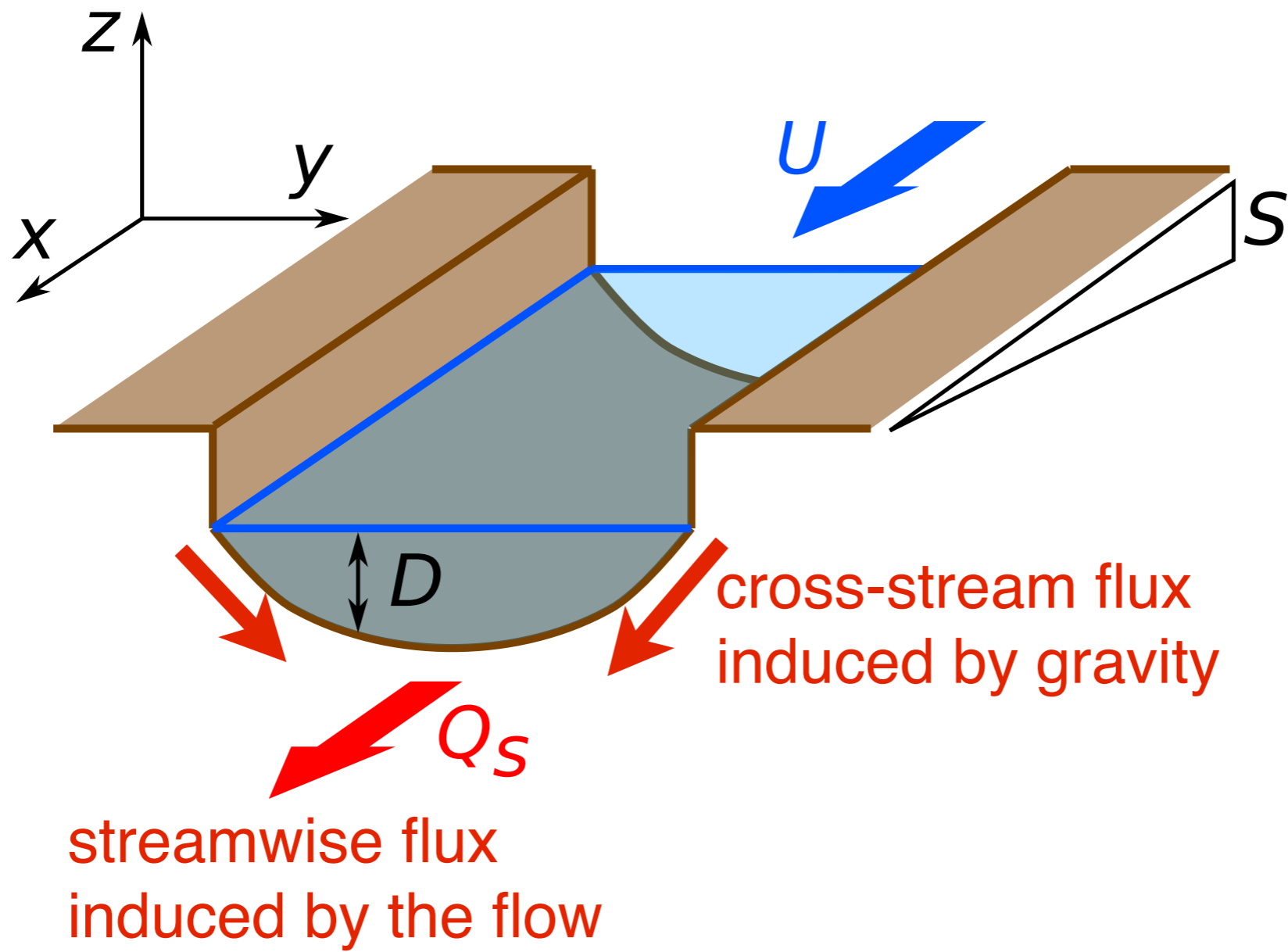


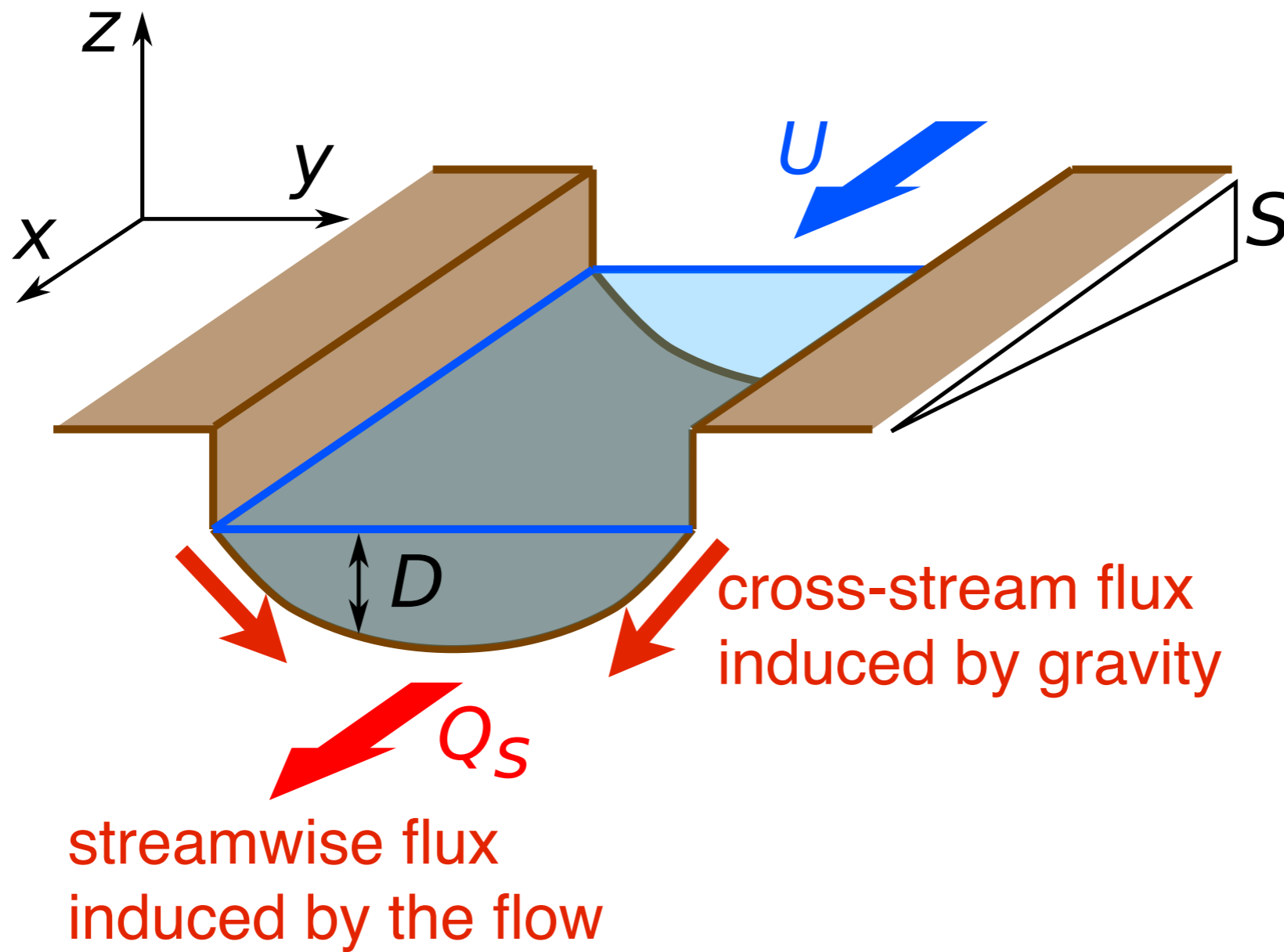
Urümqi He, chinese Tian-Shan



Q_s

streamwise flux
induced by the flow

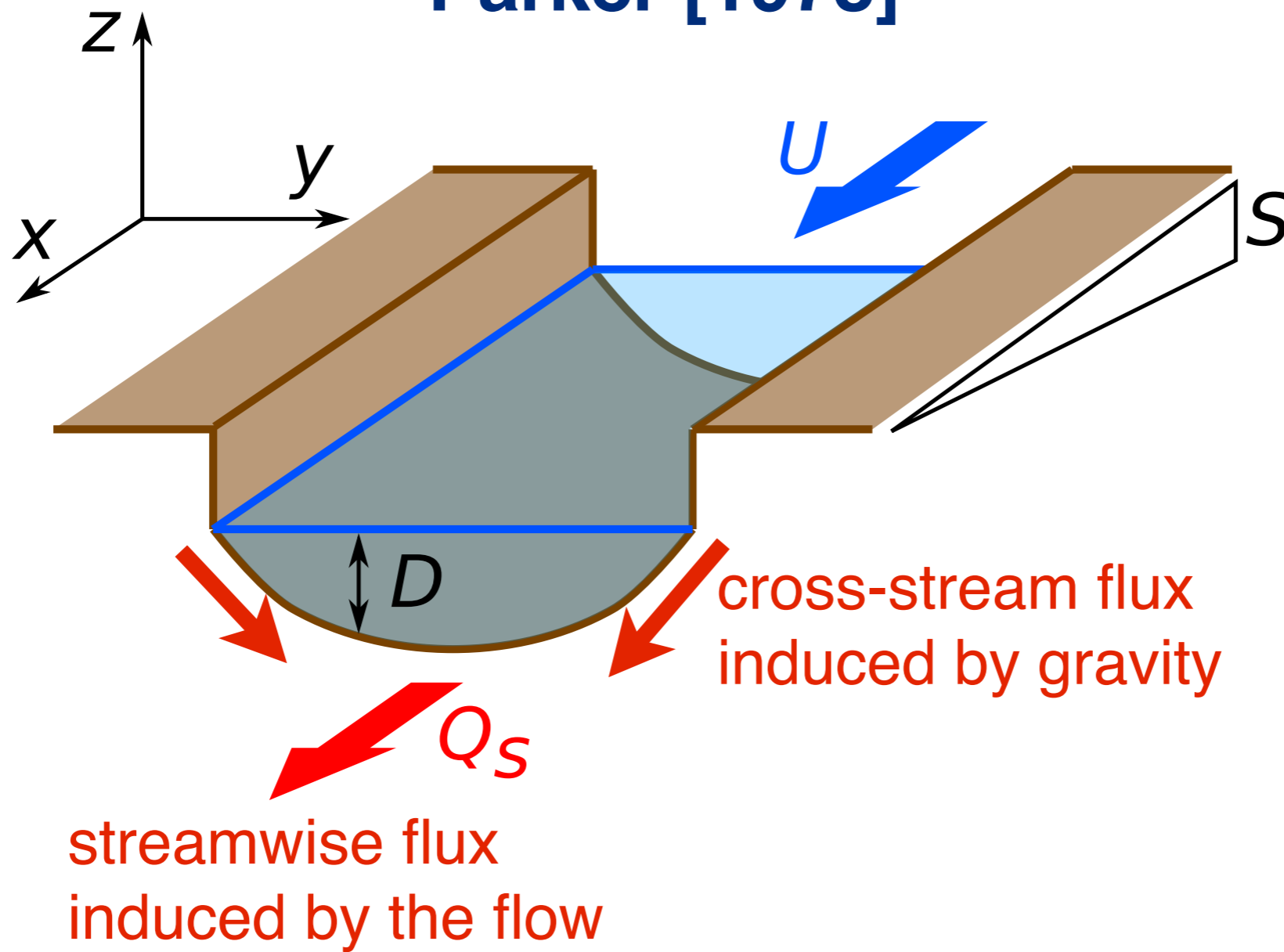




transverse gravity flux \rightarrow bank erosion

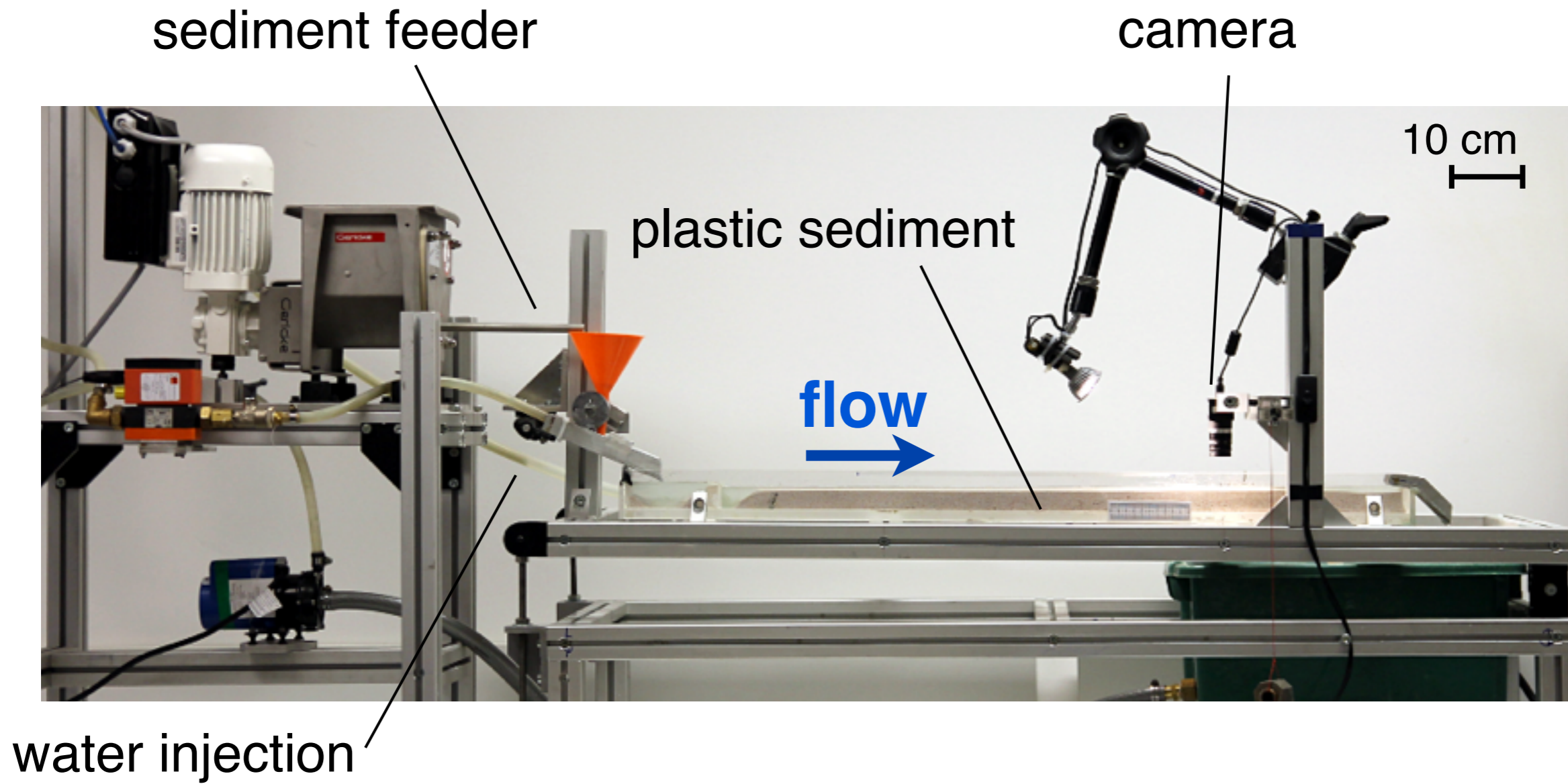
'stable channel paradox'

Parker [1978]

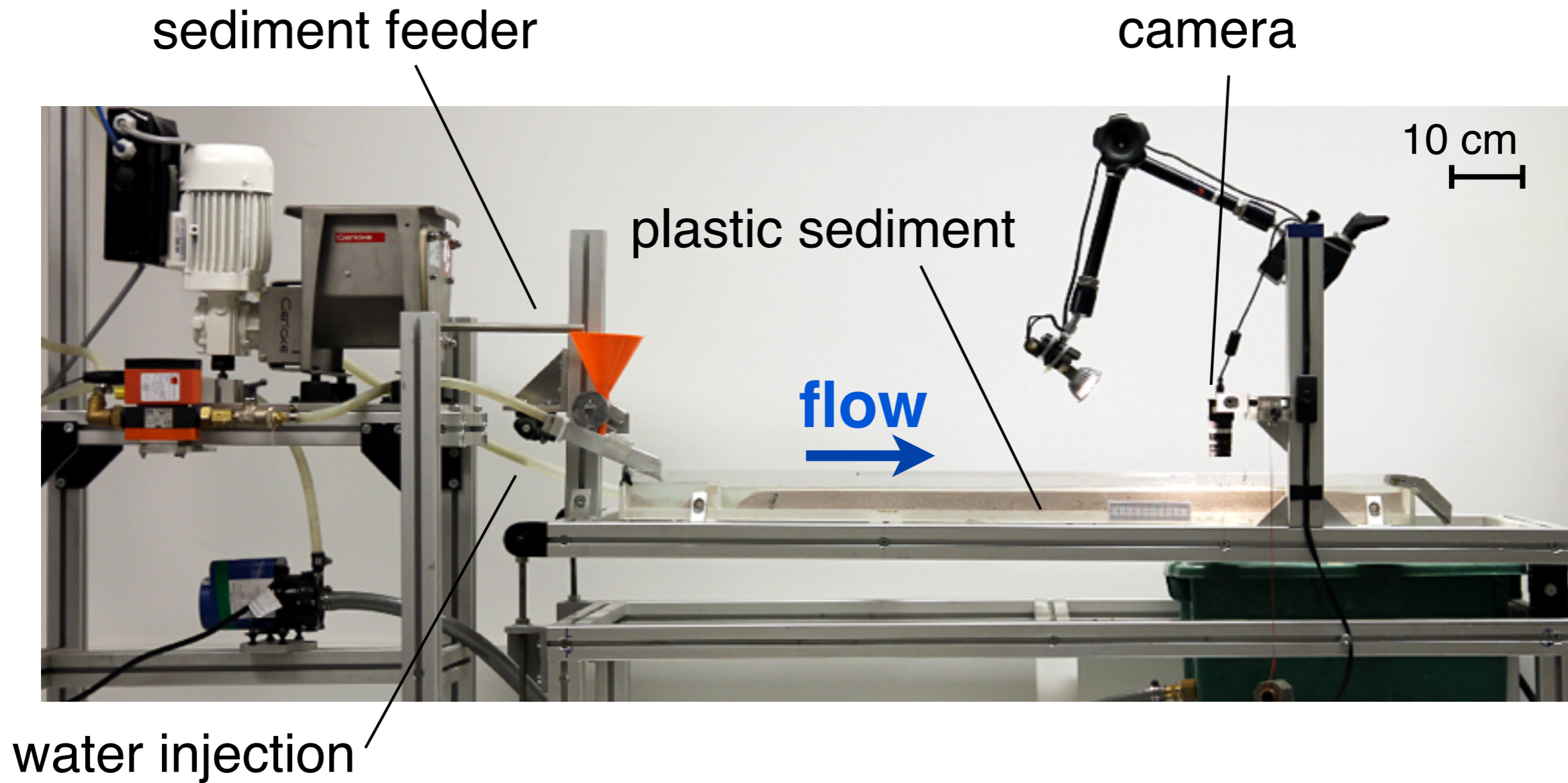


A river transporting sediment cannot be stable unless we find an effect which compensates for the gravity flux.

Experimental channel



Experimental channel

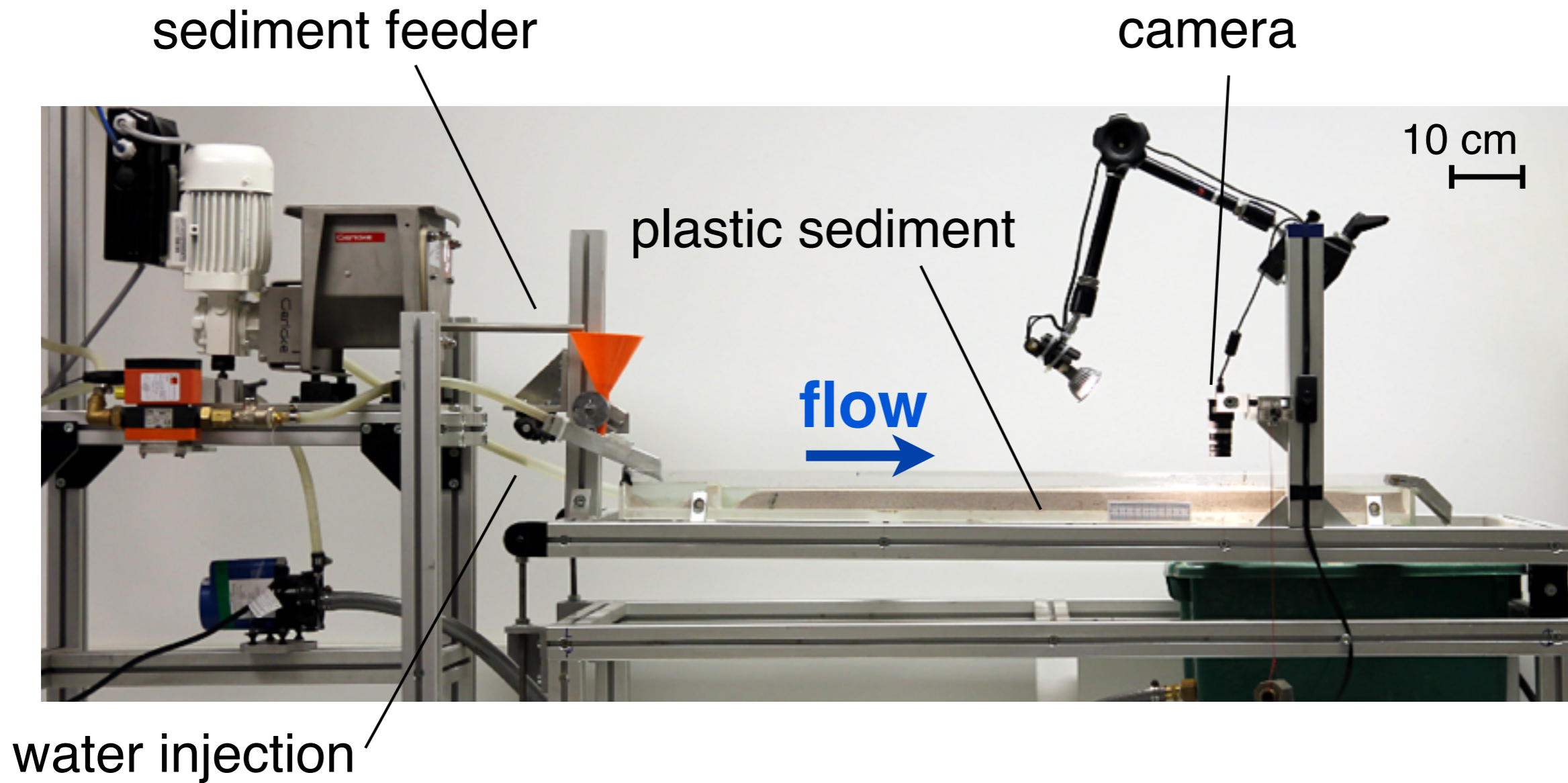


constant flow discharge
constant sediment discharge



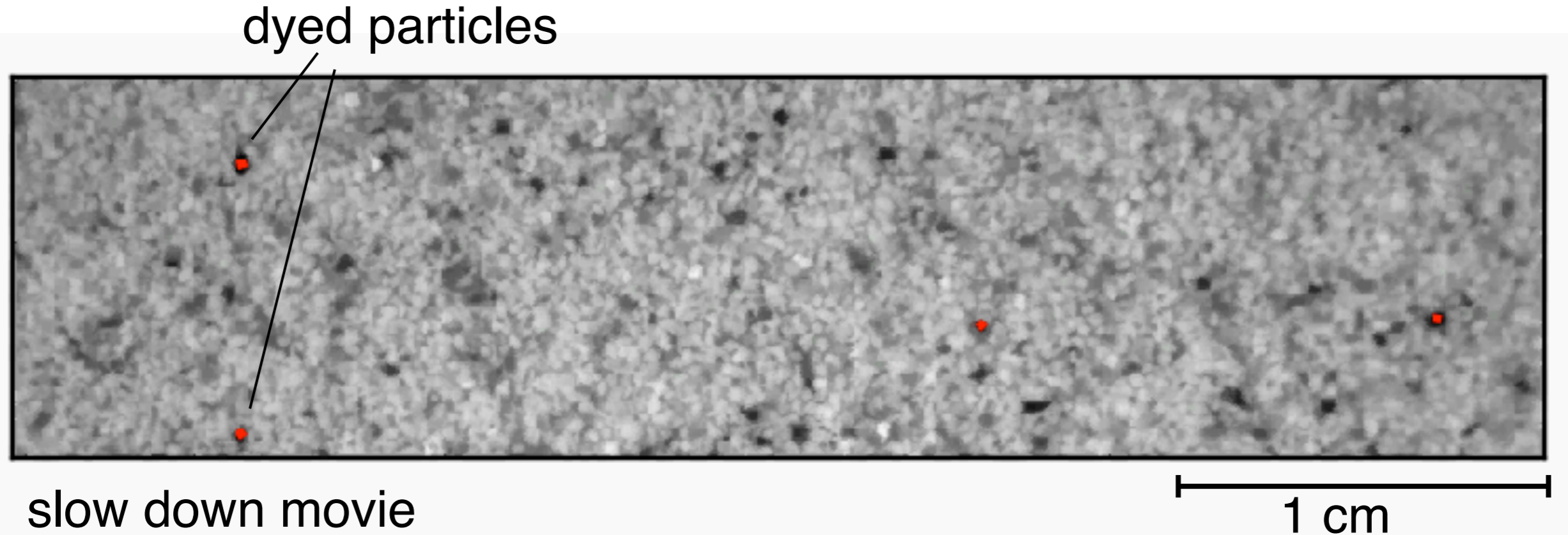
steady-state

Experimental channel



- The sediment bed is flat !
- Close to the entrainment threshold !

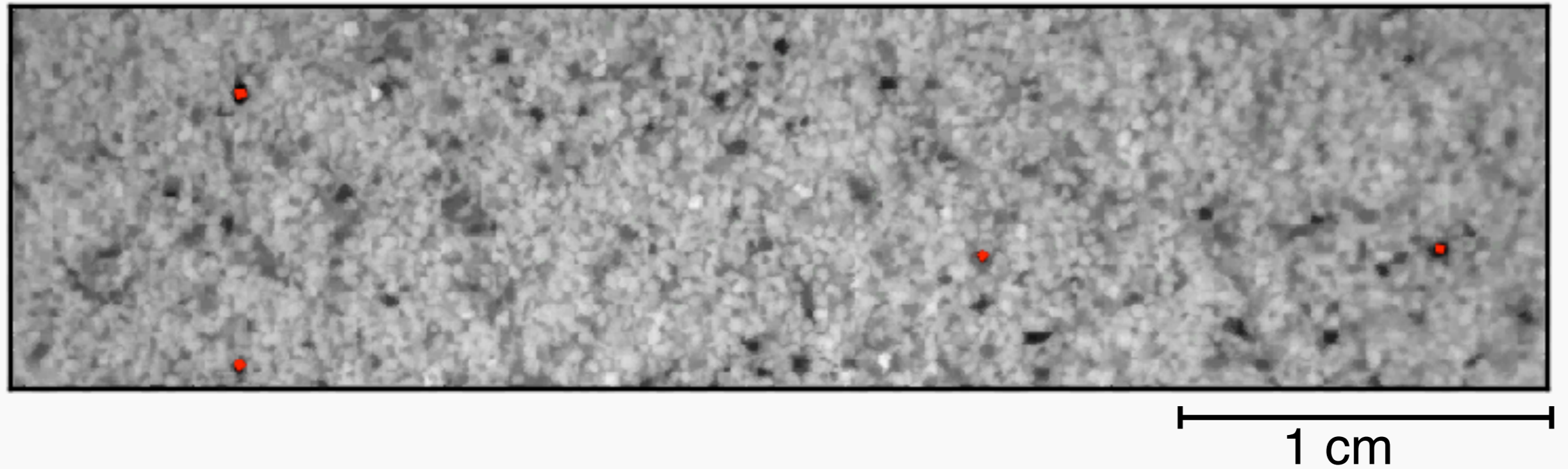
Particle trajectories



Bedload transport : rolling, jumping & sliding

The motion of an individual particle is stochastic!

