

From Plasticity to a Renormalisation Group

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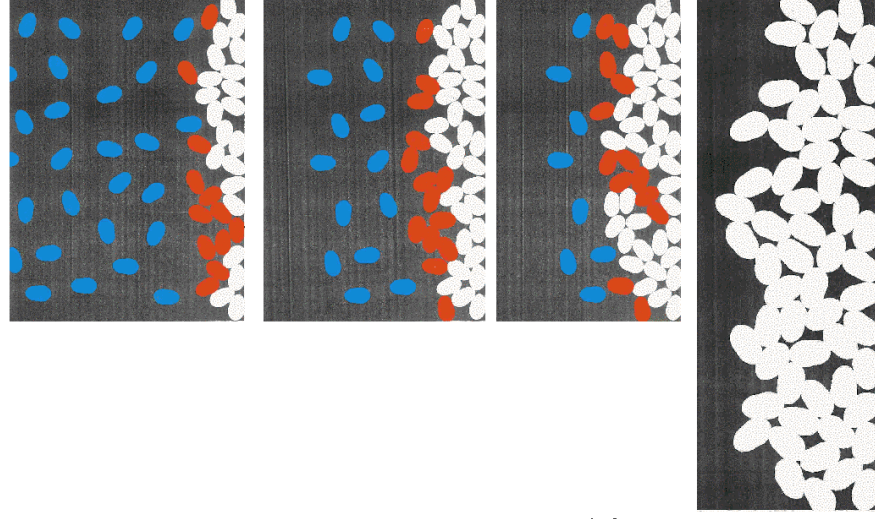
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C Thornton	Aston
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Outline of talk

- Marginally Rigid State
 - Experimental Evidence
 - Stress Transmission
- *ab initio vs FPA*
 - RG & generalised statics
- Yield Equations & Grain Rolling
- Rough vs Smooth grain response