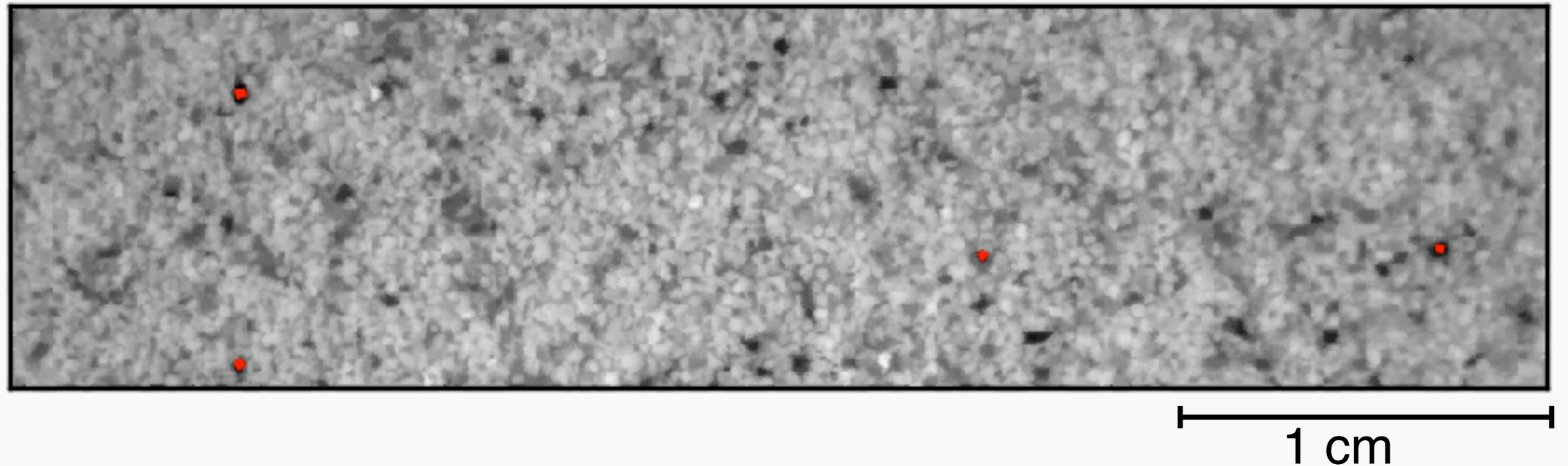
Particle trajectories



$$q_b = n V$$

transport rate surface concentration of moving particles average velocity of the particles

Particle trajectories



$$q_b = nV$$

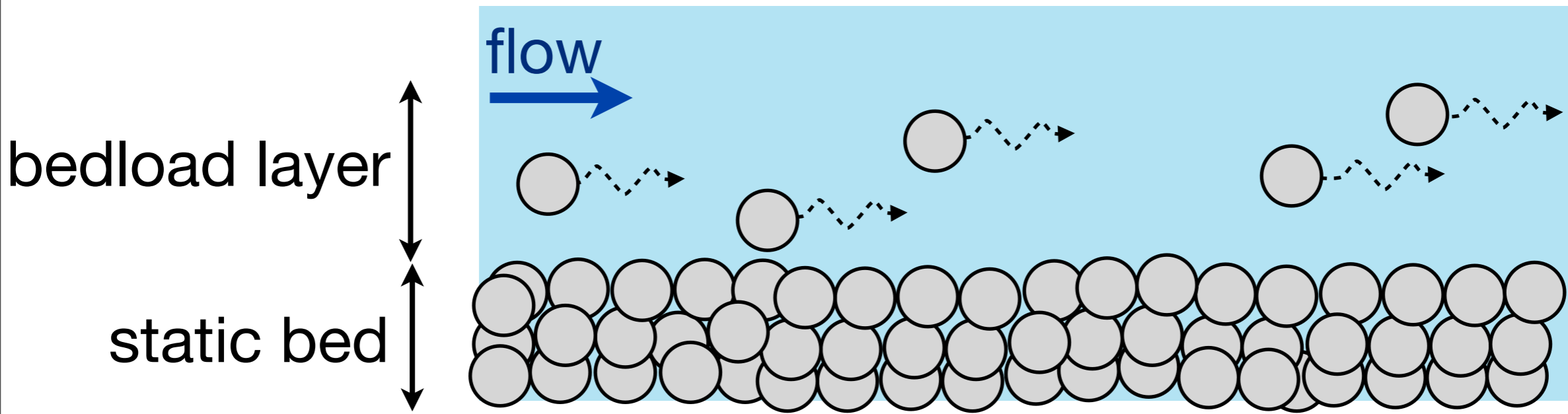
transport rate

surface concentration of moving particles

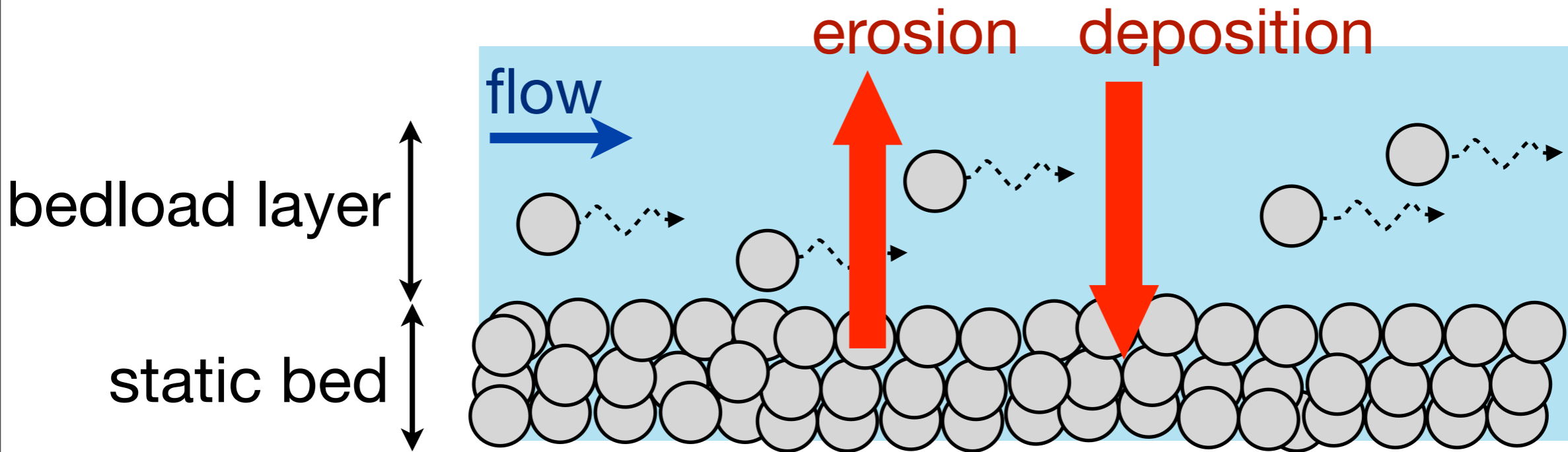
average velocity of the particles

Charru et al. [2004]
Lajeunesse et al. [2010]
Furbish et al. [2012]
Seizilles et al. [in press]

Erosion-deposition model



Erosion-deposition model



steady-state
uniform flow



$$n \propto \frac{(\theta - \theta_t)}{d_s^2}$$

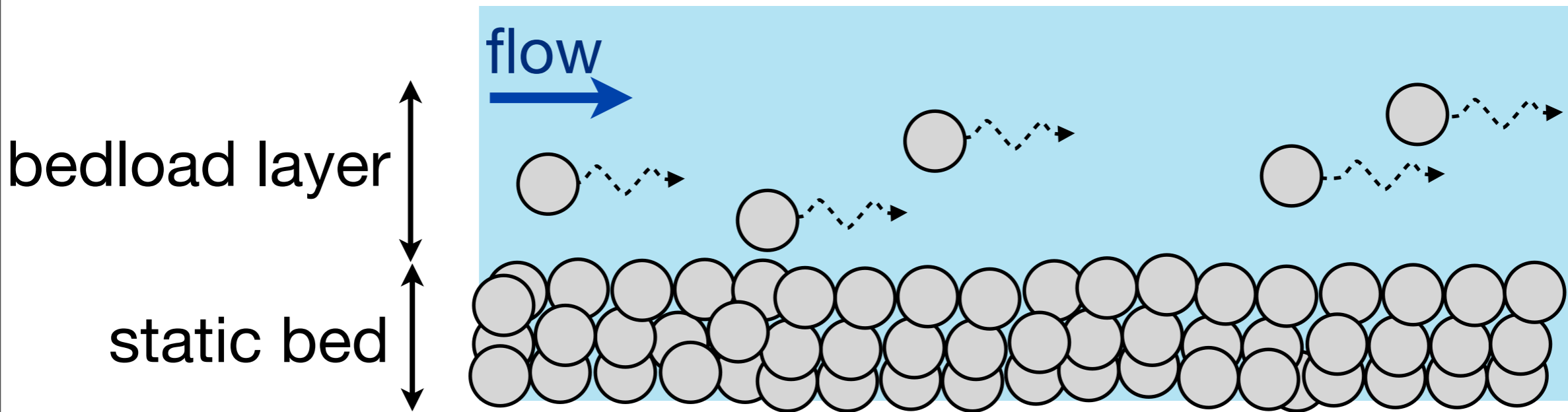
concentration of moving particles

grain size

excess Shields stress

[Charru et al., 2004;
Lajeunesse et al., 2010]

Erosion-deposition model



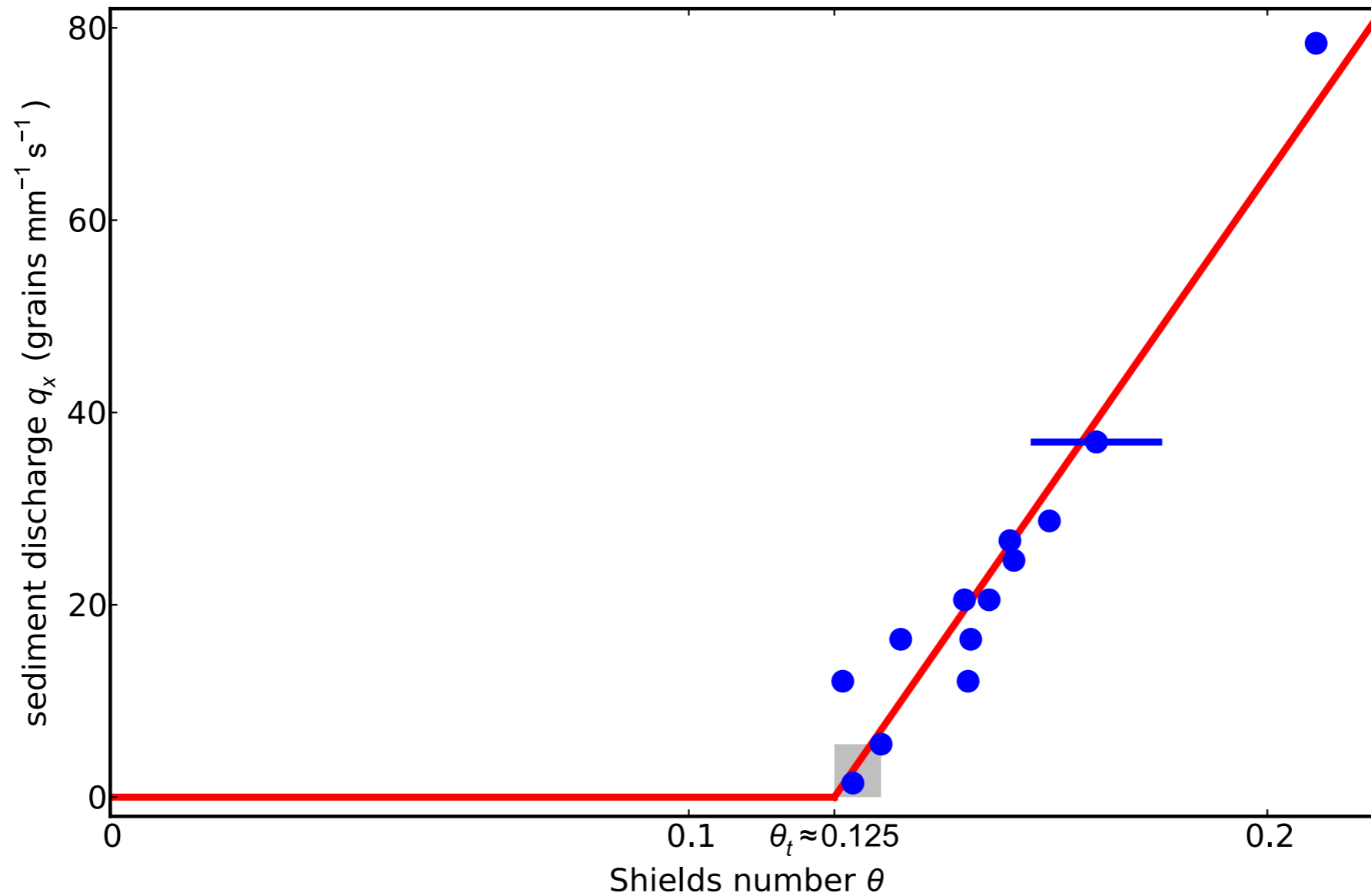
average particle velocity

grain size shear rate stoke velocity

$$V_x \sim d_s \frac{\partial u}{\partial z} \sim V_s \theta$$

[Charru et al., 2004;
Seizilles et al., in press]

Erosion-deposition model



At leading order

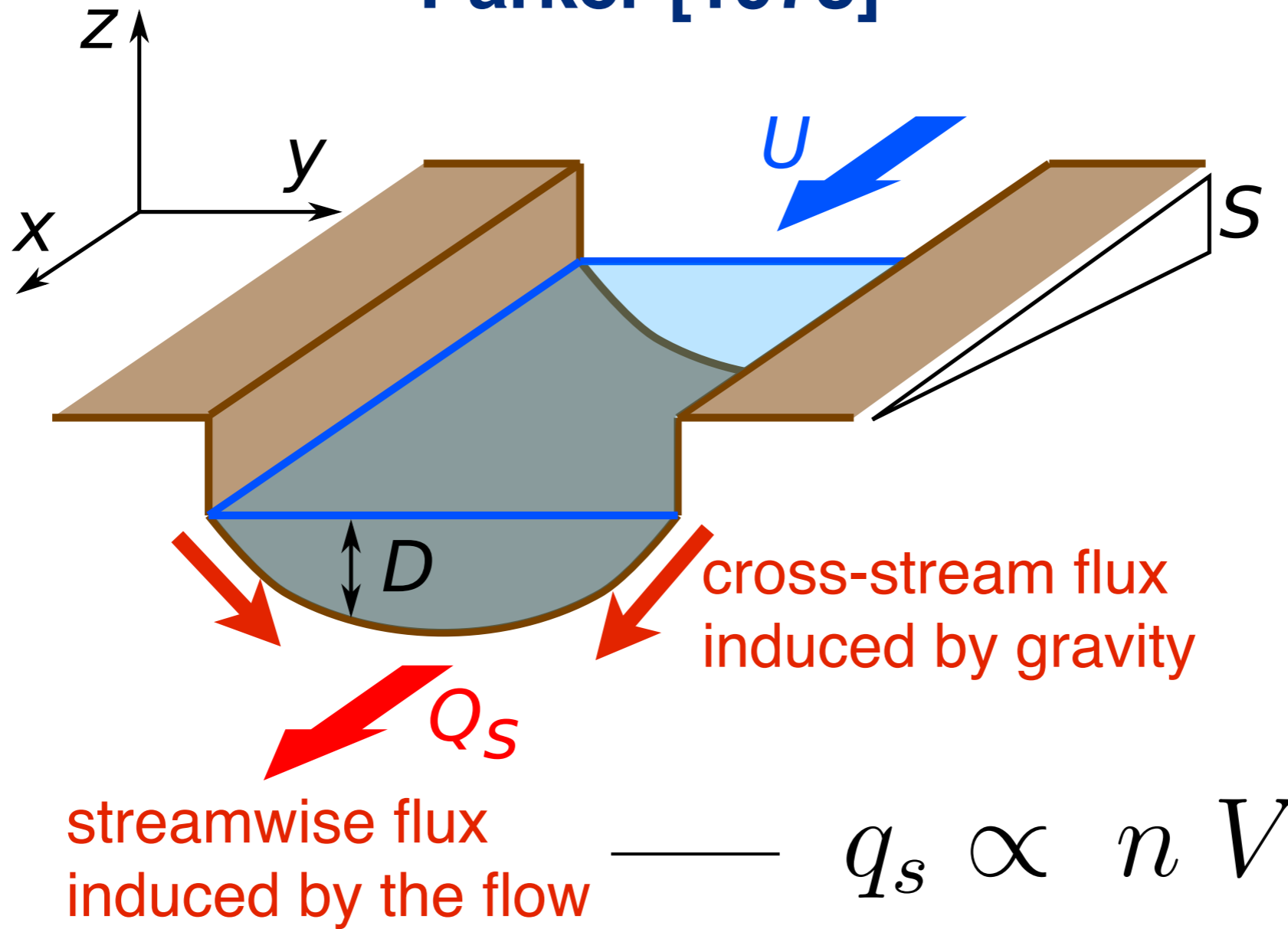
$$q_s \propto n V_x \propto \frac{\theta_t V_s}{d_s^2} (\theta - \theta_t)$$

excess Shields stress

[Charru et al., 2004;
Seizilles et al., in press]

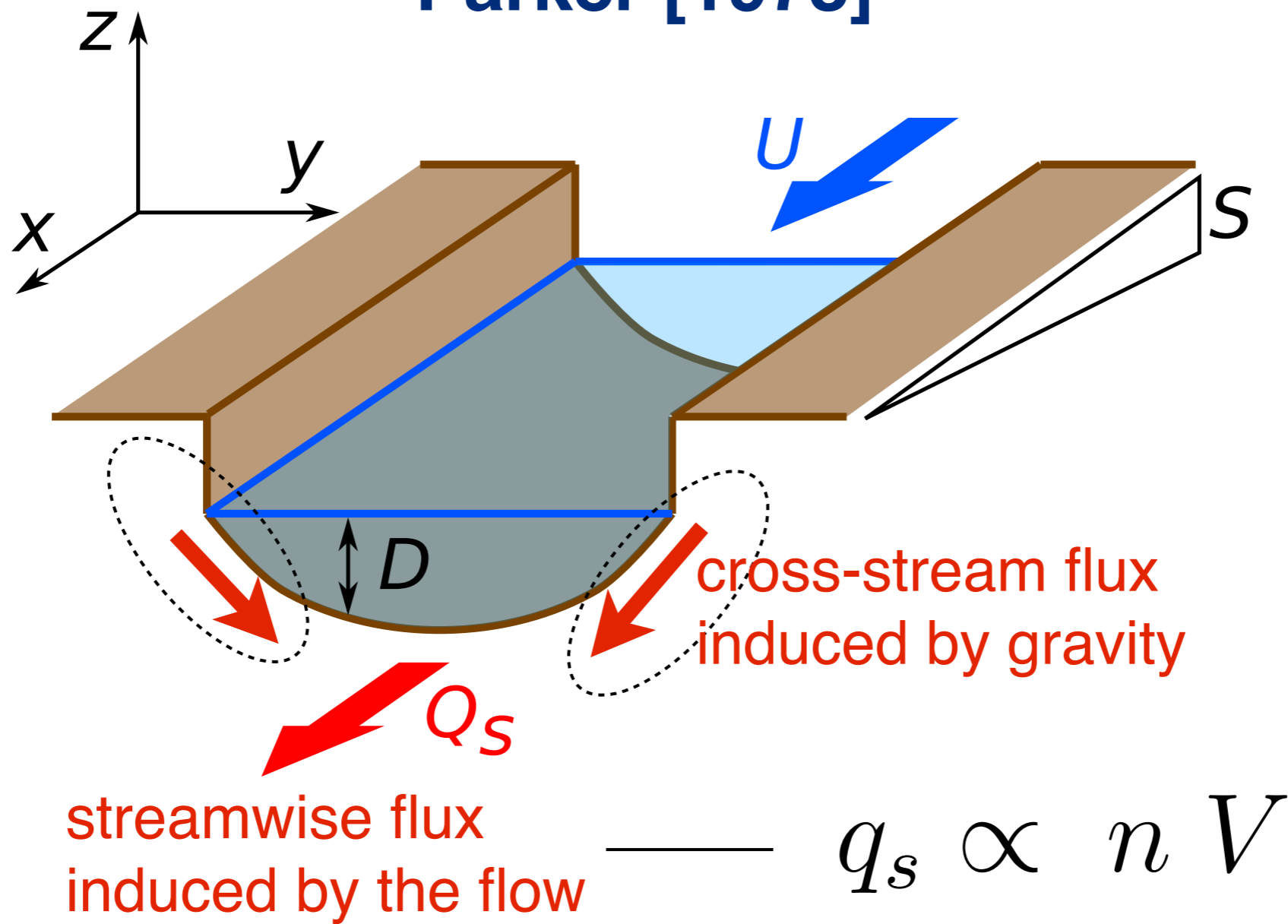
'stable channel paradox'

Parker [1978]



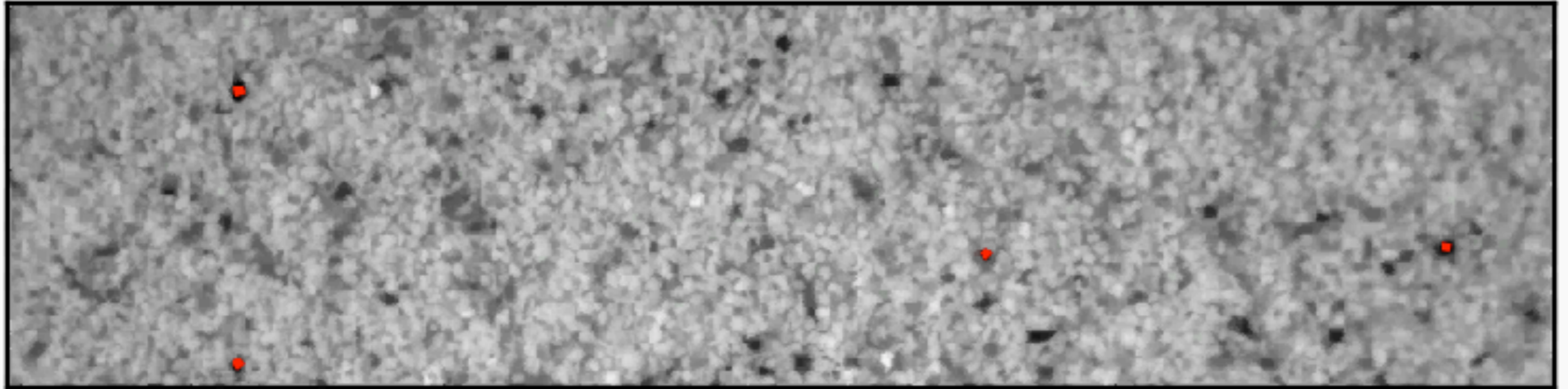
'stable channel paradox'

Parker [1978]



What opposes the cross-stream gravity-induced flux ?

Bedload transport



view from above
slow down movie

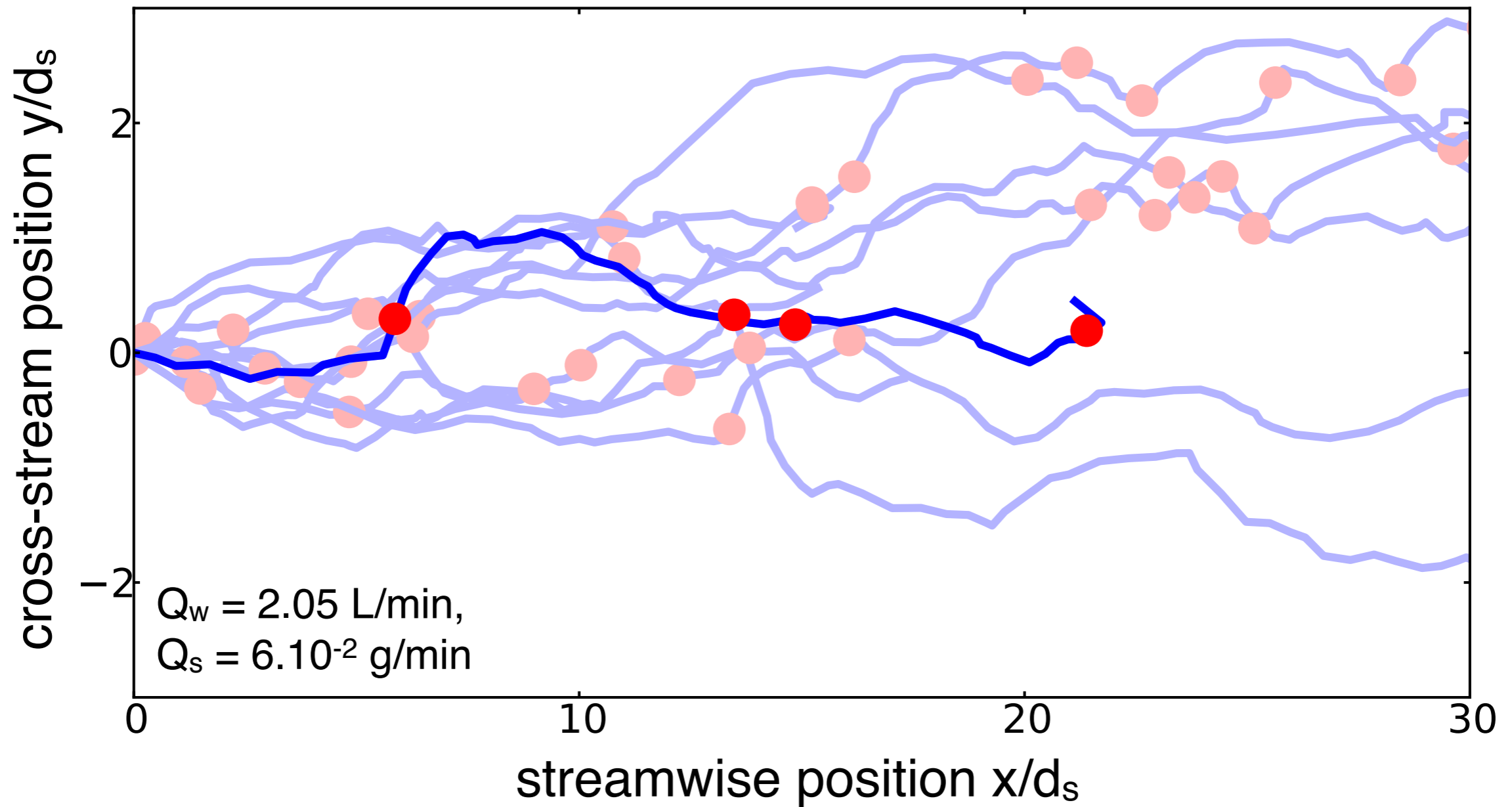
Trajectories are not straight lines!

bed roughness → deviations along the cross stream direction

Samson et al. [1998], Lajeunesse et al. [2010], Roseberry et al. [2012]

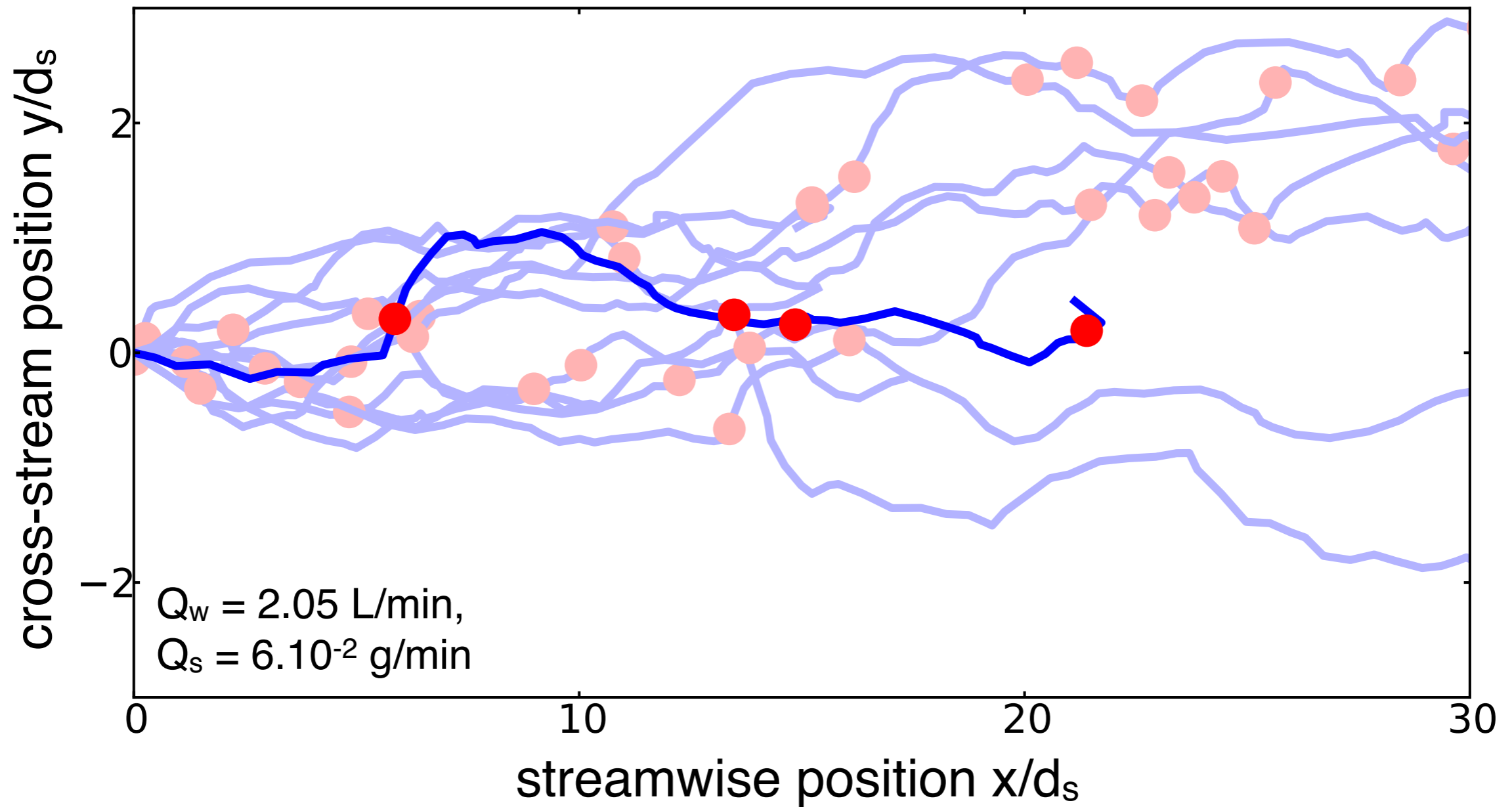
Particle trajectories

translated so that their initial starting point coincide



Particle trajectories

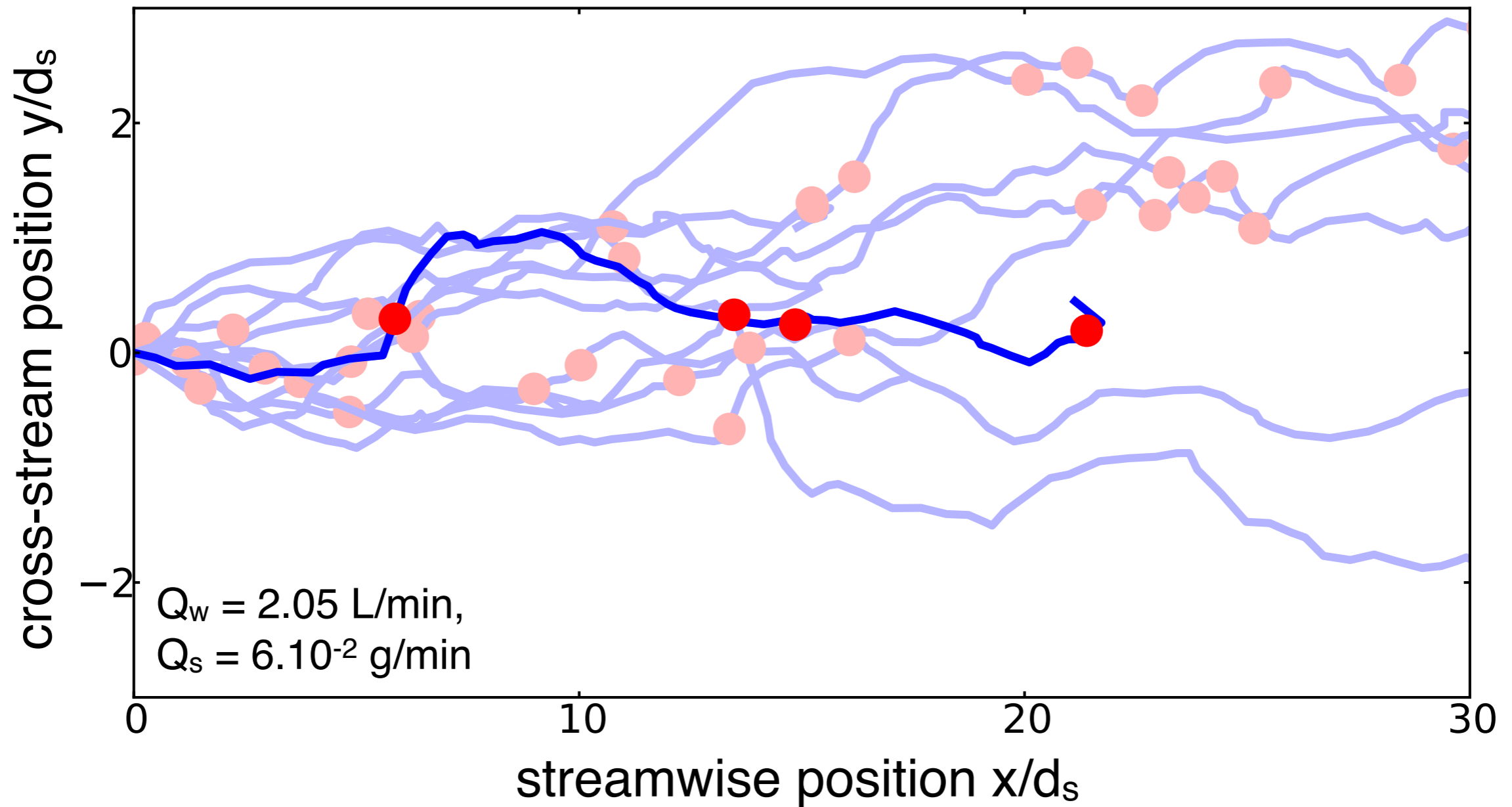
translated so that their initial starting point coincide



spreading along the cross-stream direction

Particle trajectories

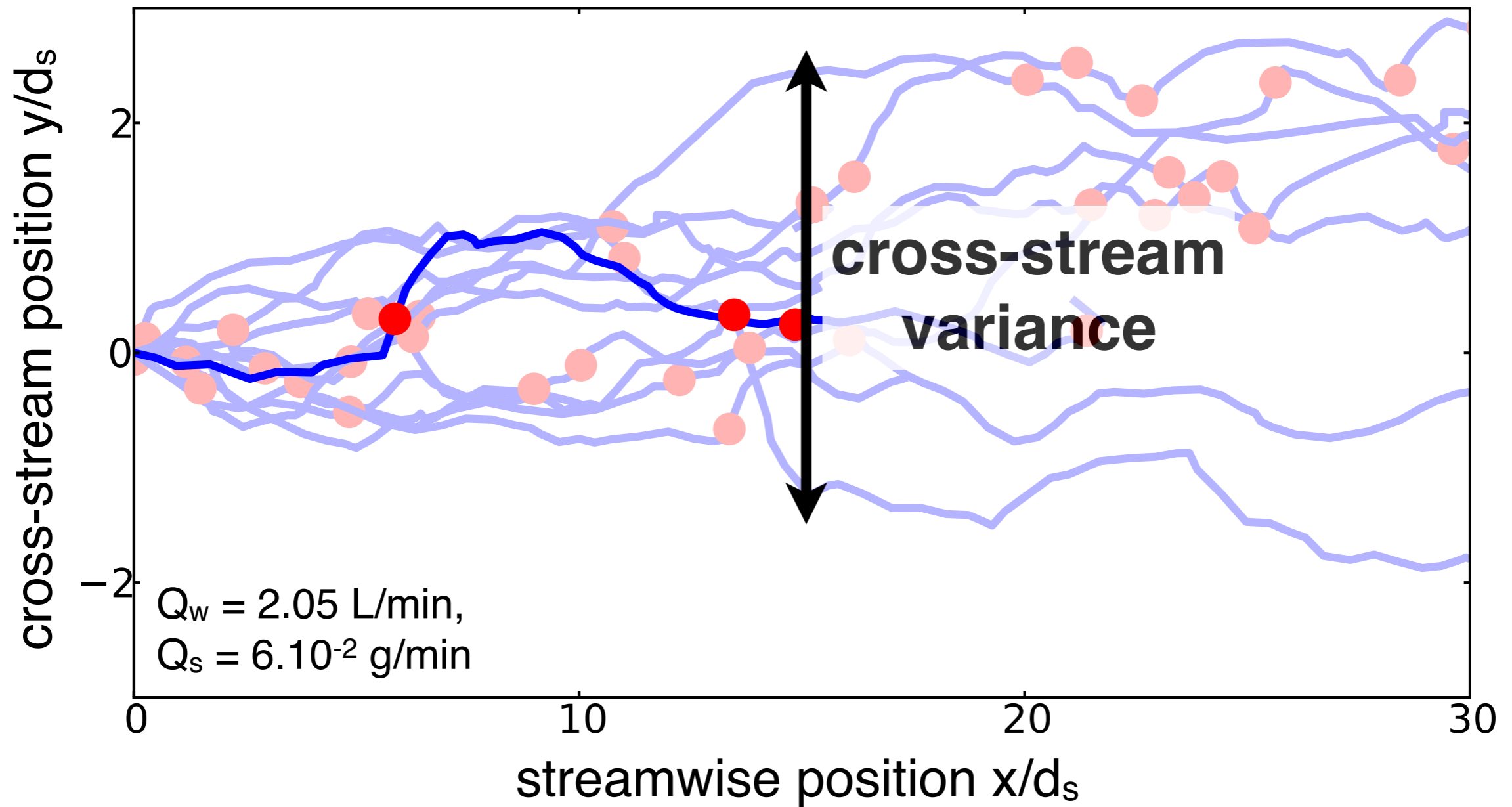
translated so that their initial starting point coincide



spreading along the cross-stream direction
random walk in the cross-stream direction ?

Particle trajectories

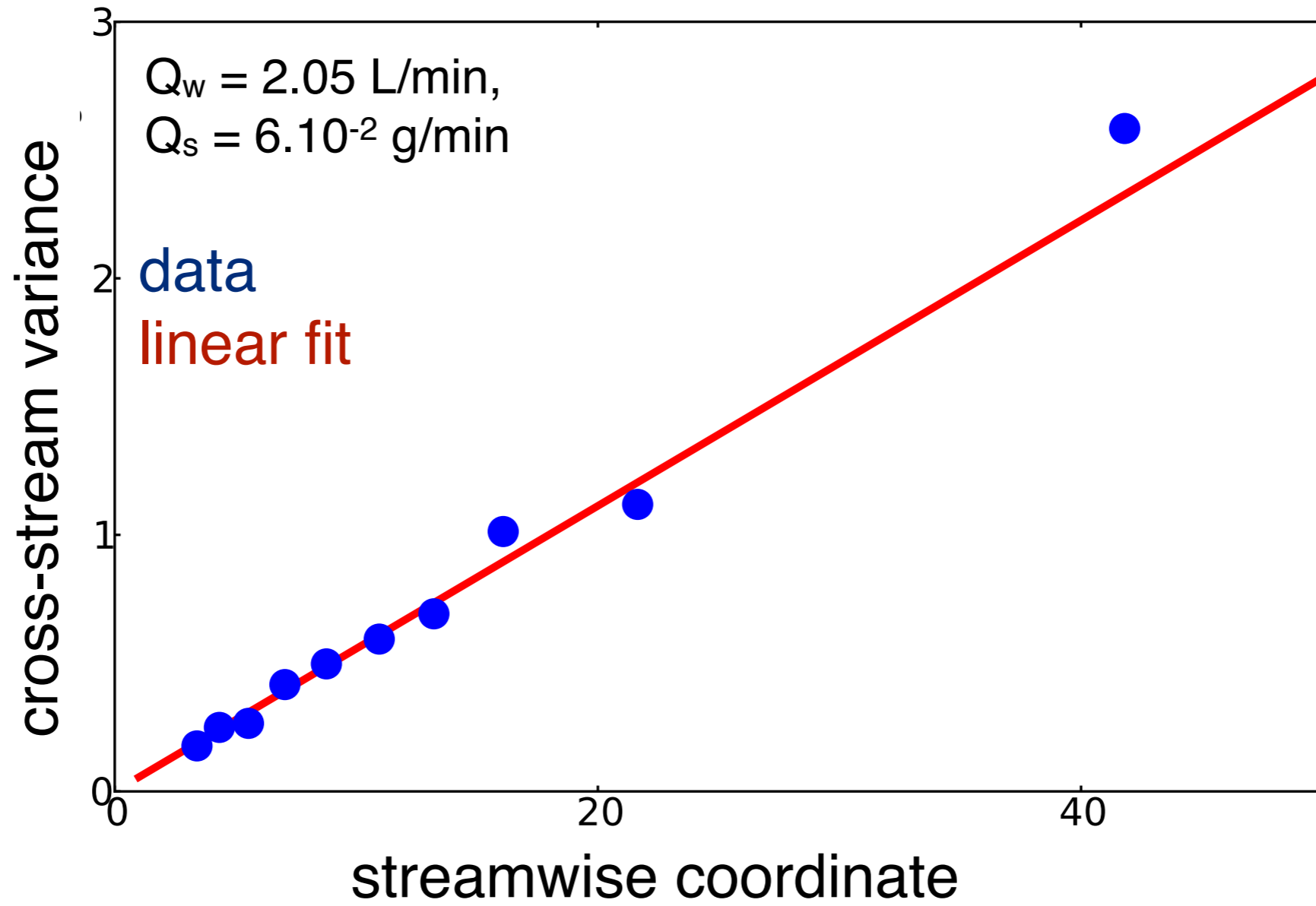
translated so that their initial starting point coincide



spreading along the cross-stream direction
random walk in the cross-stream direction ?

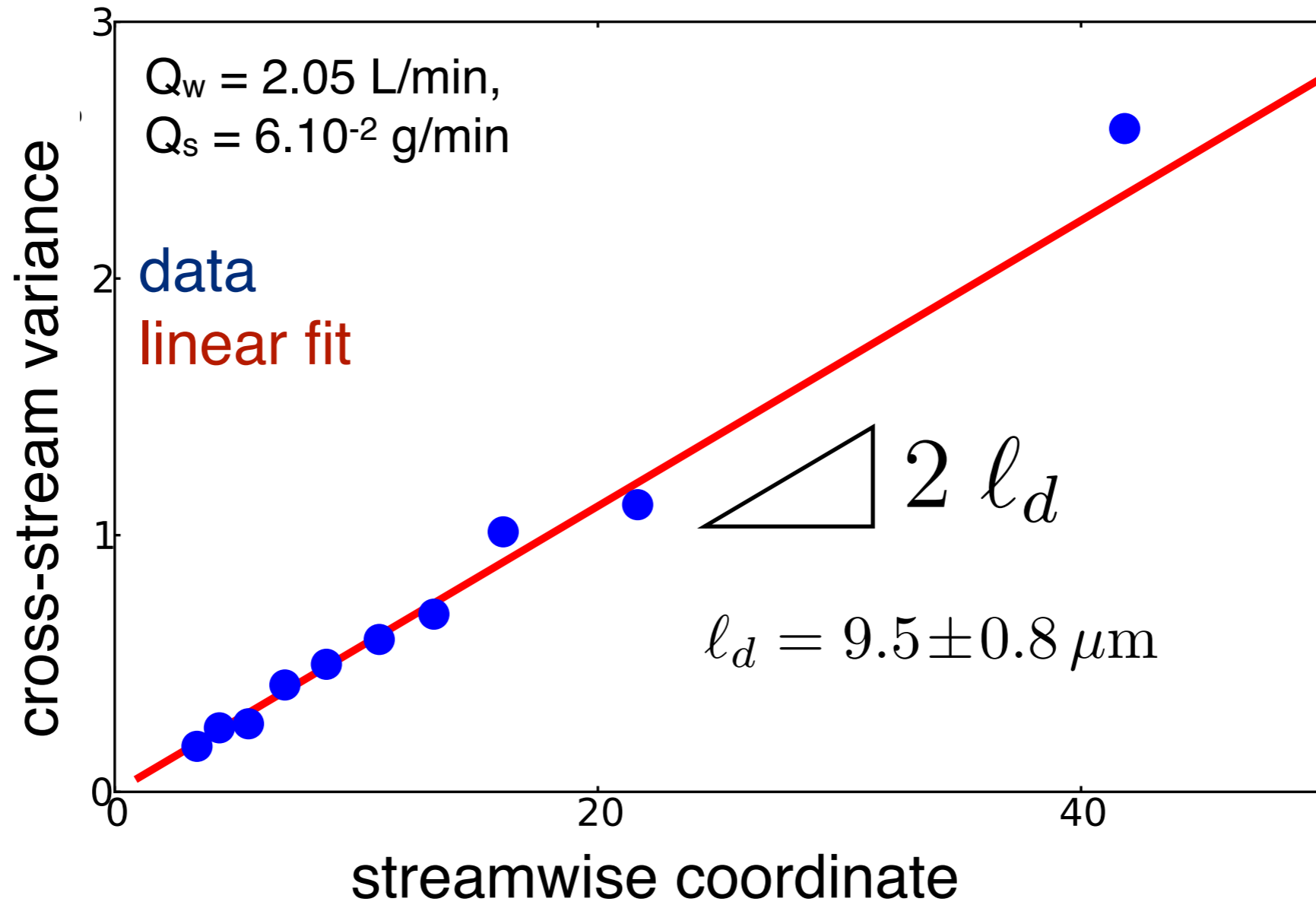
variance of $y \propto x$?

Random walk



[Seizilles et al., sub.]

Random walk



variance of y diffusion length

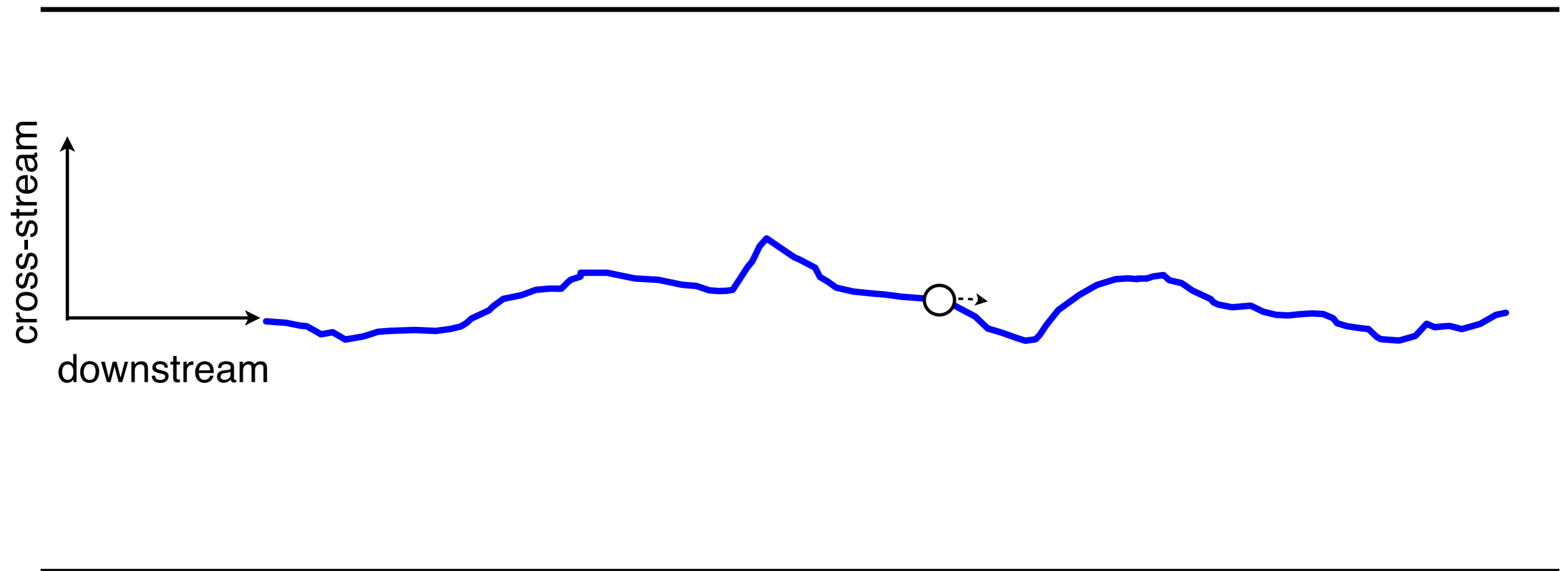
$$\langle y^2 \rangle = 2 \ell_d x$$

[Seizilles et al., sub.]

From random walk

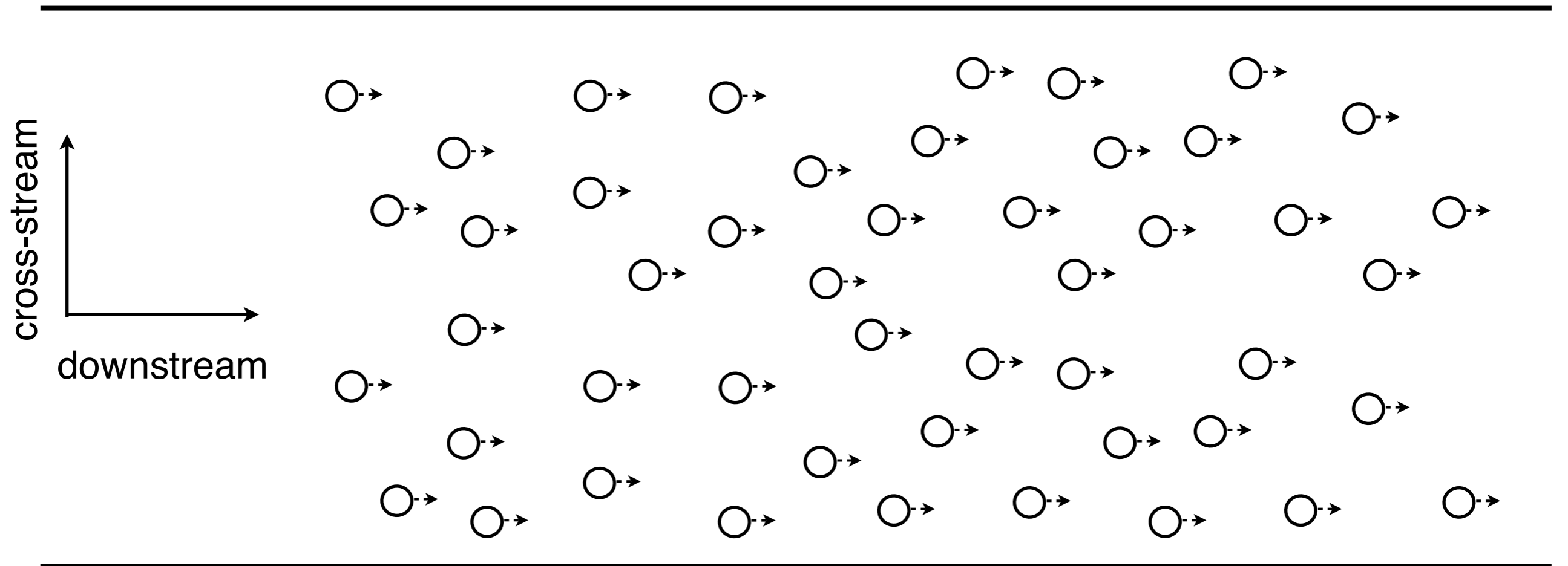
- **single particle = random walker**

Each time the particle takes one step in the streamwise direction, it also takes one random step either to the left or to the right in the cross-stream direction.



... to diffusion

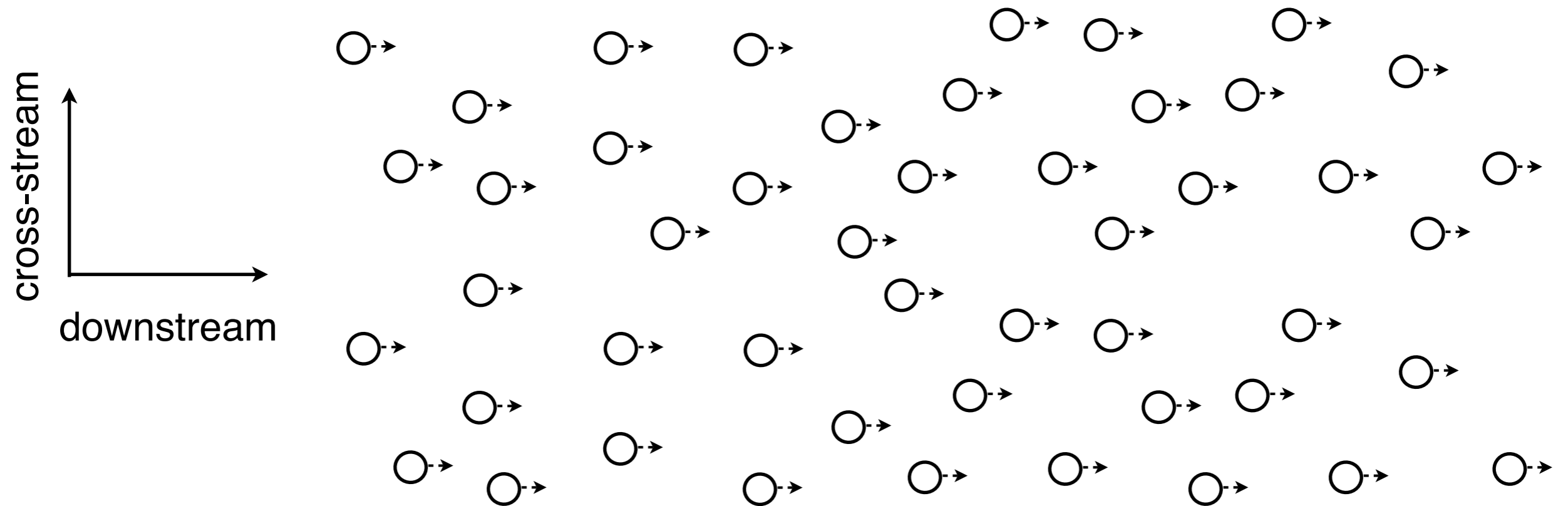
- many random walkers



... to diffusion

- many random walkers

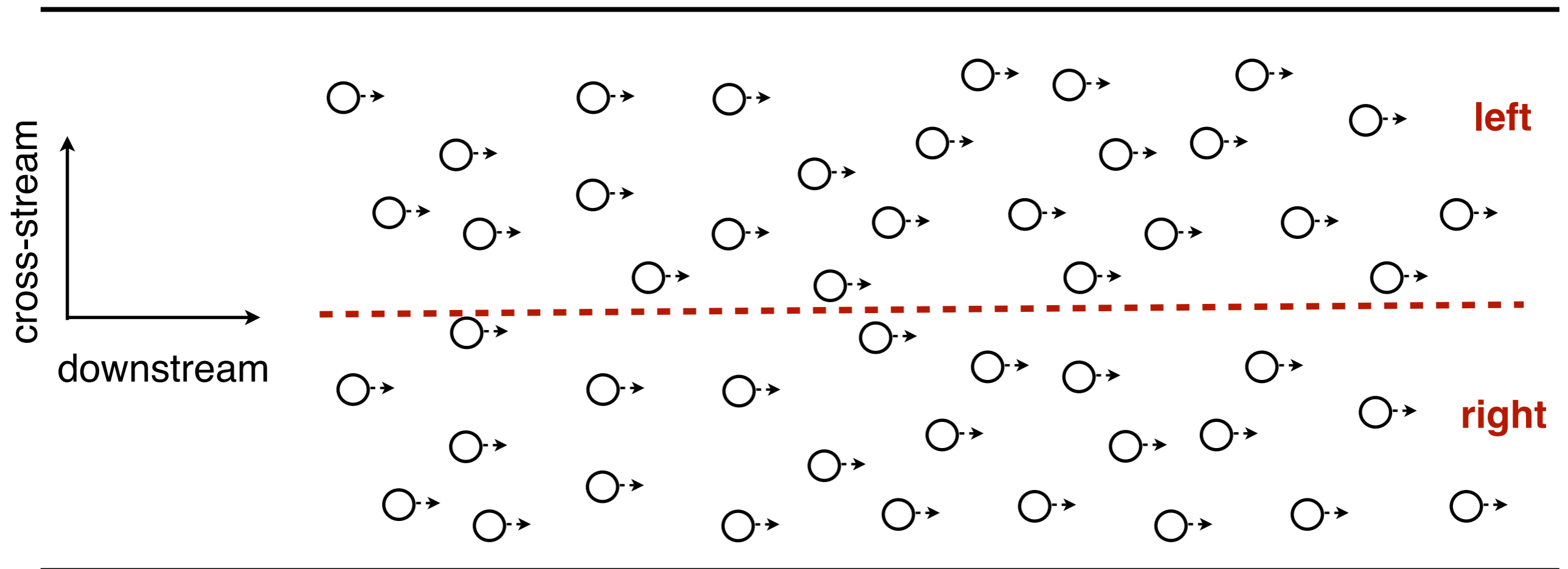
moving grains homogeneously distributed



... to diffusion

- many random walkers

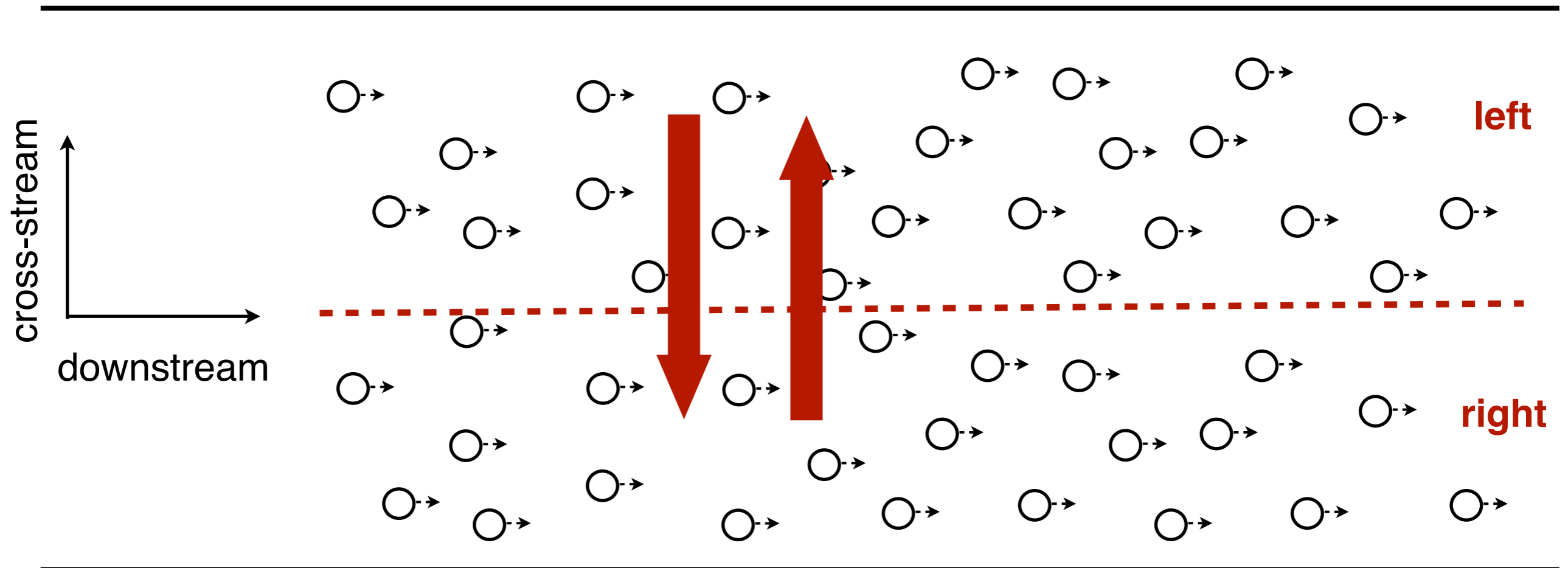
moving grains homogeneously distributed



... to diffusion

- many random walkers

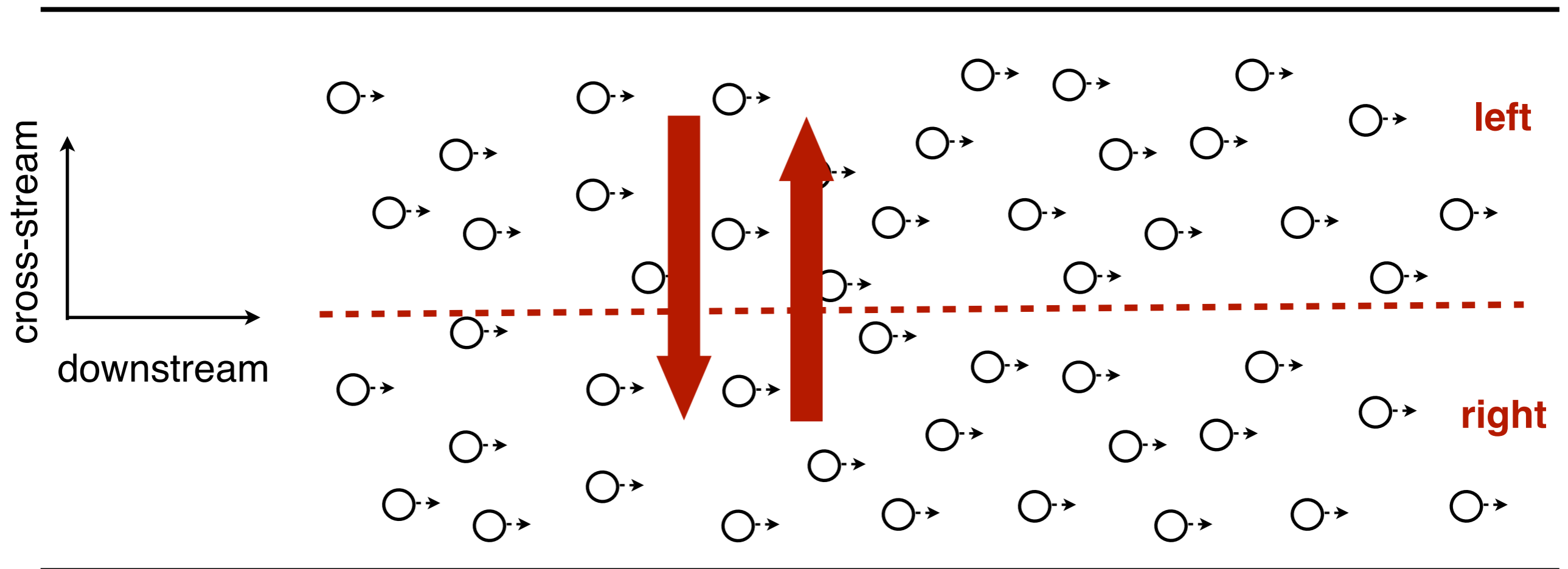
moving grains homogeneously distributed



... to diffusion

- many random walkers

moving grains homogeneously distributed

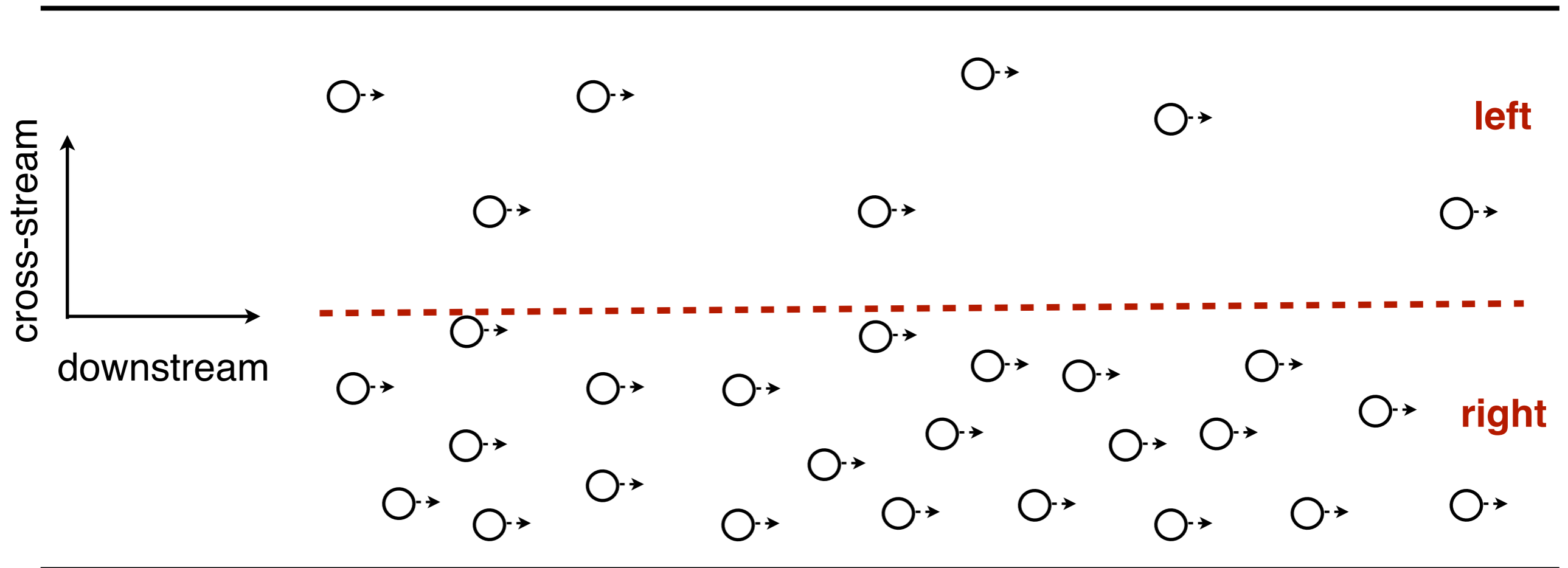


→ no net flux along the cross-stream direction

... to diffusion

- many random walkers

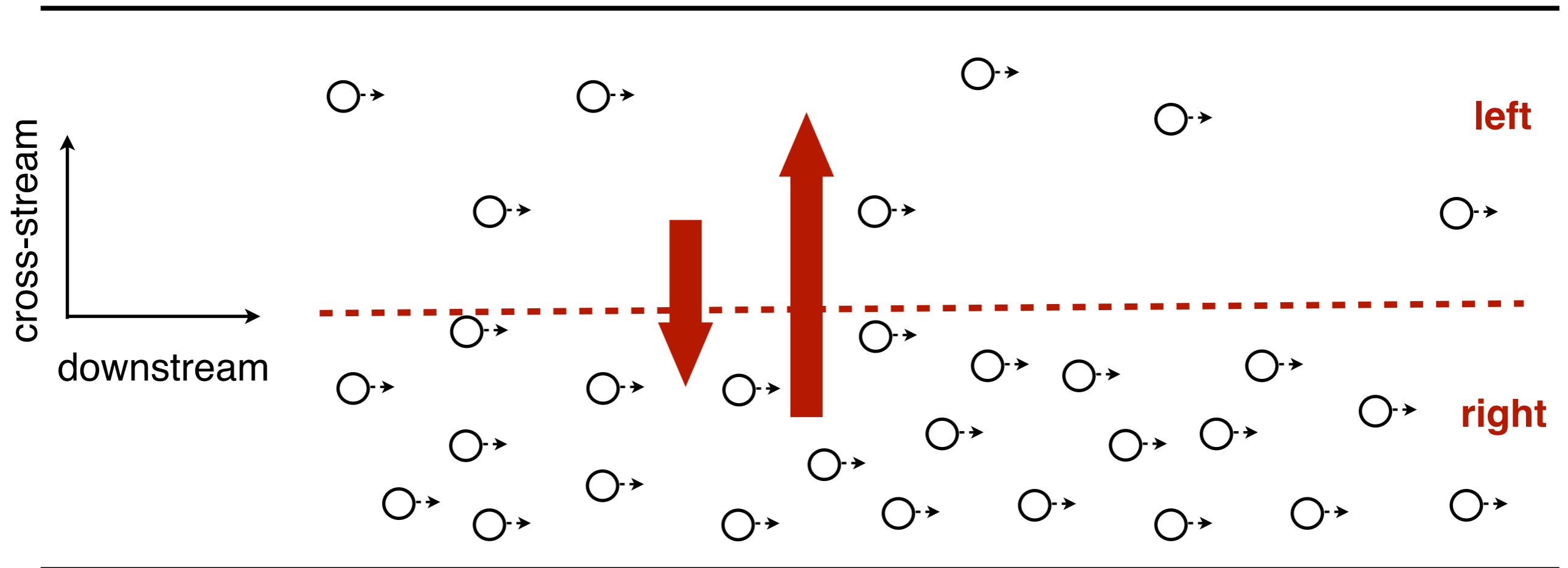
gradient of concentration of moving grains



... to diffusion

- many random walkers

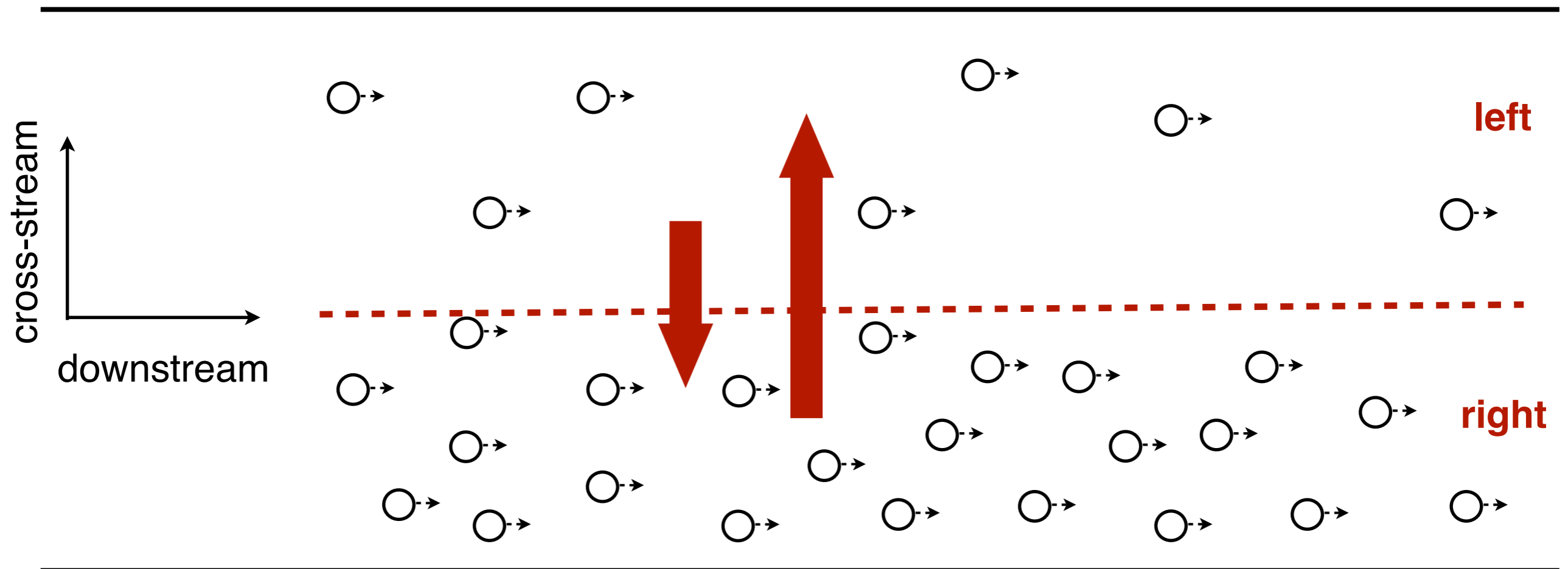
gradient of concentration of moving grains



... to diffusion

- many random walkers

gradient of concentration of moving grains



→ net sediment flux

- directed toward the less populated areas
- proportional to the gradient of the number of particles

Bedload diffusion

diffusive flux diffusion coef. concentration of moving particles

$$\vec{q}_d = -\zeta_D \nabla_{\perp} n$$

velocity distributions
& correlation time

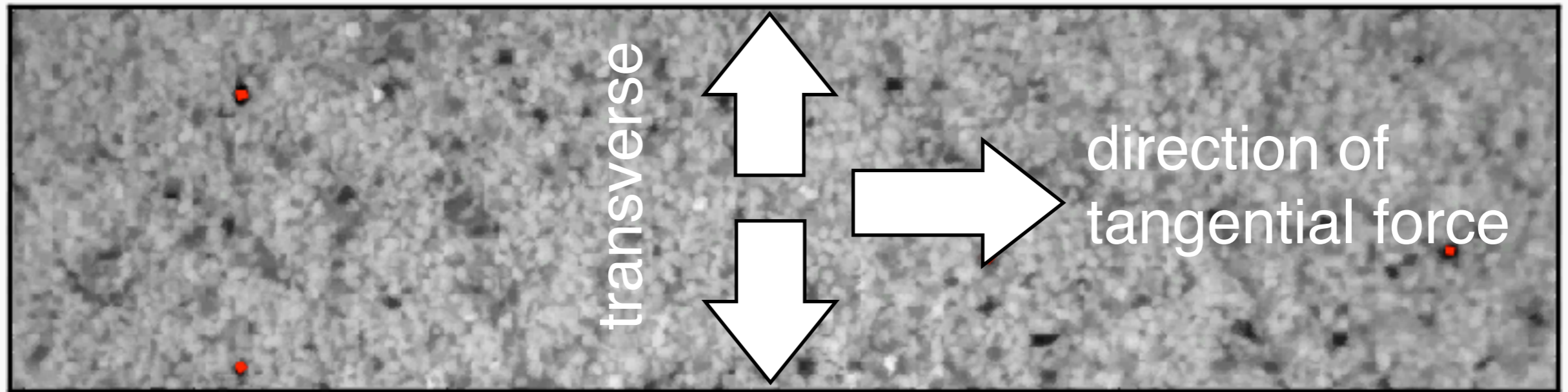


diffusive length particle velocity

$$\zeta_D = \ell_D V \approx 0.03 d_s V$$

[Seizilles et al., in press]

Bedload transport : the complete picture !



bedload transport, advection $\vec{q}_b = n \vec{V}$

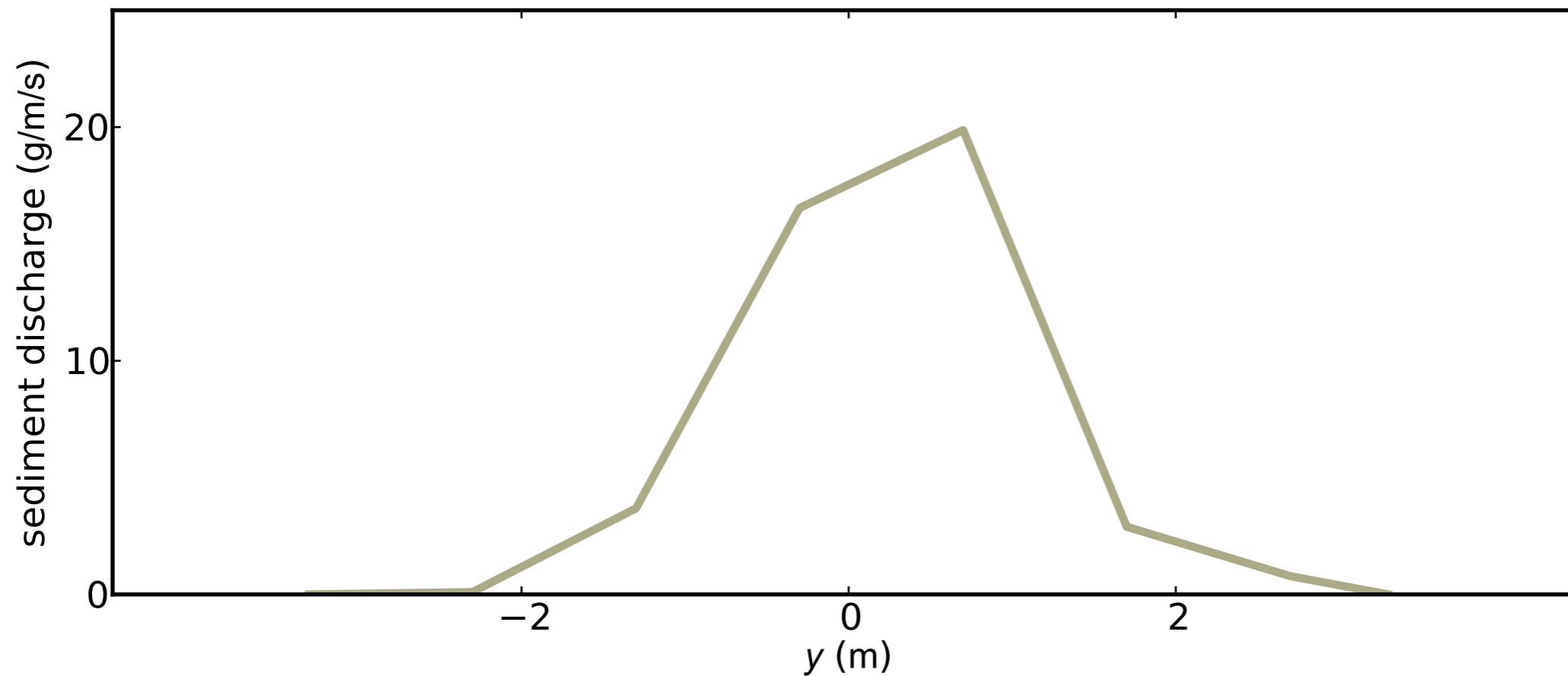
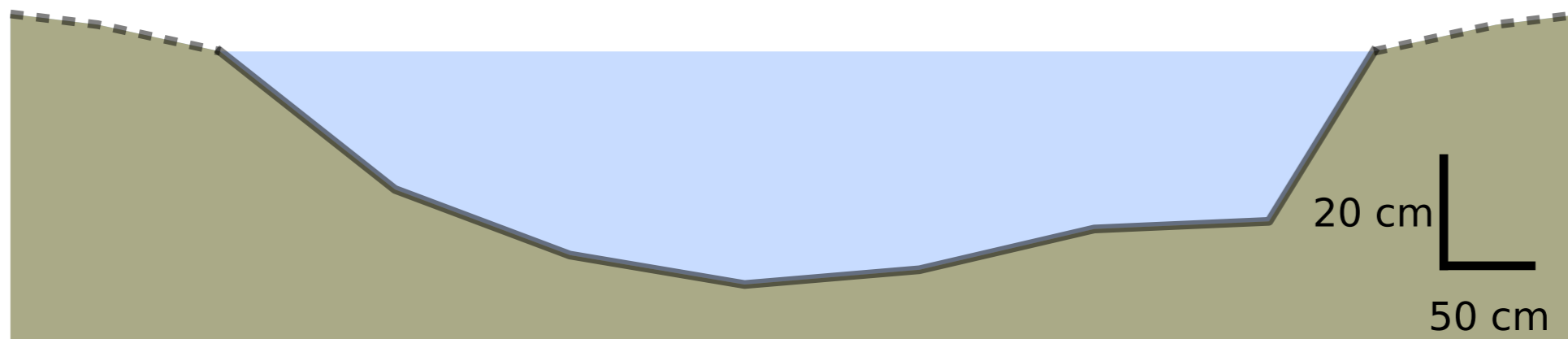
transverse flux, diffusion $\vec{q}_d = -\zeta_D \vec{\nabla}_{\perp} n$

Do gradients of bedload particles exist in nature ?



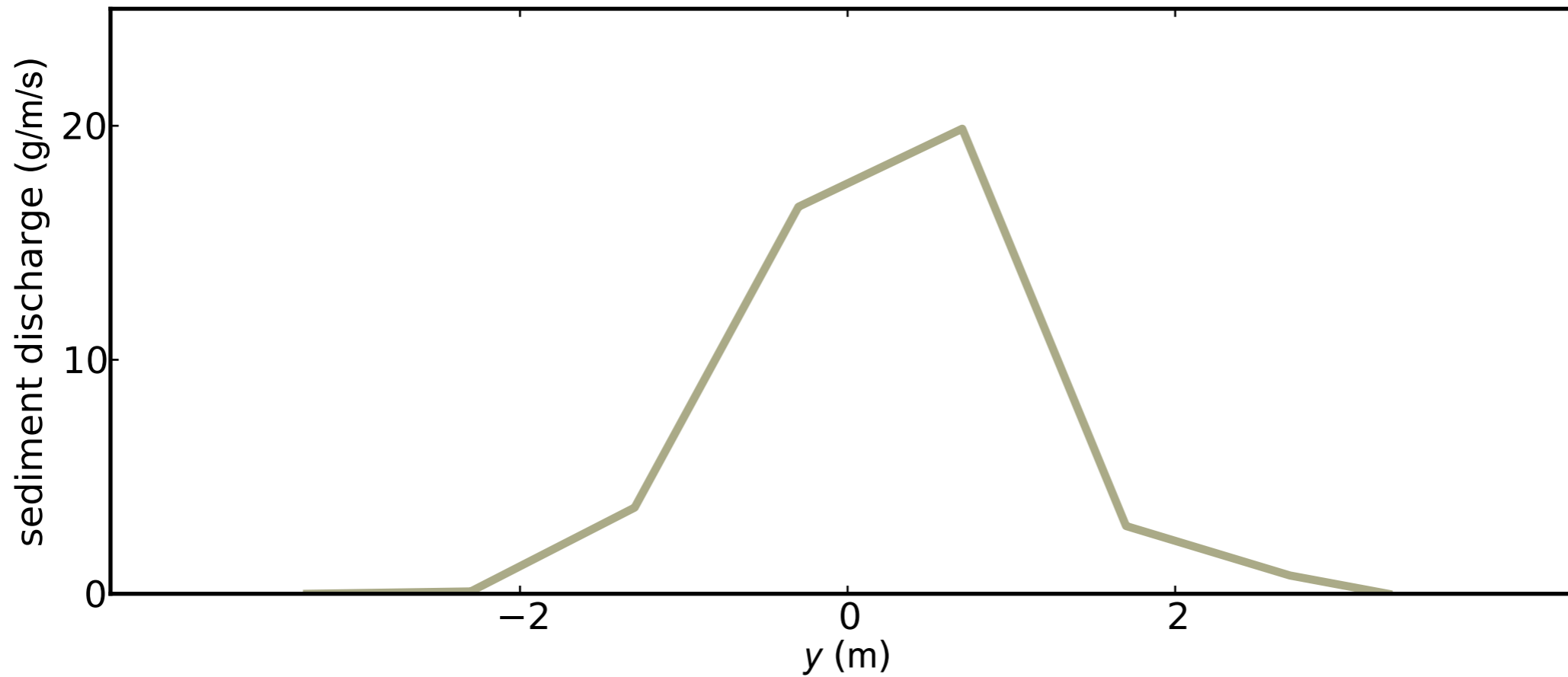
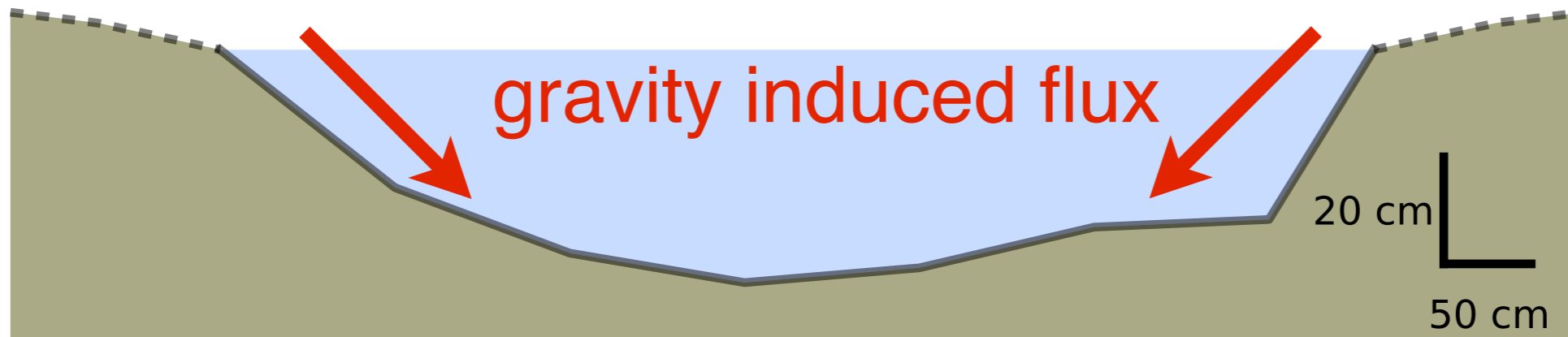
Urumqi He, Tian-Shan, China

Do gradients of bedload particles exist in nature ?



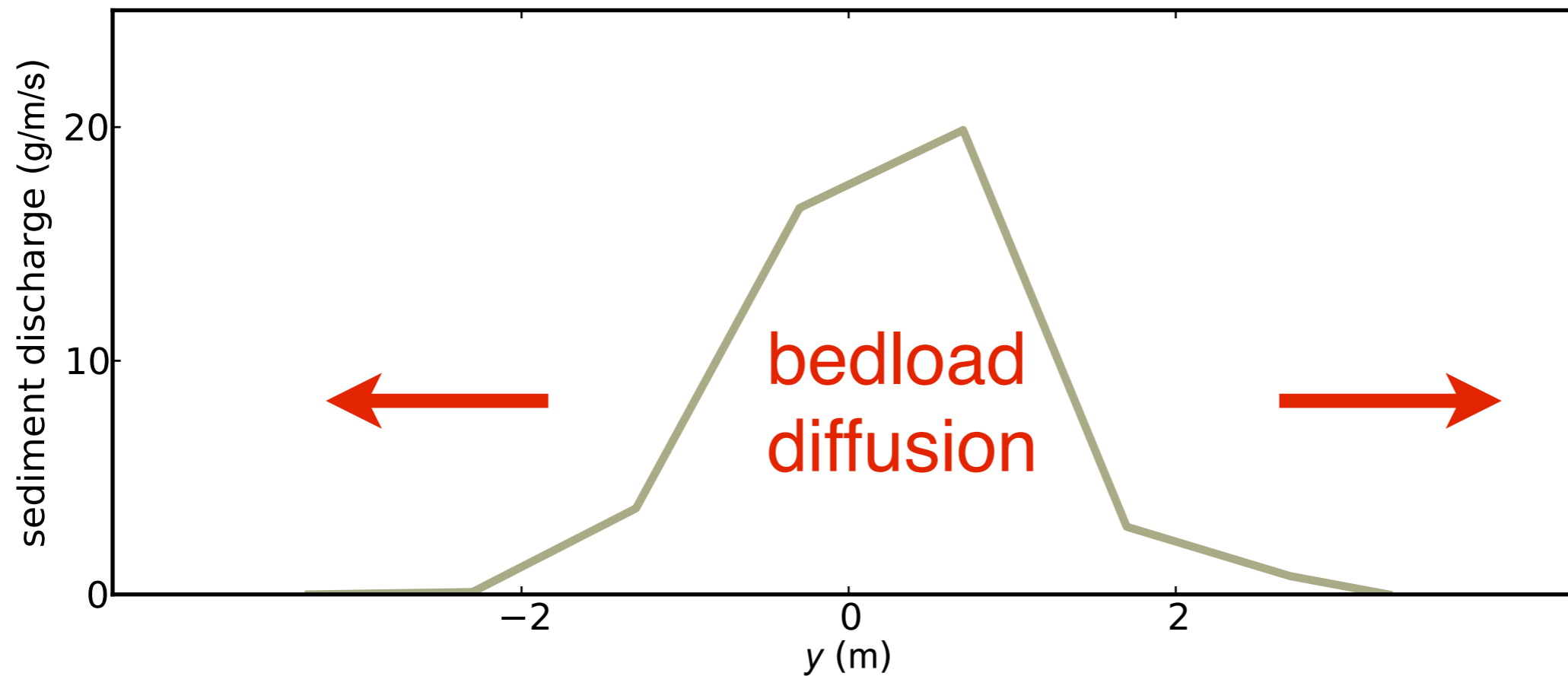
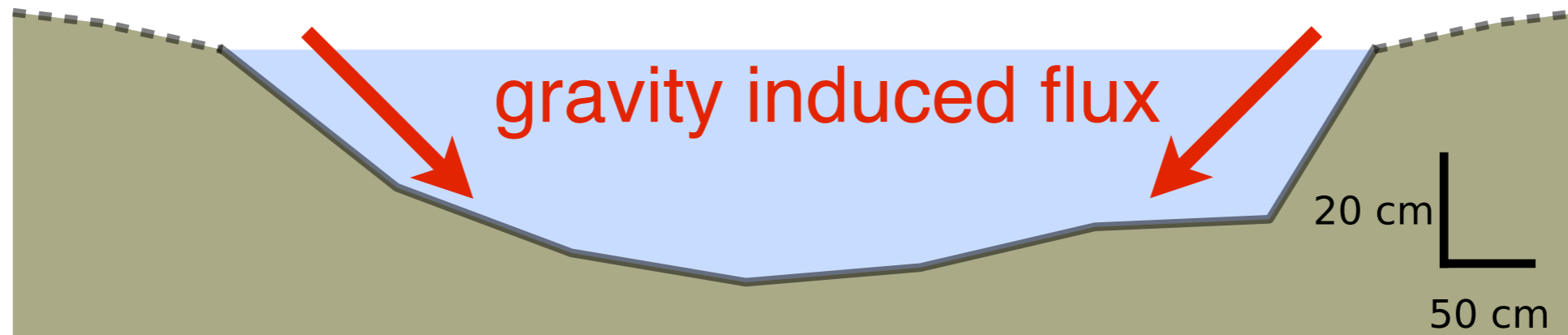
Urumqi He, Tian-Shan, China

Do gradients of bedload particles exist in nature ?



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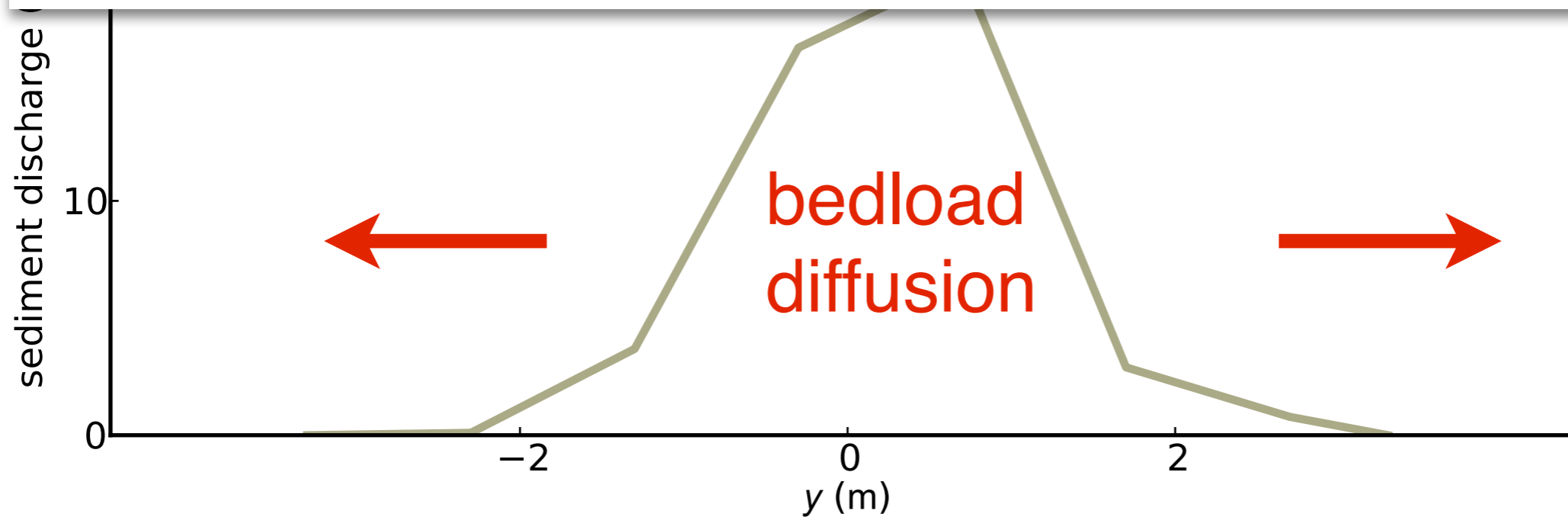


Urumqi He, Tian-Shan, China

Do gradients of bedload particles exist in nature ?

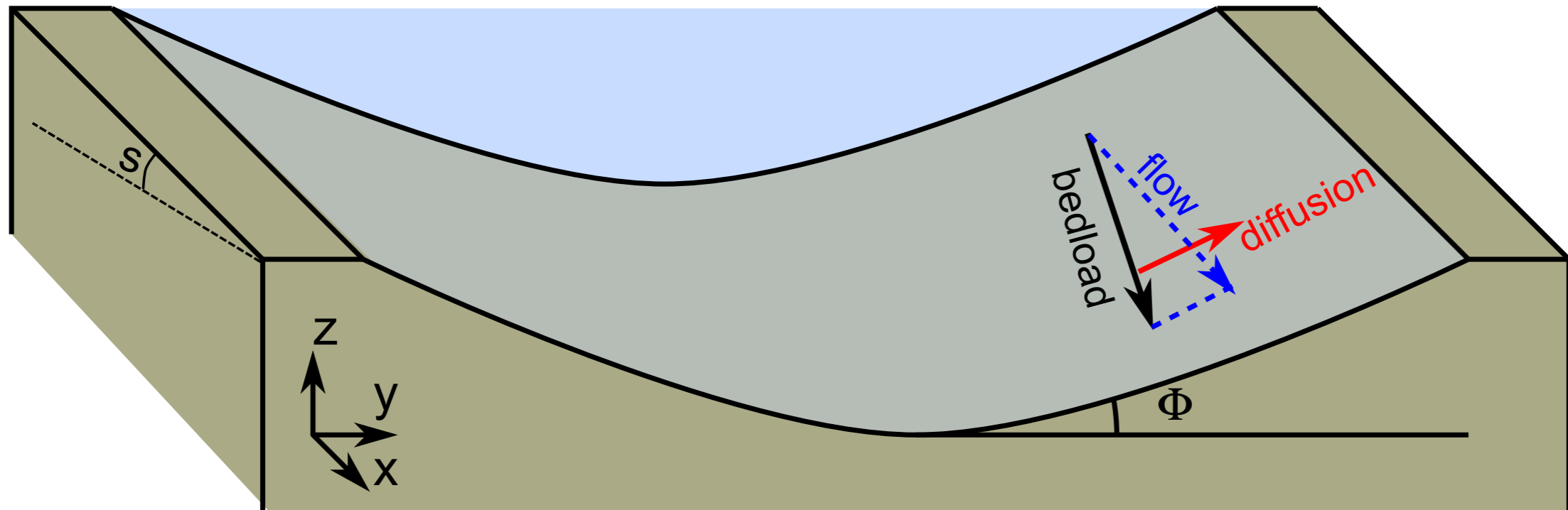


What would be the shape of a river in which the gravity-induced flux and the diffusion flux would balance each-other?

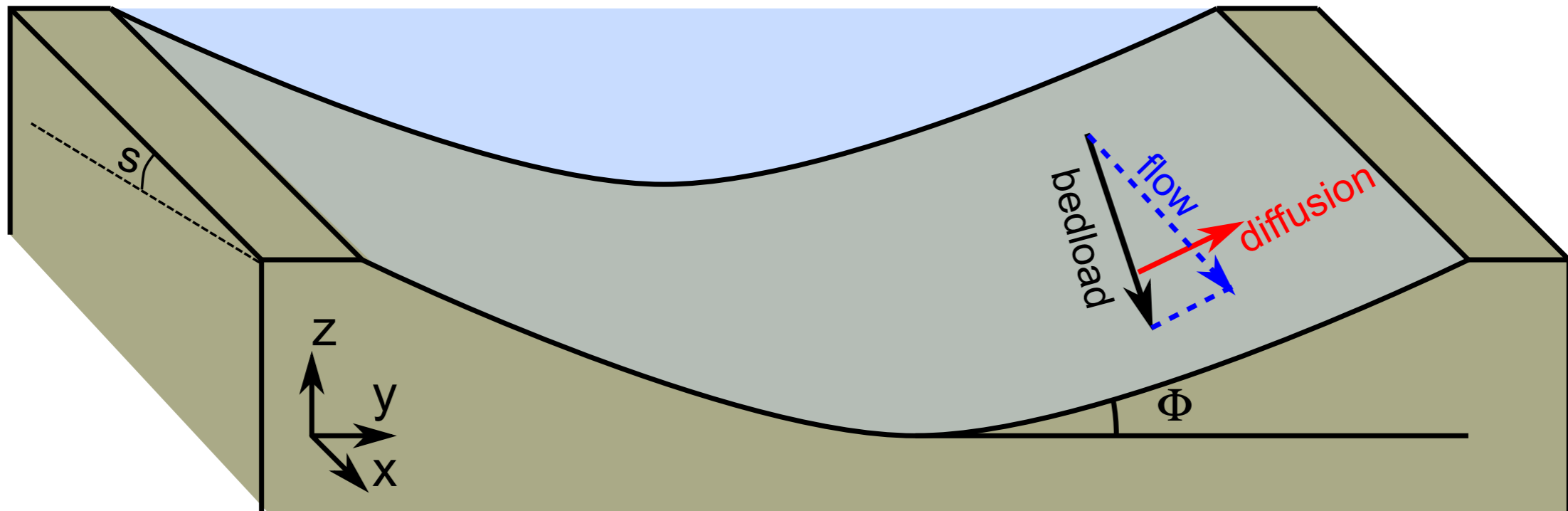


Urumqi He, Tian-Shan, China

Equilibrium with sediment transport



Equilibrium with sediment transport



$$Pe \left(\sqrt{\tilde{D}^2 + \tilde{D}'^2} - \mu \right) \left(\tilde{D}^2 + \tilde{D}'^2 \right) - \tilde{D}^2 \left(\tilde{D} + \tilde{D}'' \right) = 0$$

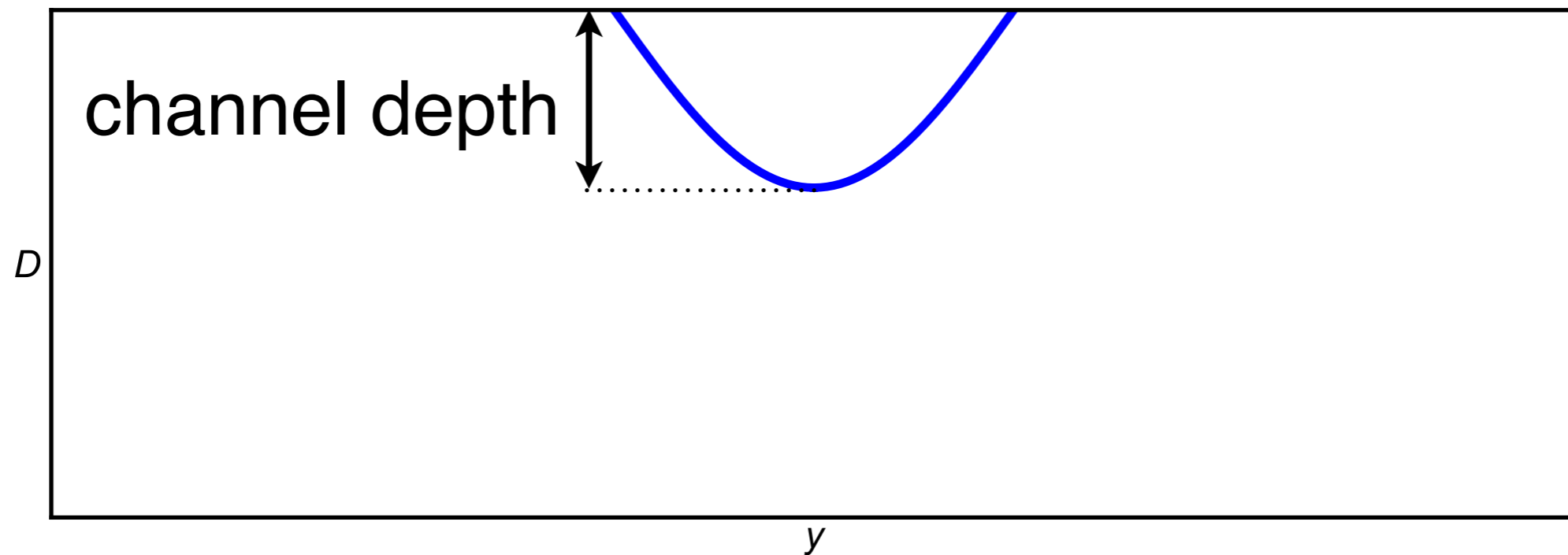
$$\tilde{D} = \frac{S}{L} D \quad \text{dimensionless depth}$$

$$Pe = \frac{L}{\ell_d} \frac{1}{S} \quad \text{Peclet number = advective / diffusive bedload}$$

Numerical solution

2 explicit parameters : Pe and channel depth

$$Pe = 2$$

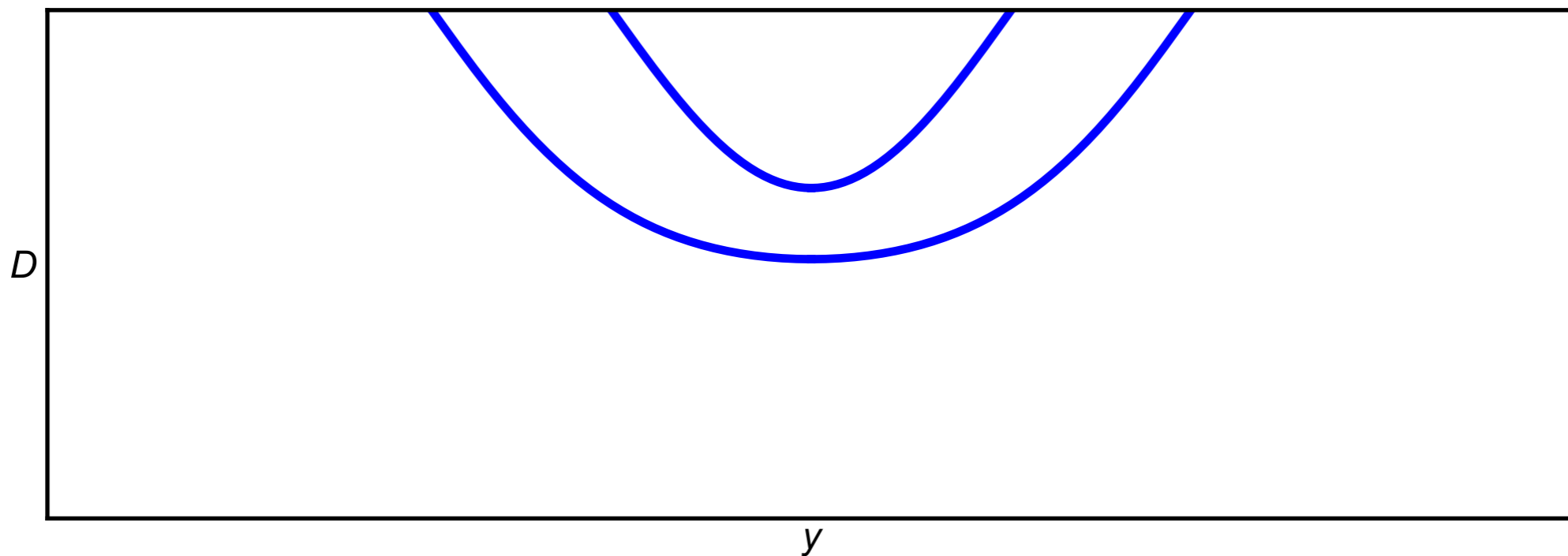


cosine solution for the 0 transport case

Numerical solution

2 explicit parameters : Pe and channel depth

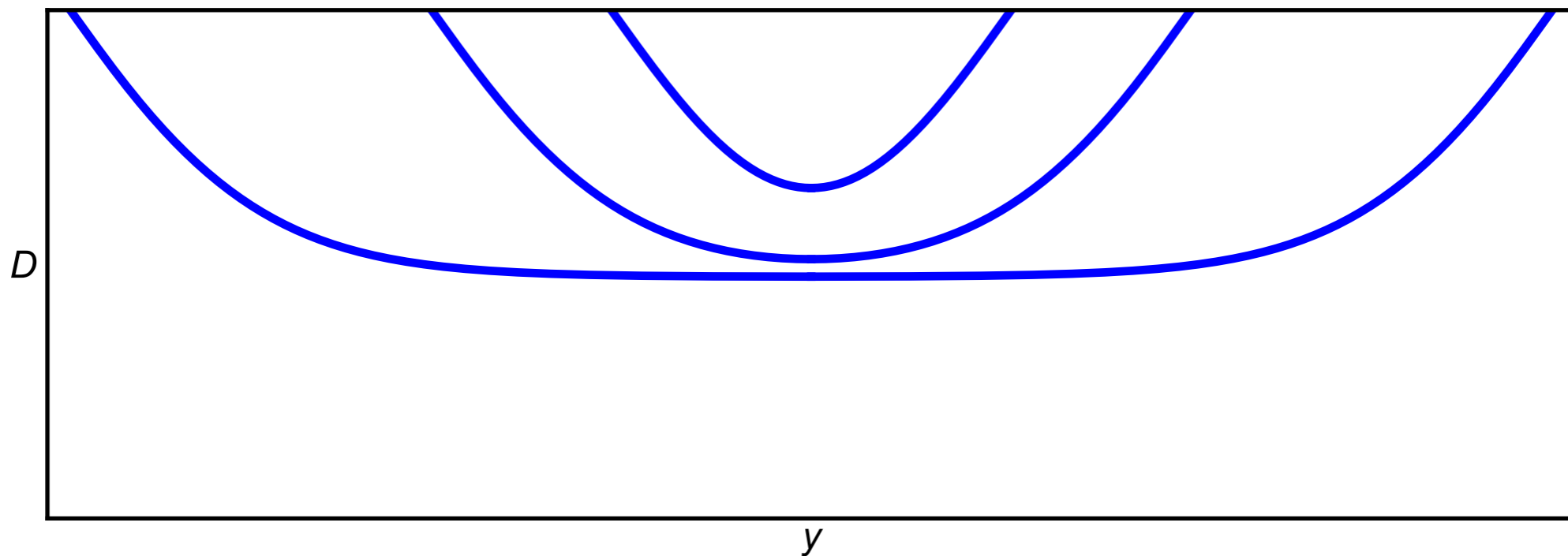
$$Pe = 2$$



Numerical solution

2 explicit parameters : Pe and channel depth

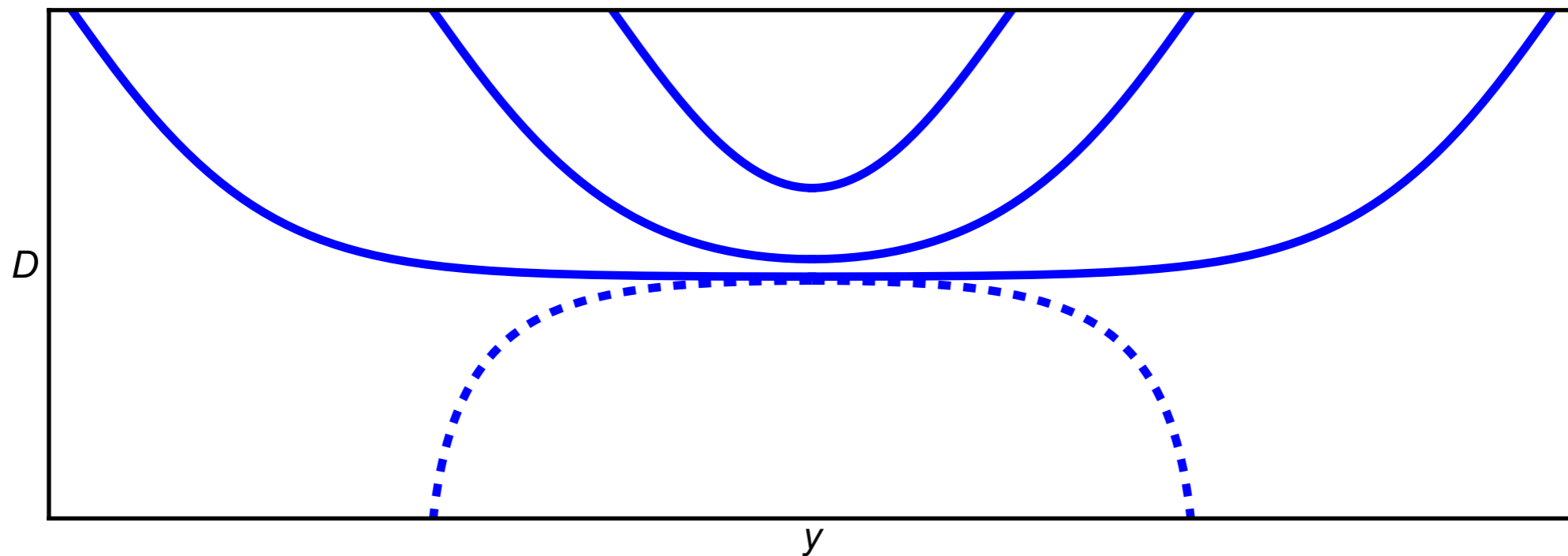
$$Pe = 2$$



Numerical solution

2 explicit parameters : Pe and channel depth

$$Pe = 2$$

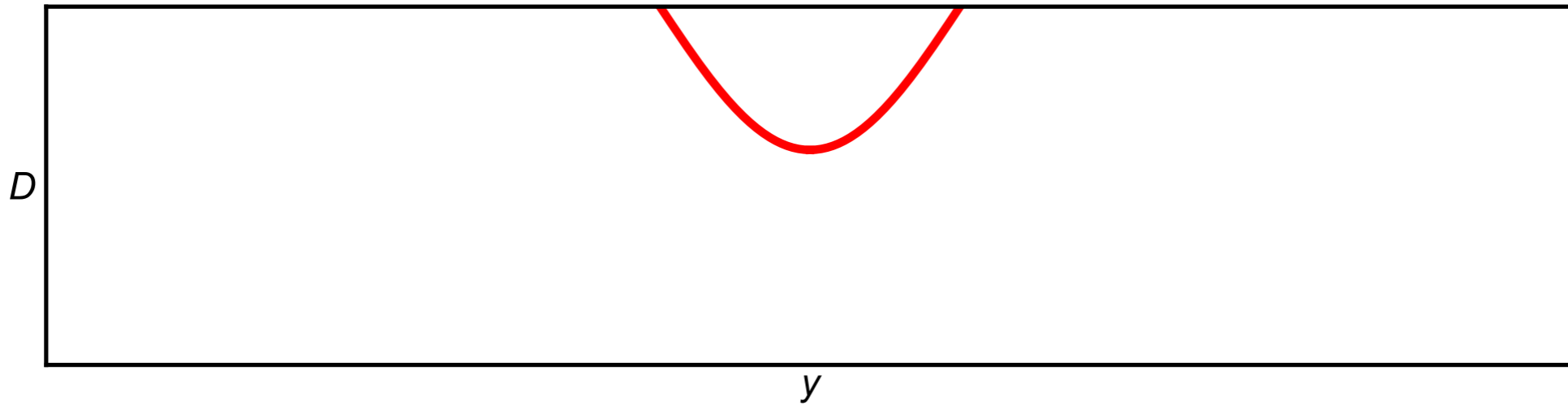
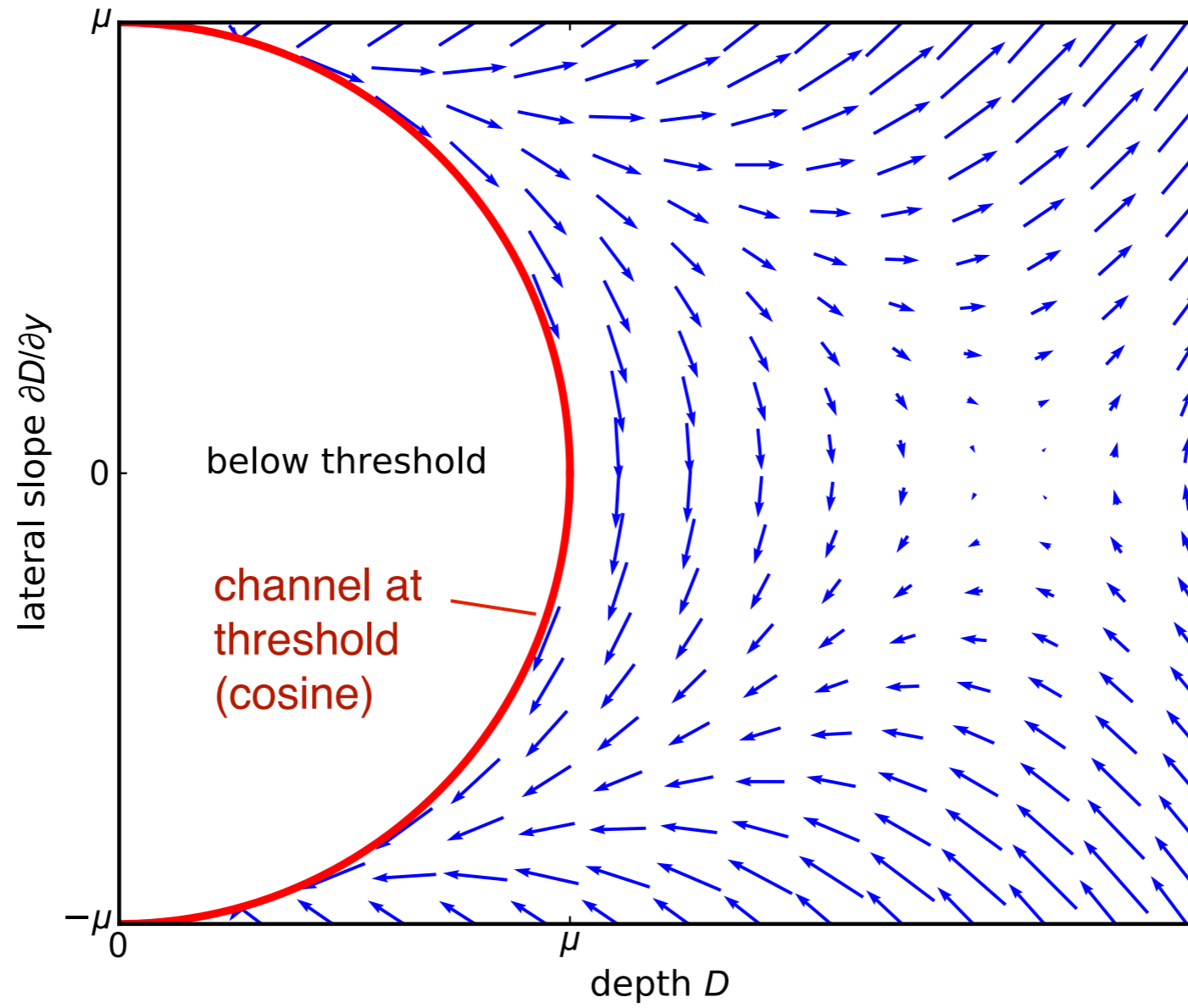


Numerical solution

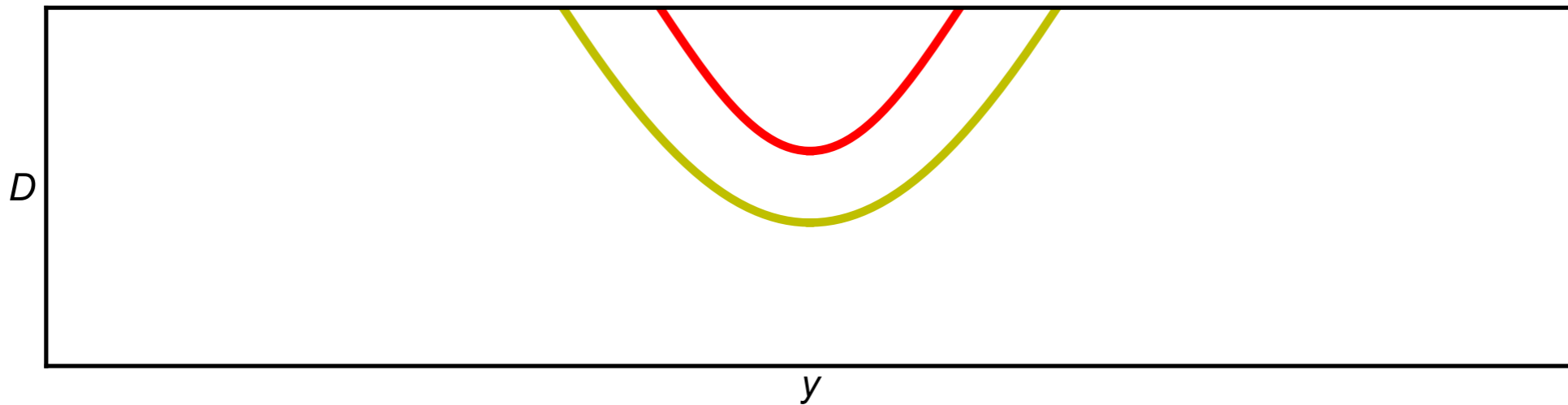
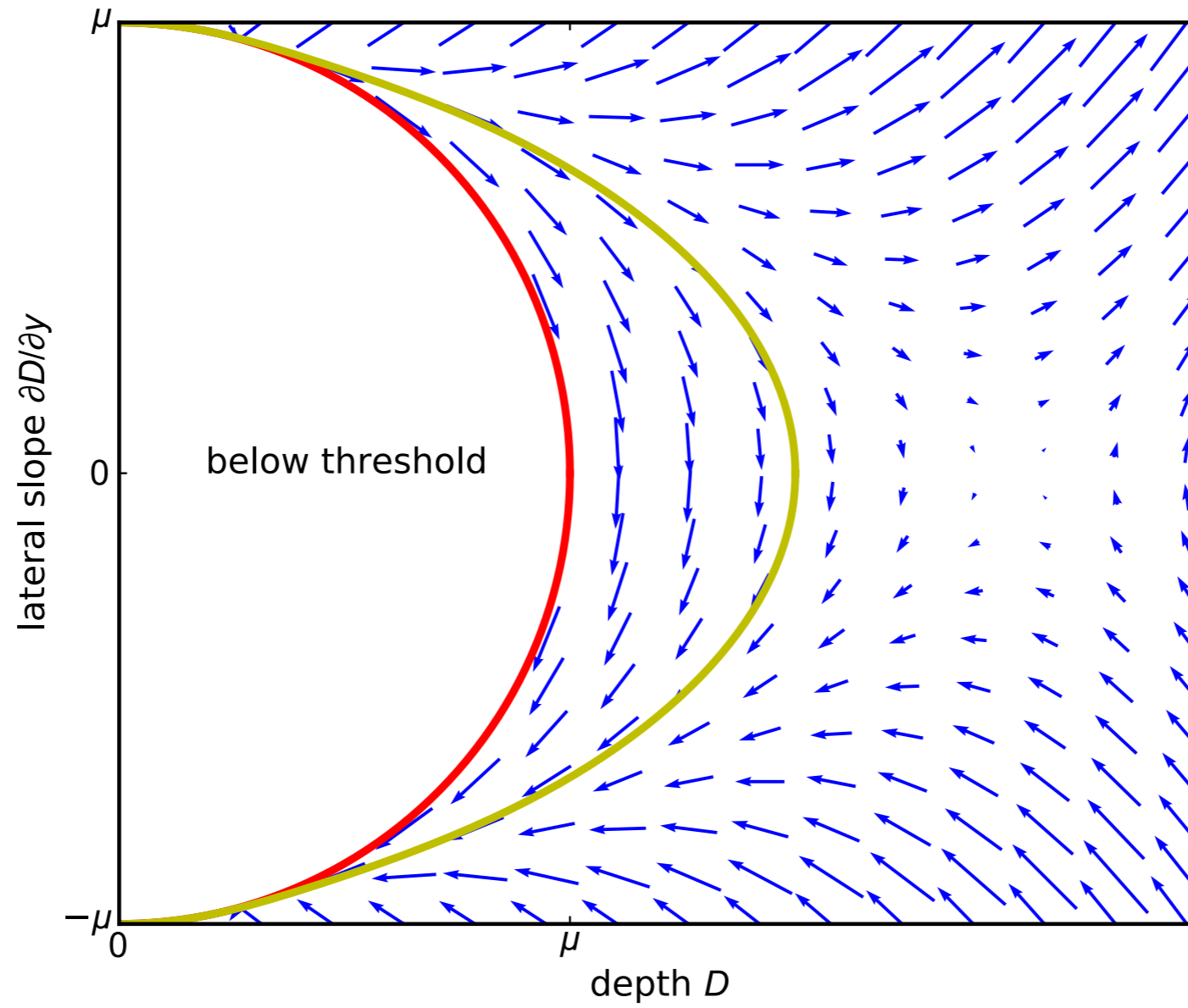
The theoretical solution depends explicitly on Pe and the channel depth

whereas we are interested in the evolution of the river morphology as a function of the flow and sediment discharges!

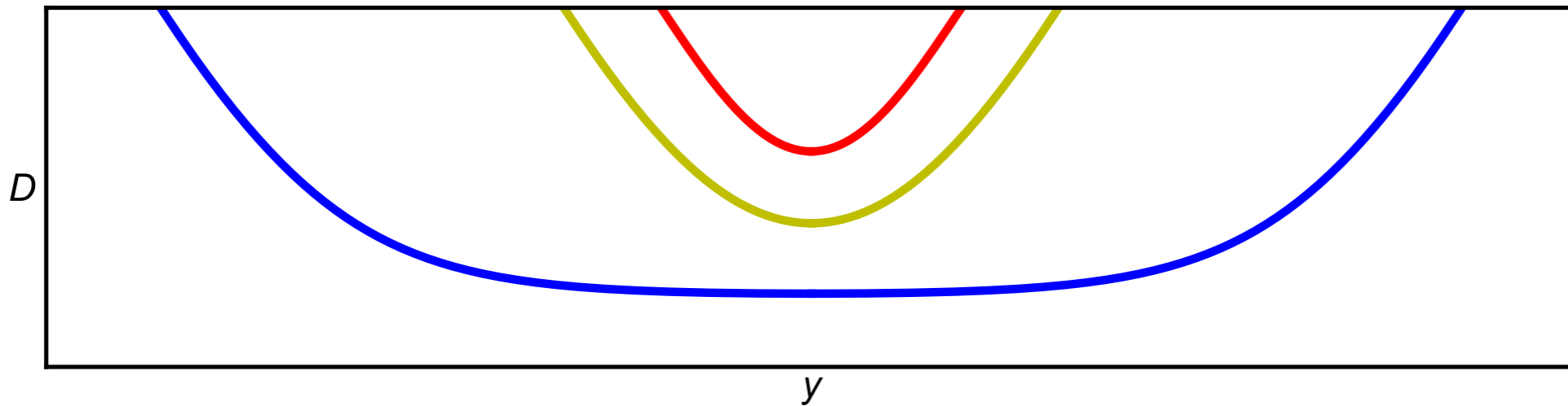
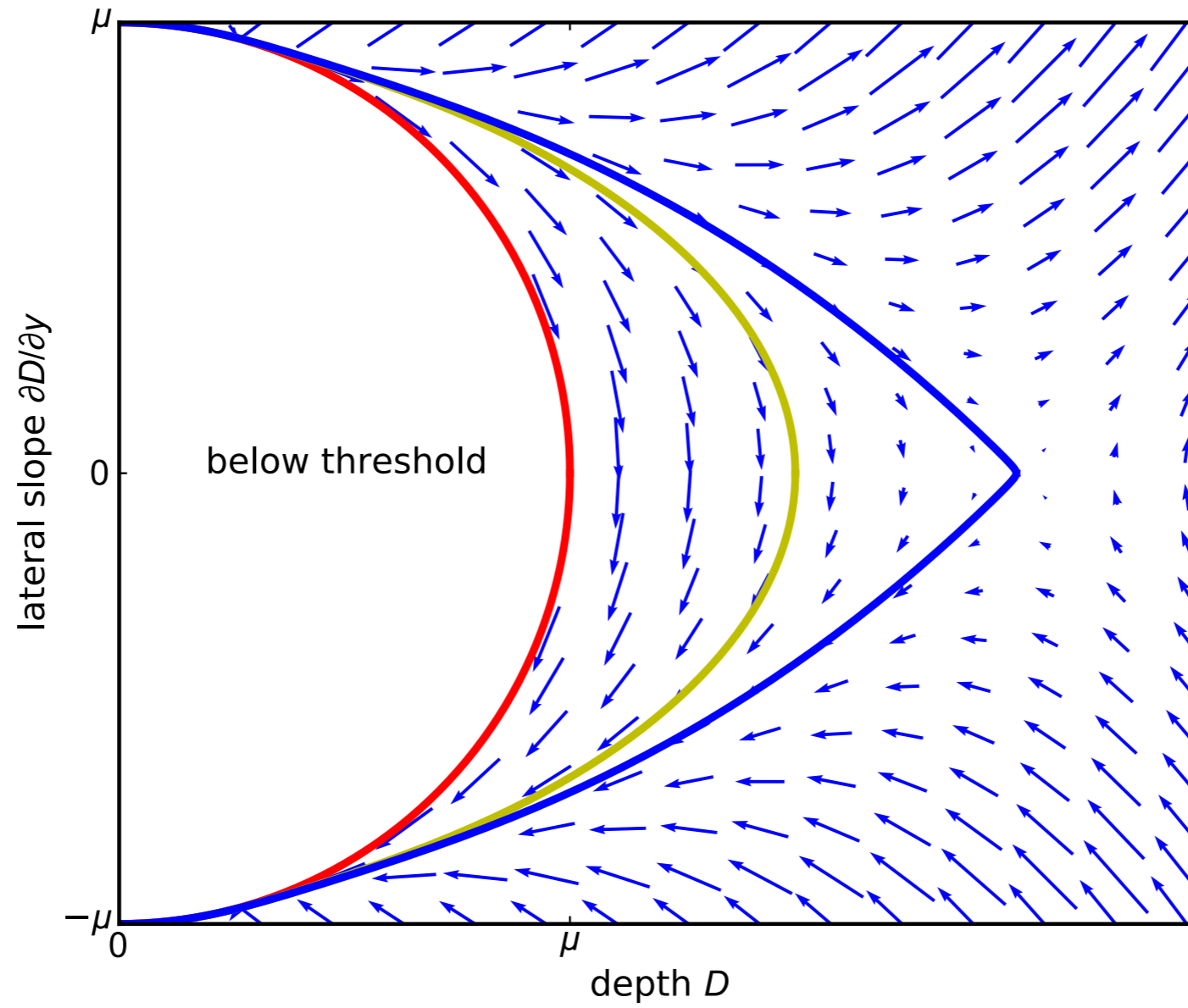
Phase space



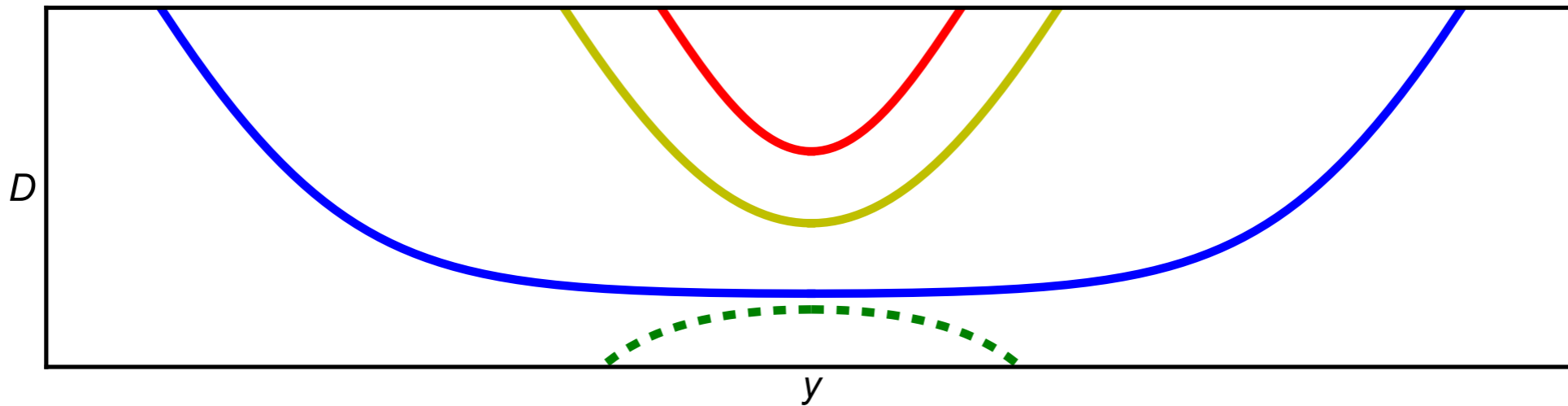
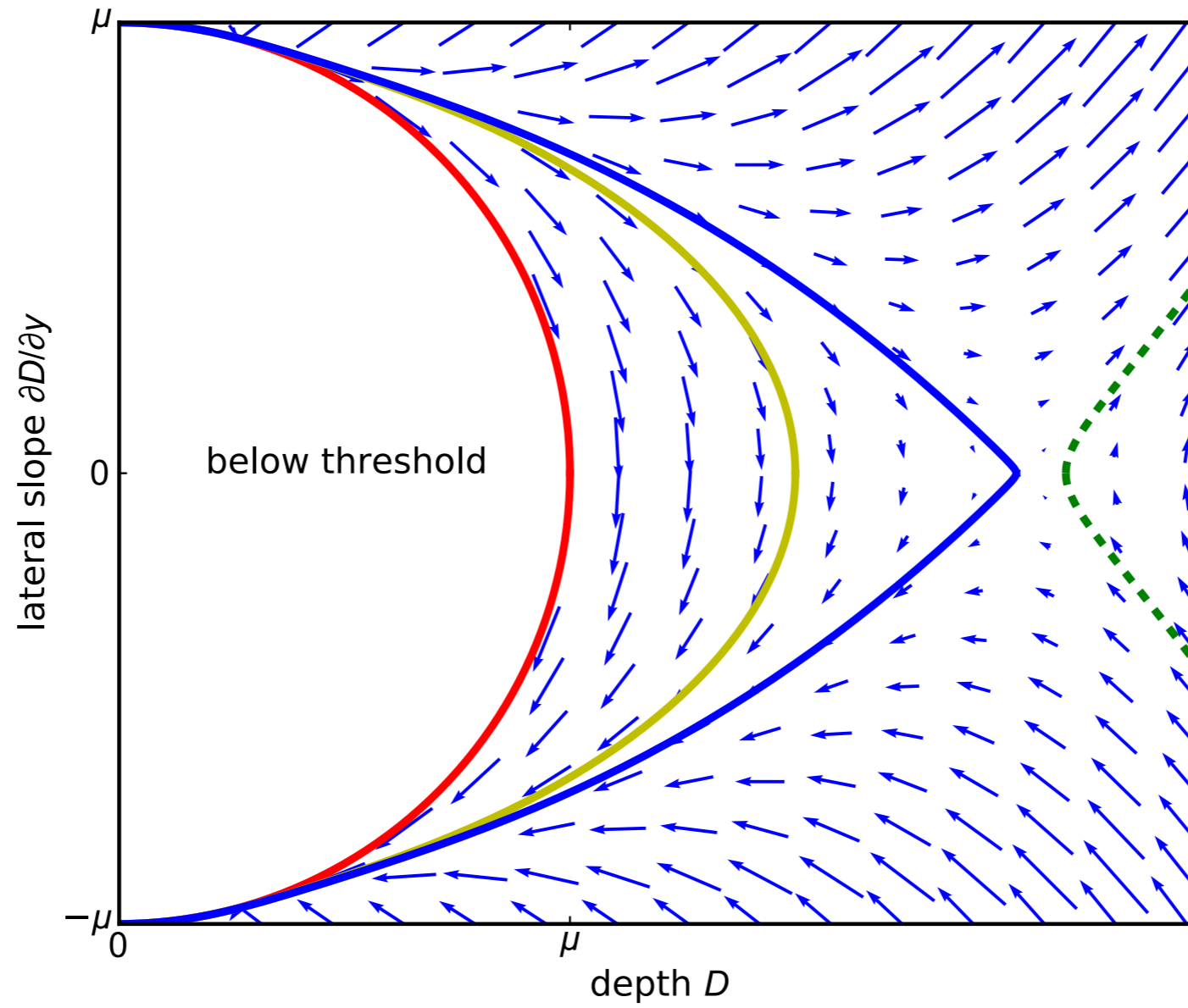
Phase space



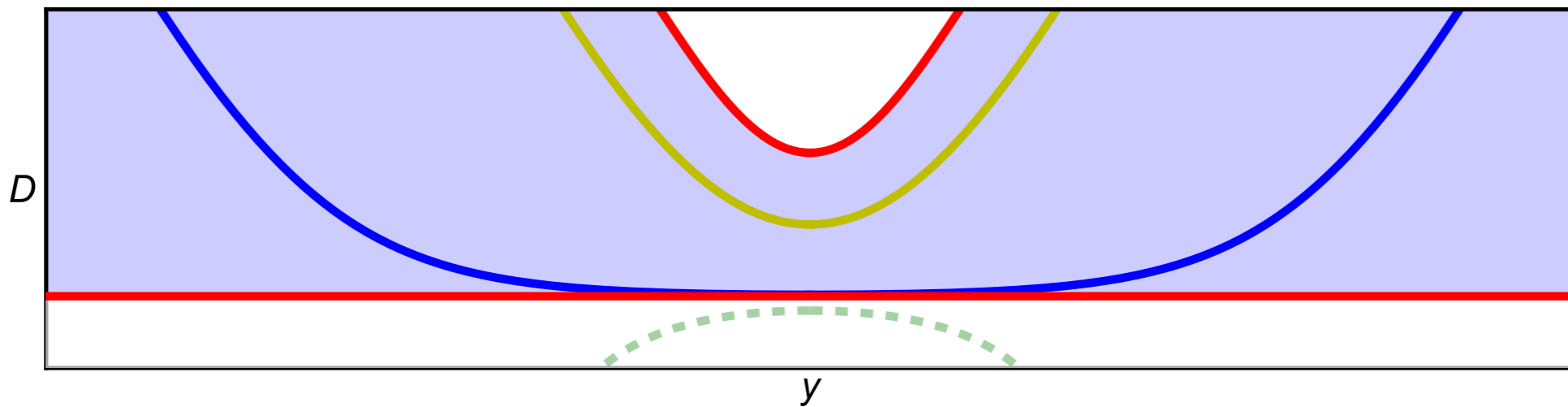
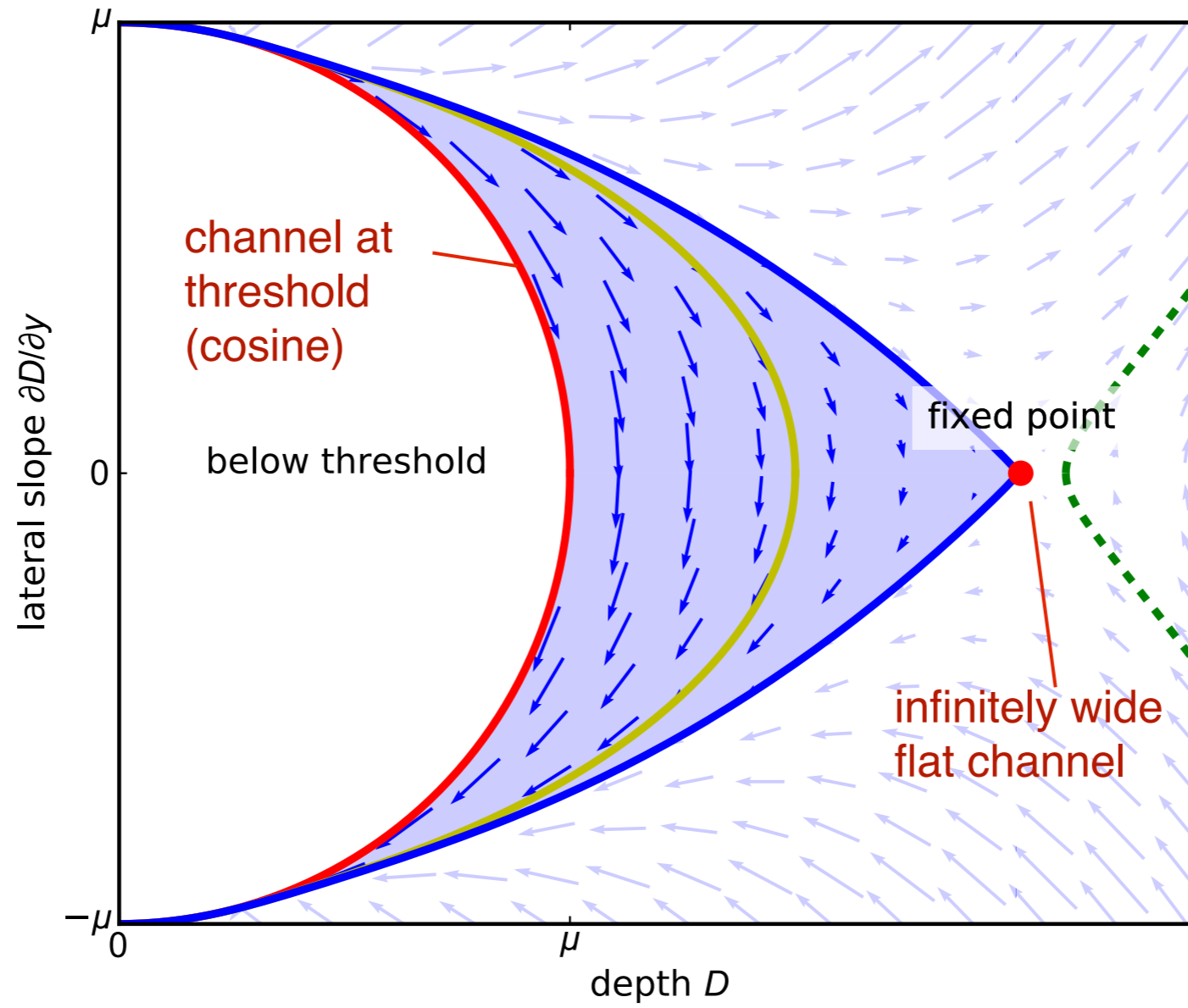
Phase space



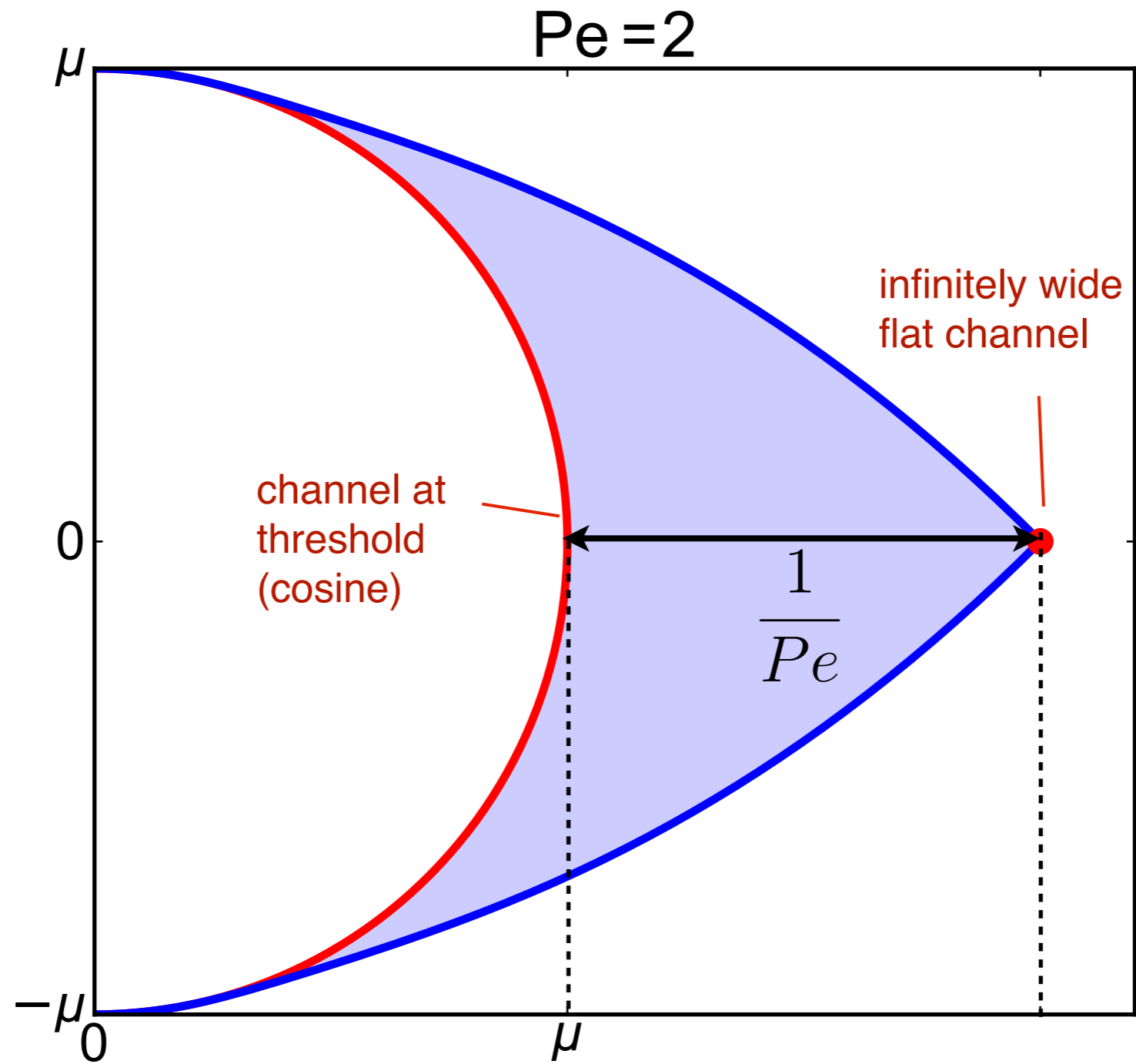
Phase space



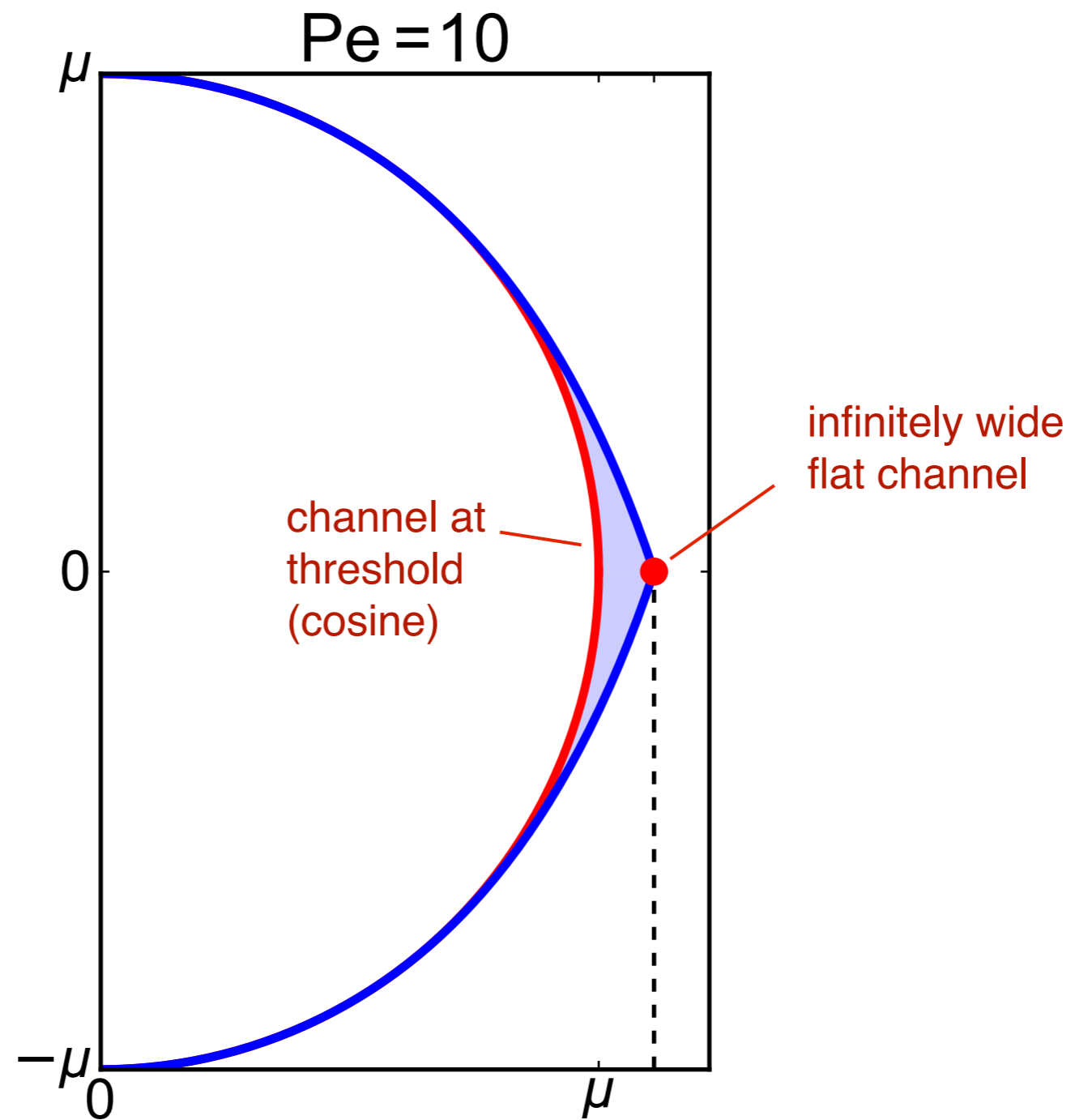
Phase space



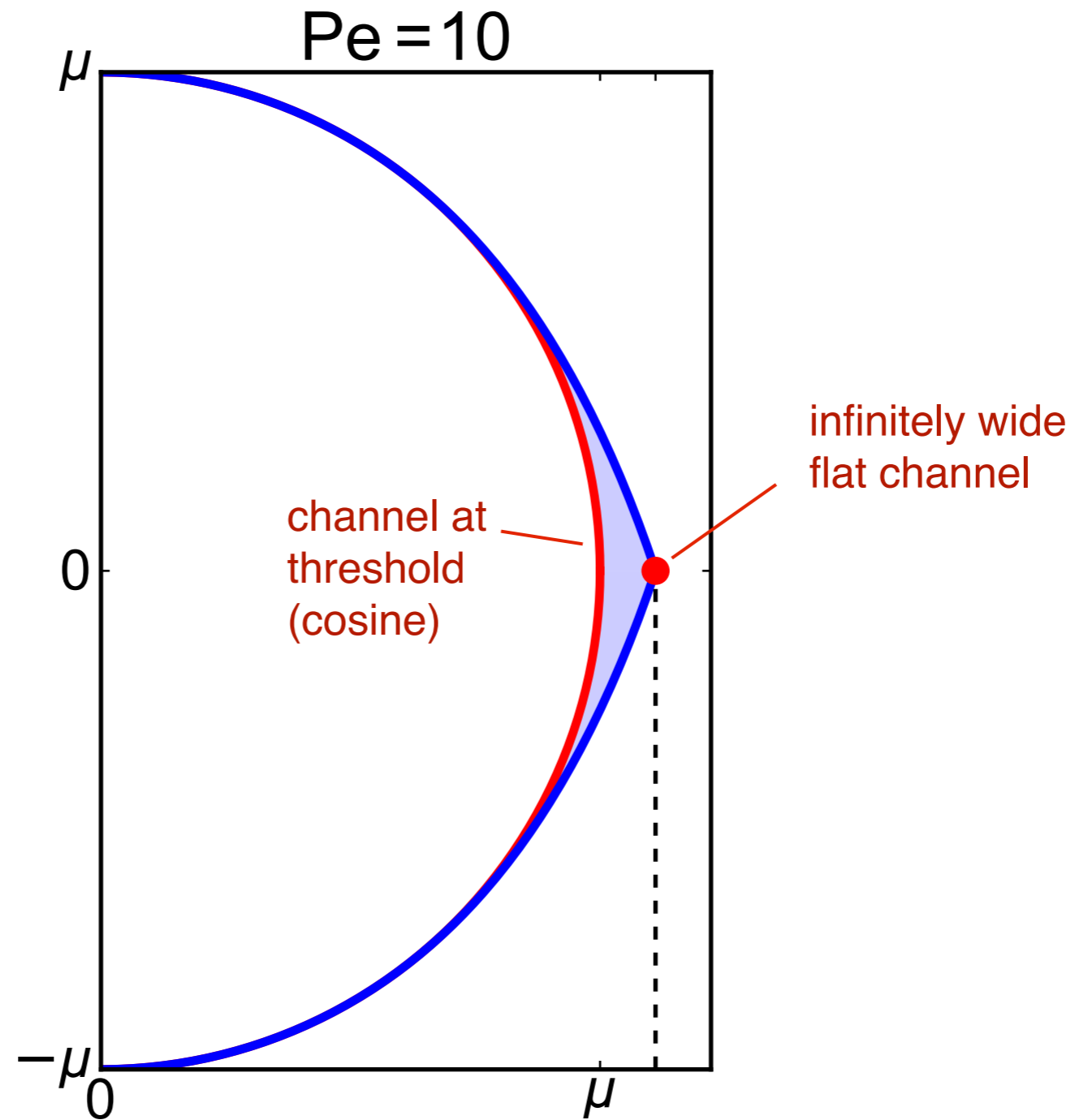
Phase space



Phase space

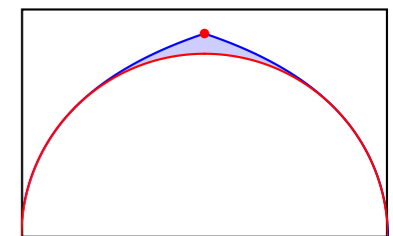
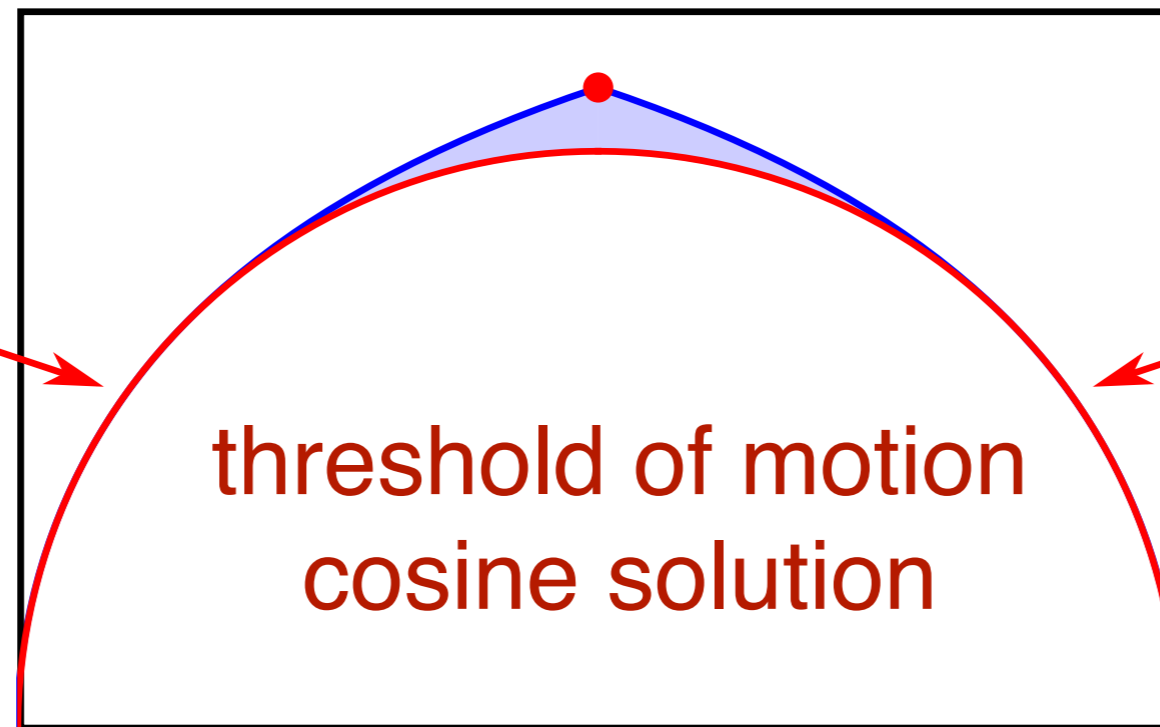
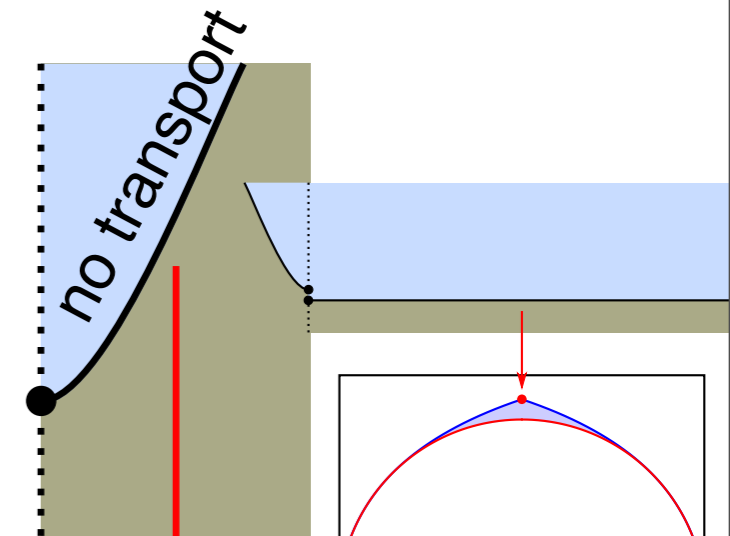
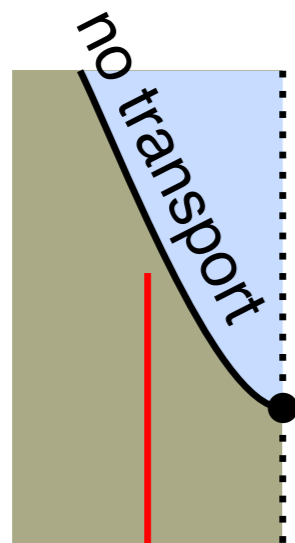


Phase space

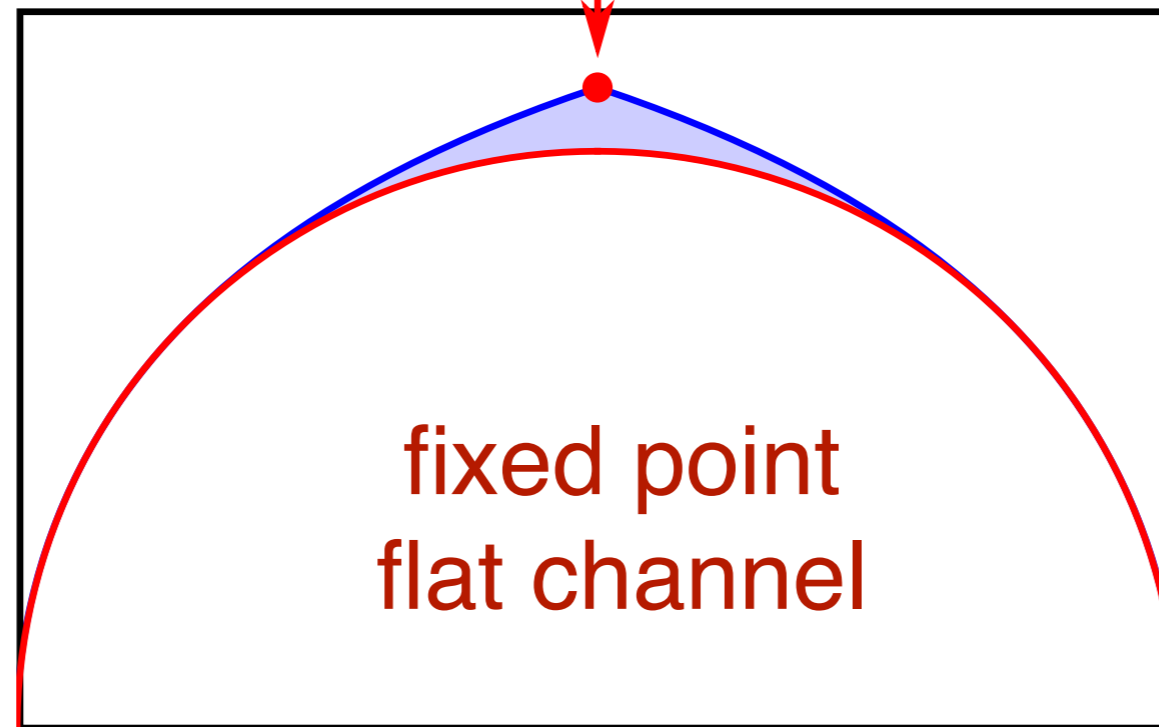
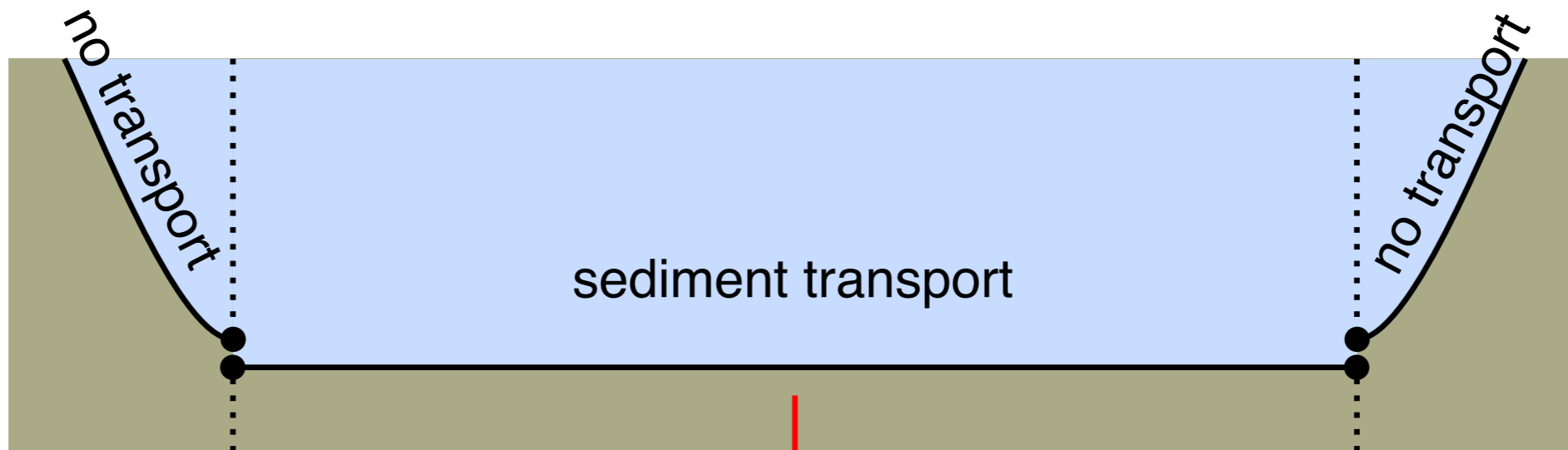


Experiments \rightarrow $Pe = \frac{L}{\ell_d} \frac{1}{S} \sim \frac{d_s}{10^{-2} d_s} \frac{1}{10^{-2}} \sim 10^4$

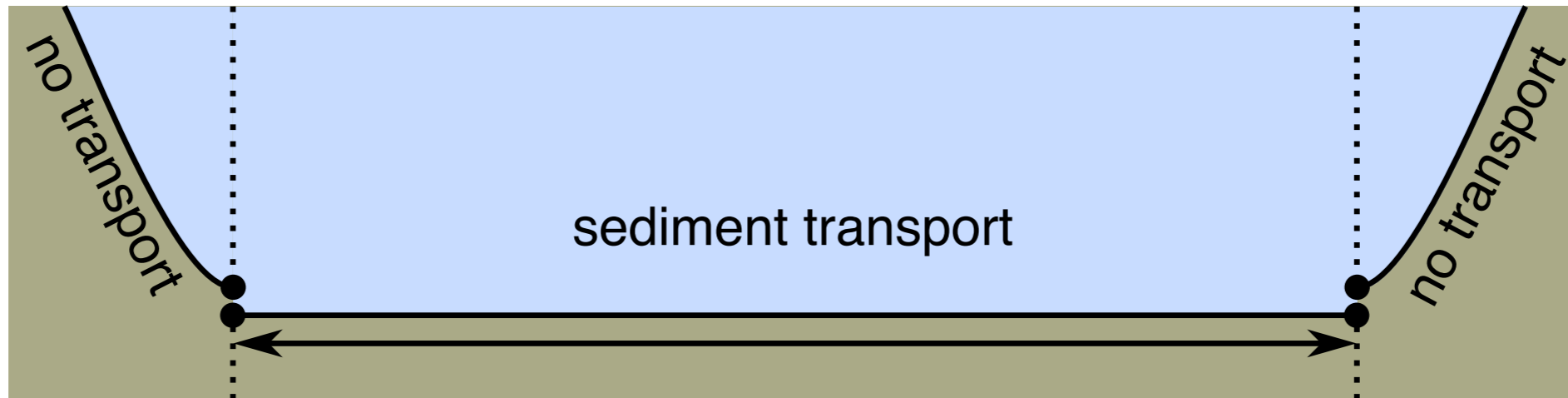
'flat river' approximation



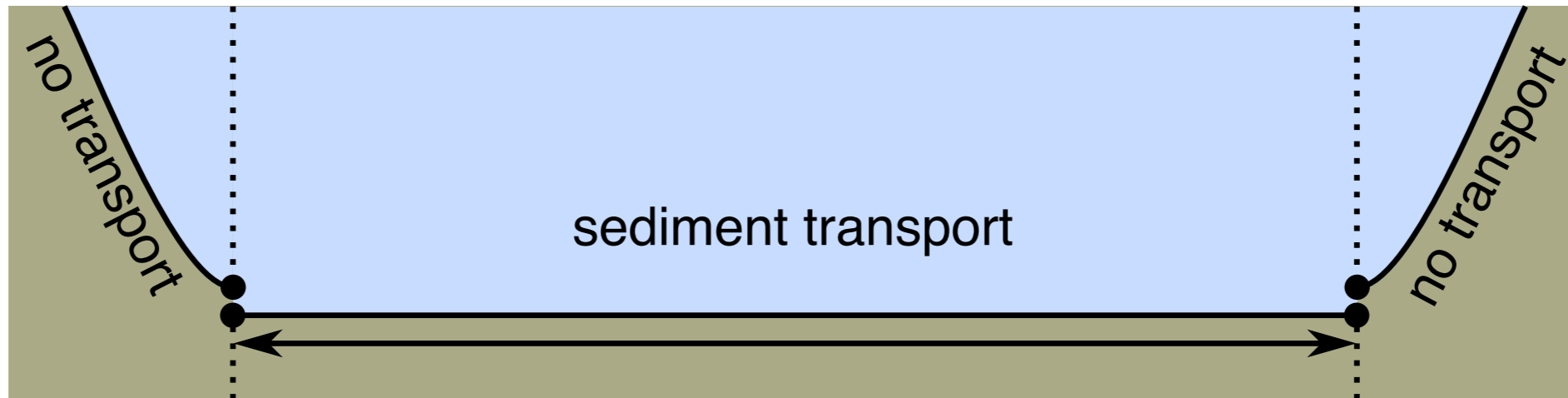
'flat river' approximation



'flat river' approximation'



'flat river' approximation'



For a laminar flow :

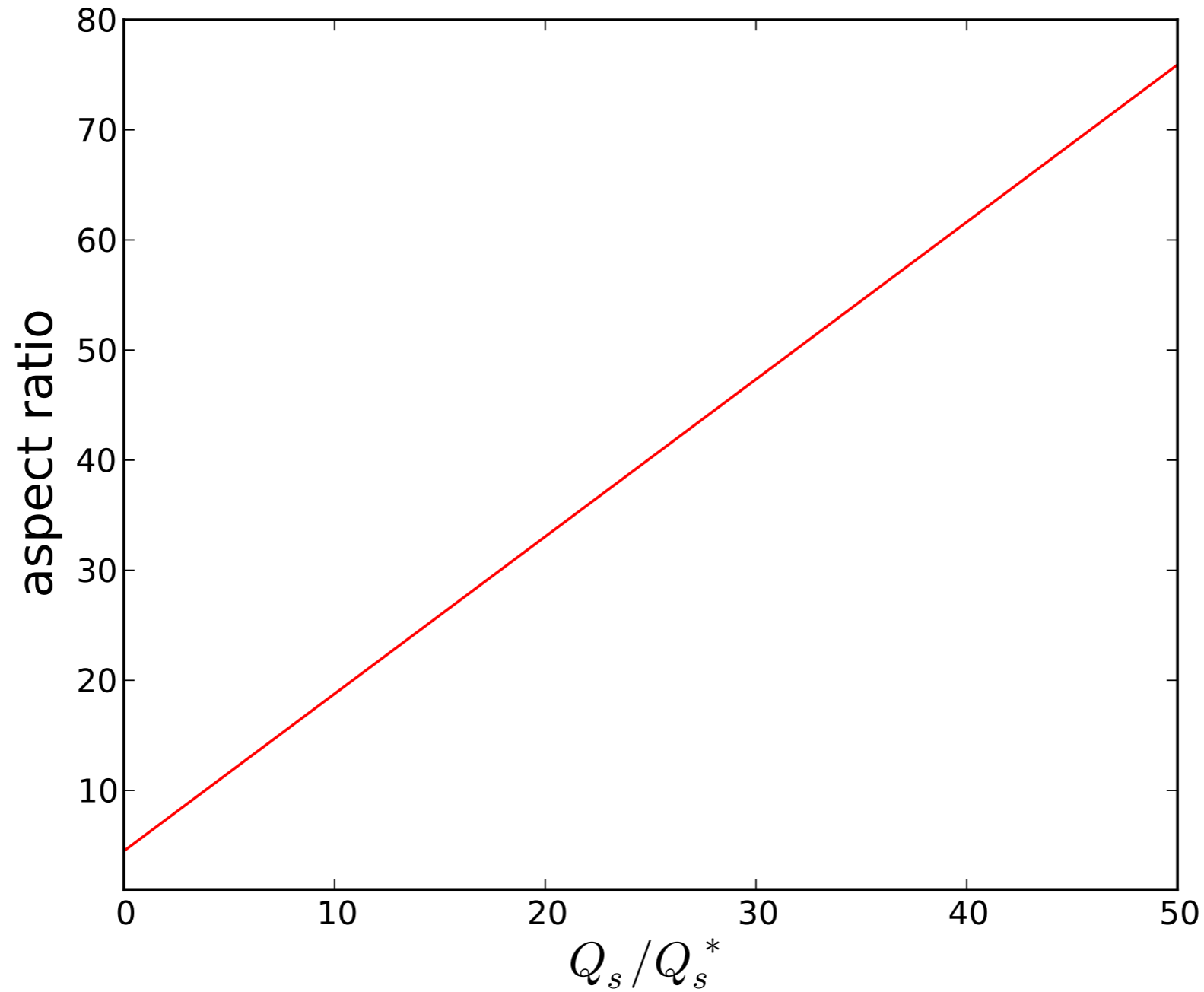
$$S = \left(\frac{4g\mu^3 L}{9\nu} \right)^{1/3} L Q_w^{-1/3} \left(1 + \frac{3}{4} \frac{Q_b}{Q_b^*} \right)^{1/3}$$

$$W = \pi \left(\frac{4g\mu^3 L}{9\nu} \right)^{-1/3} Q_w^{1/3} \left(1 + \frac{1}{\pi} \frac{Q_b}{Q_b^*} \right)^{1/3} \left(1 + \frac{3}{4} \frac{Q_b}{Q_b^*} \right)^{-1/3}$$

$$\frac{W}{D_0} = \frac{\pi}{\mu} \left(1 + \frac{1}{\pi} \frac{Q_b}{Q_b^*} \right)$$

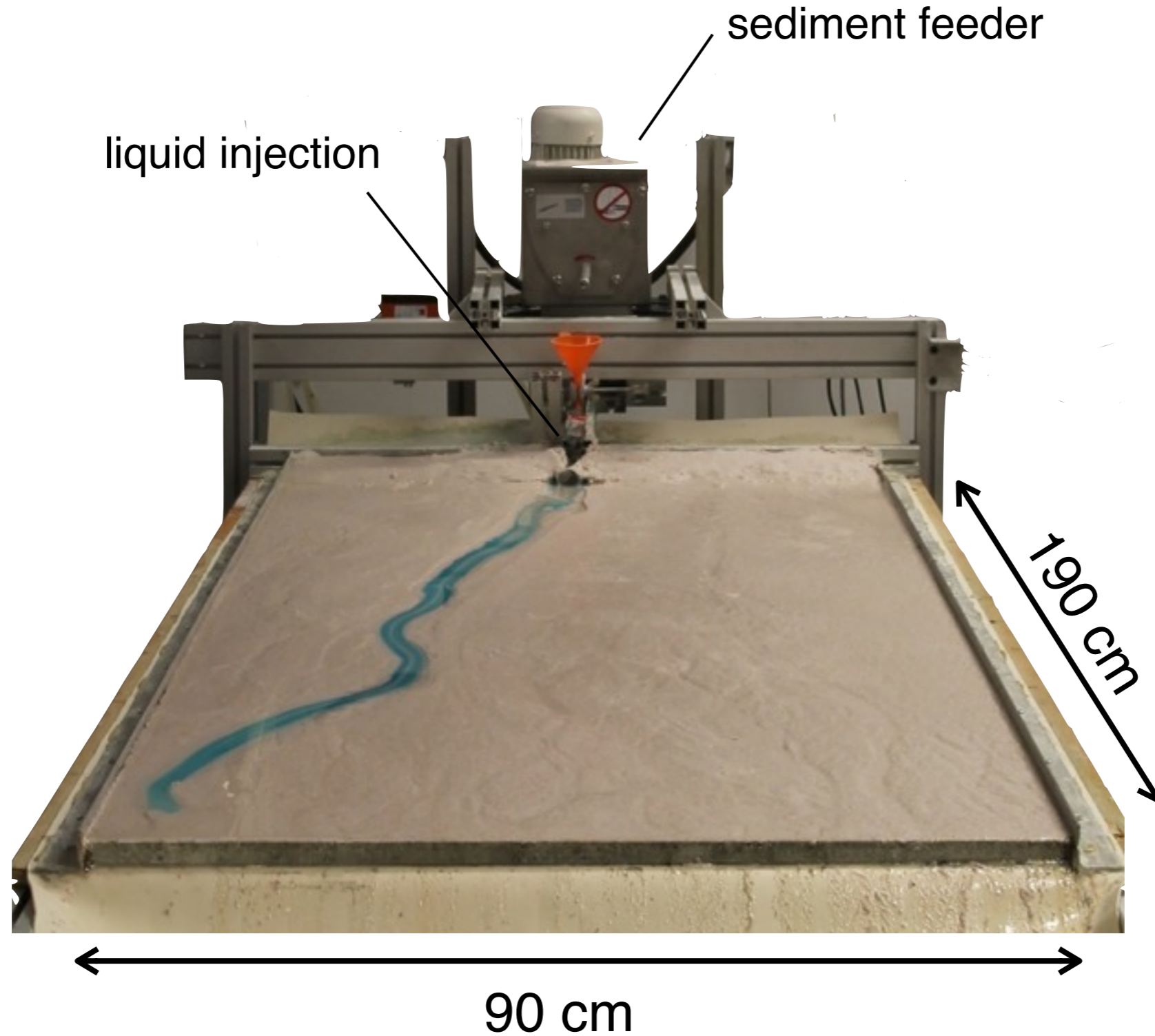
$$Q_b^* = \beta \frac{V}{d_s^2} \mu \ell_d$$

'flat river' approximation



$$\frac{W}{D_0} = \frac{\pi}{\mu} \left(1 + \frac{1}{\pi} \frac{Q_b}{Q_b^*} \right)$$

Laboratory rivers



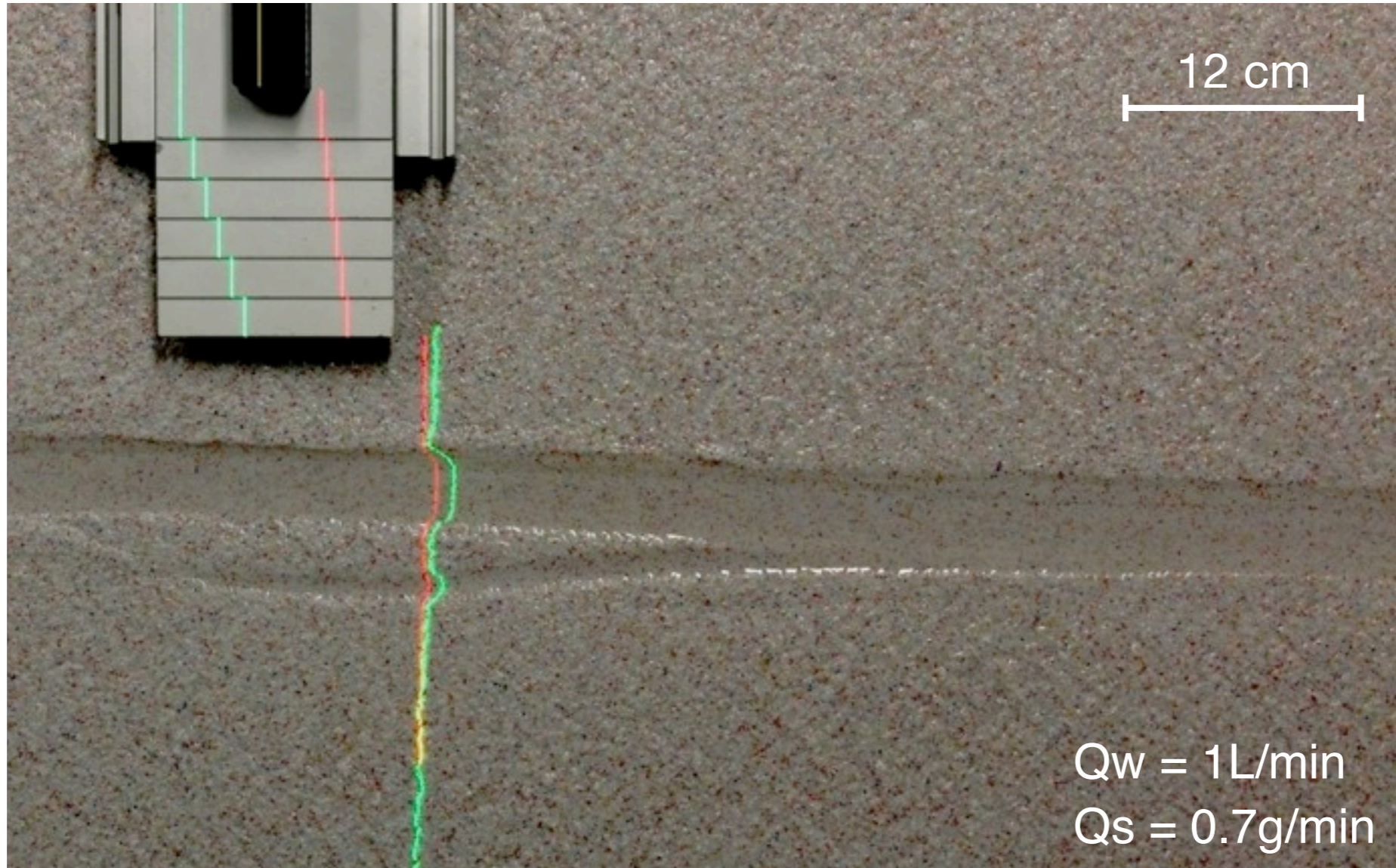
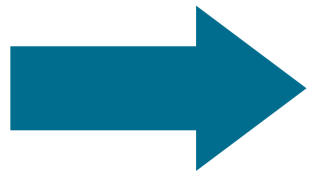
- **constant discharge**
- **constant input sediment rate**



steady-state

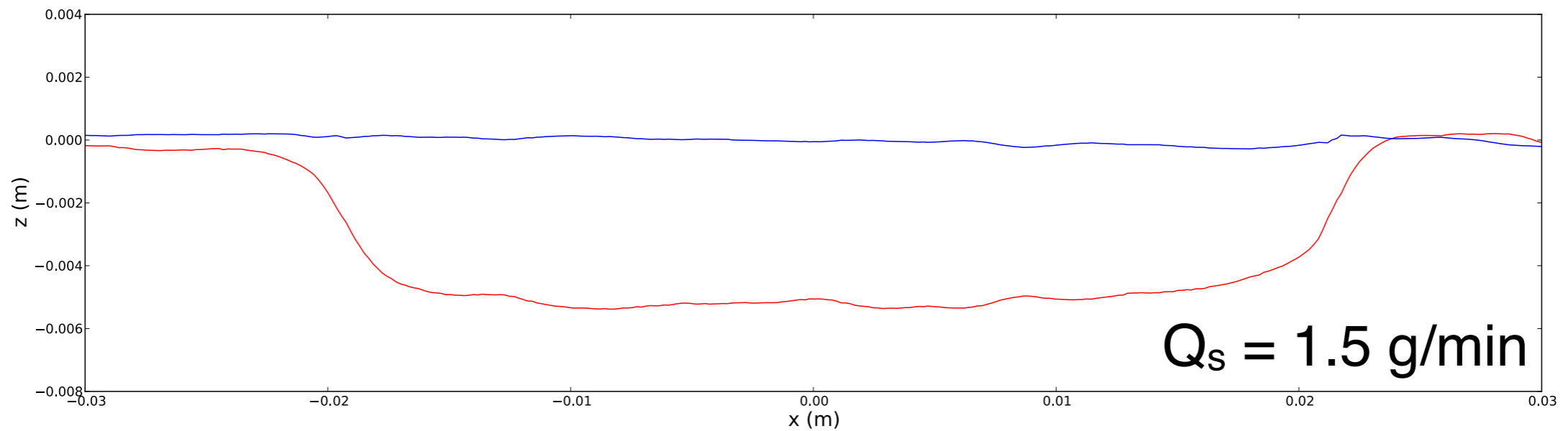
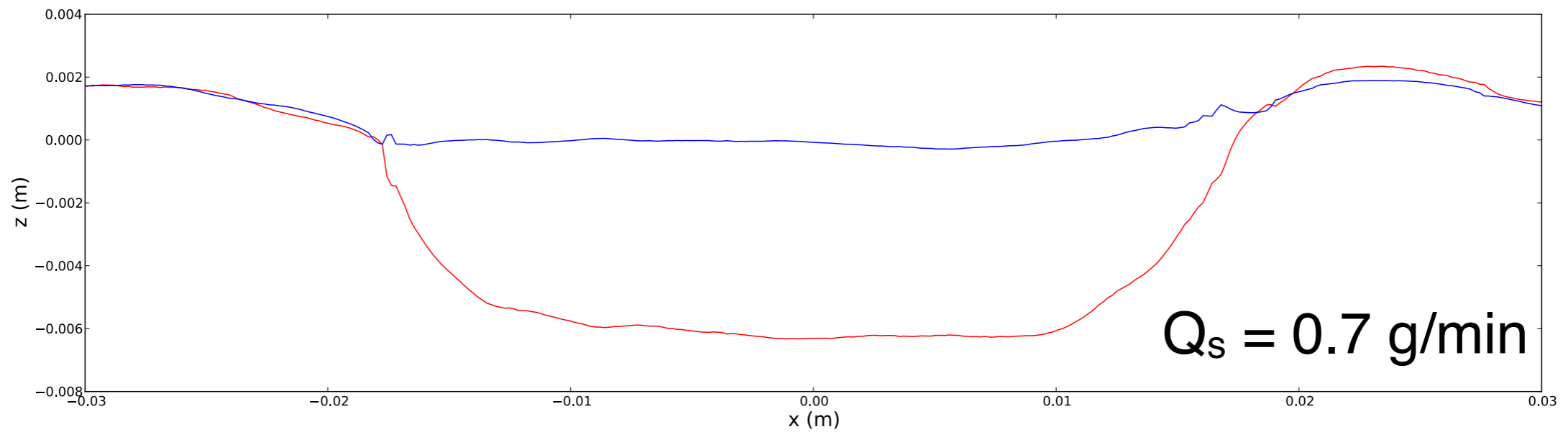
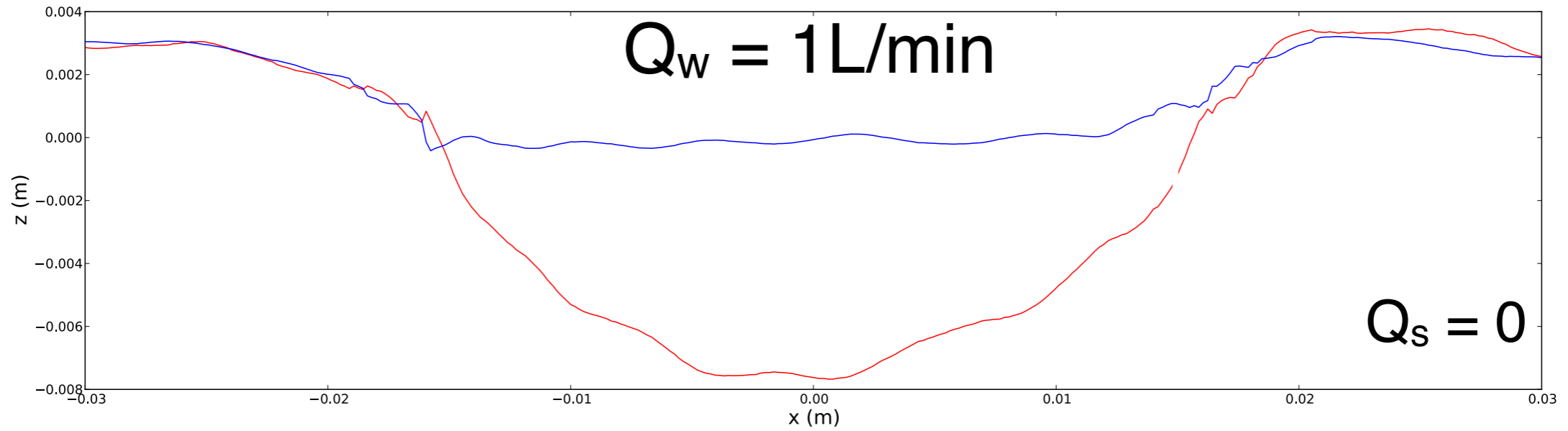
Experiments

flow

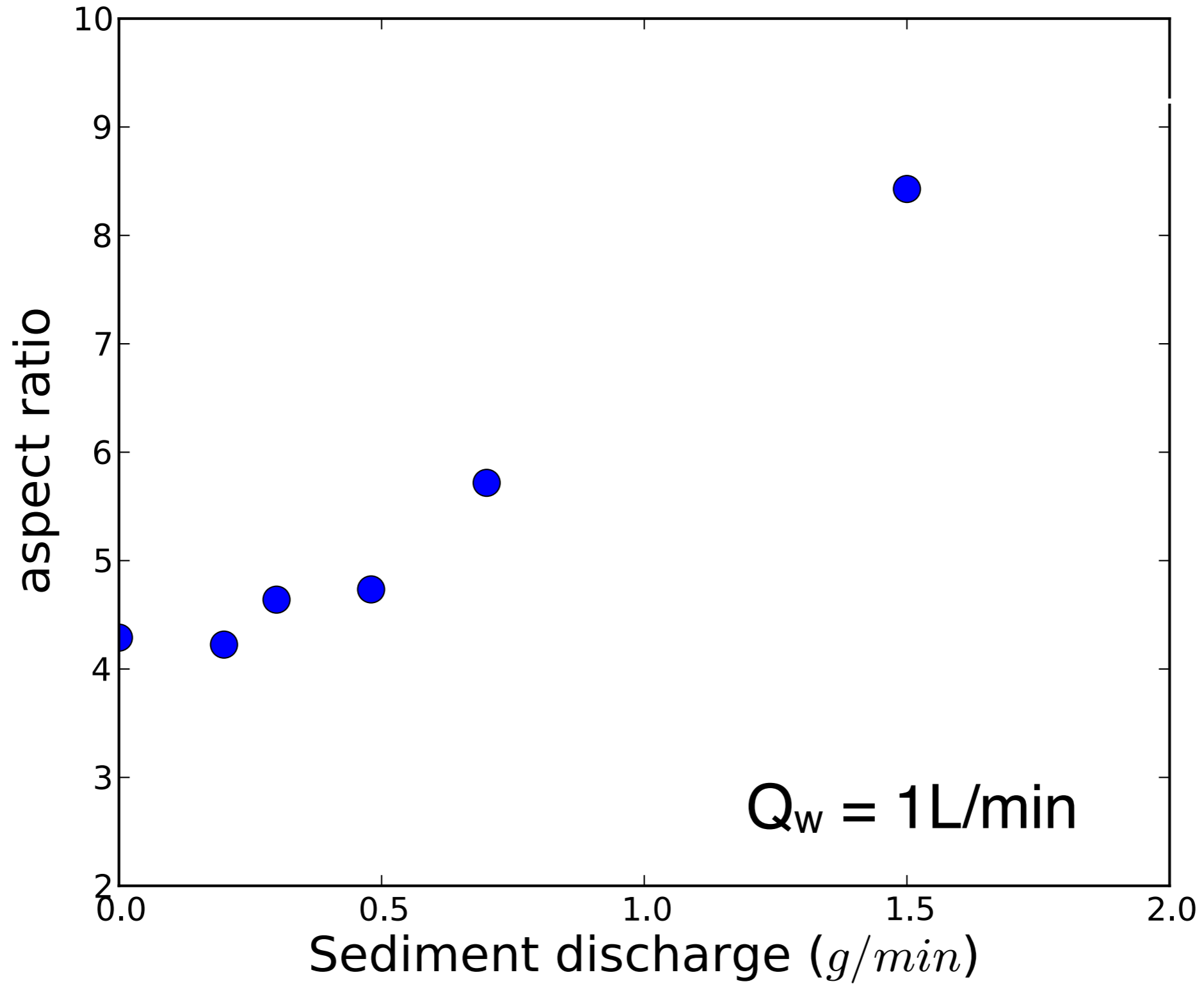


$Q_w = 1 \text{ L/min}$
 $Q_s = 0.7 \text{ g/min}$

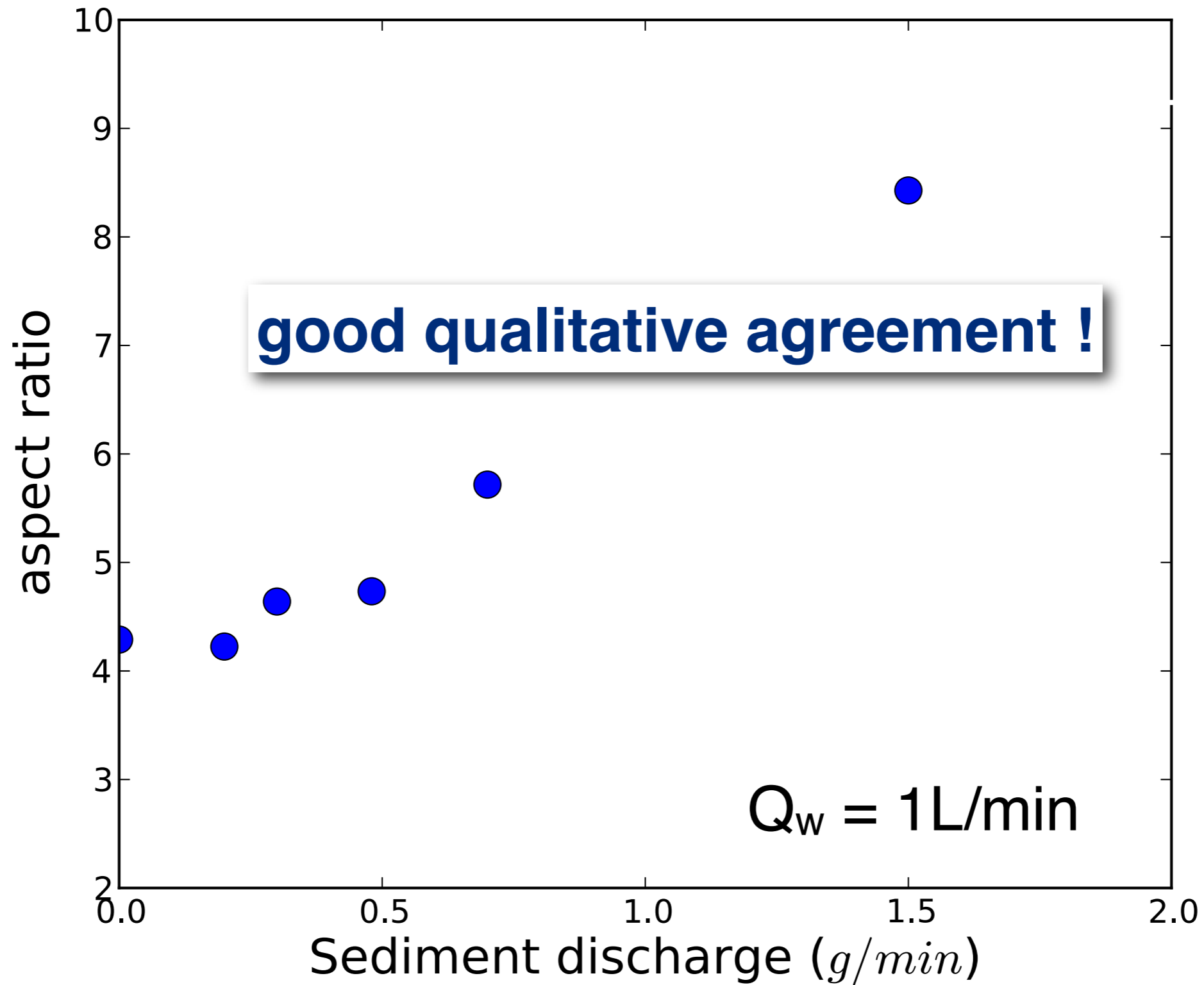
Experiments



Experiments

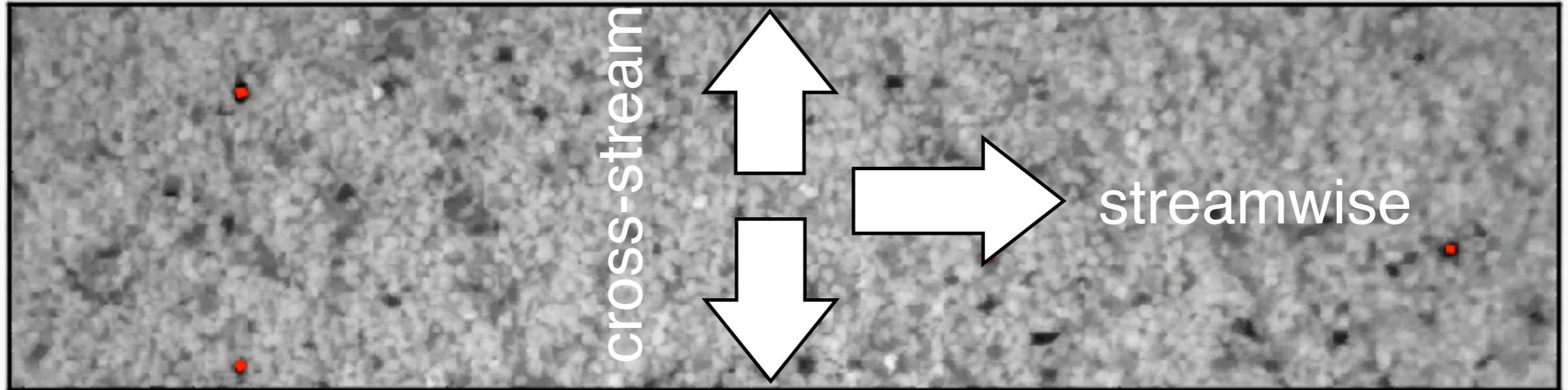


Experiments



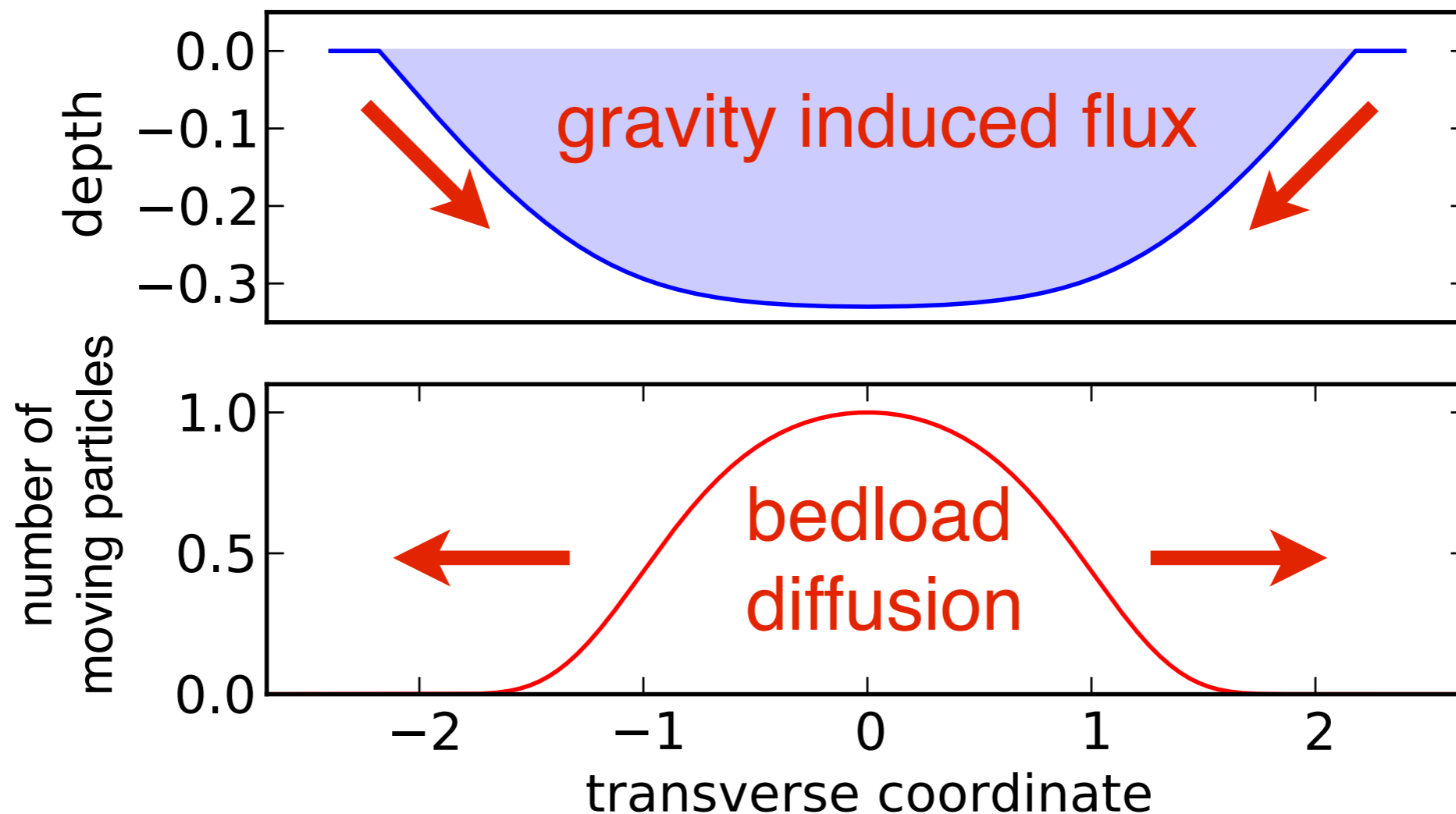
Summary

- **Bedload transport generates transverse diffusion.**



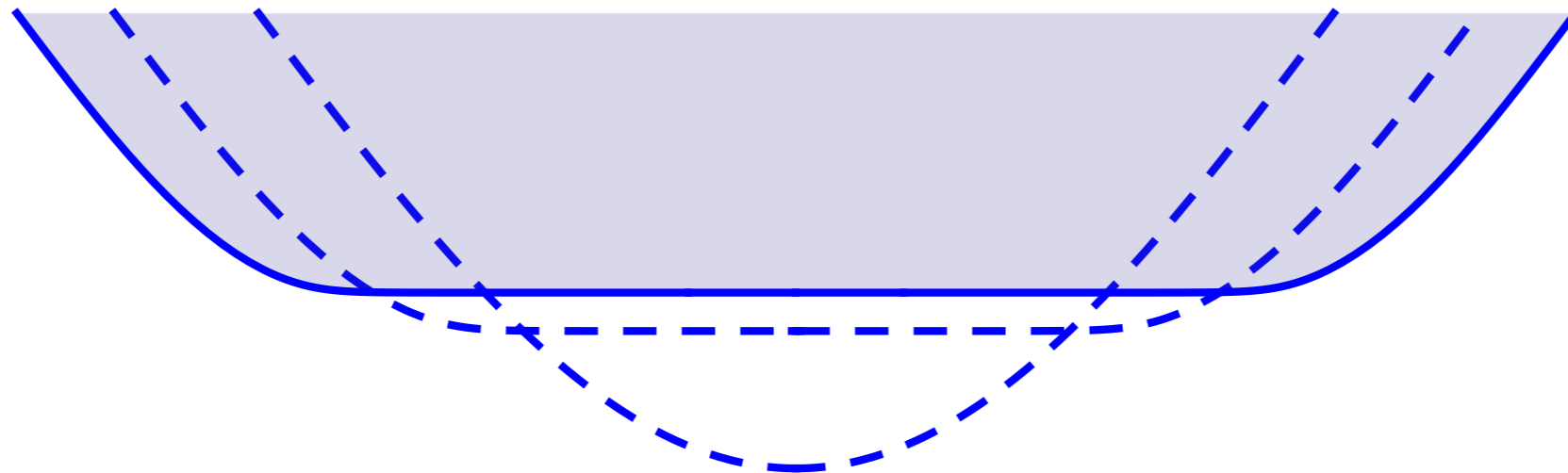
Summary

- **Bedload transport generates transverse diffusion.**
- **Equilibrium shape of rivers is selected by the balance between bedload diffusion and gravity induced flux.**



Summary

- **Bedload transport generates cross-stream diffusion.**
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- **The width to depth ratio of the river increases with the sediment discharge.**



Summary

- **Bedload transport generates cross-stream diffusion.**
- **Equilibrium shape of rivers is selected by the balance between bedload diffusion and gravity induced flux.**
- **The width to depth ratio of the river increases with the sediment discharge.**
- **Experimental (in)validation in progress !**



**How does this physical framework
compare to field data ?**

Field data

single thread gravel-bed rivers

Data compiled by Métivier & Barrier [2011]

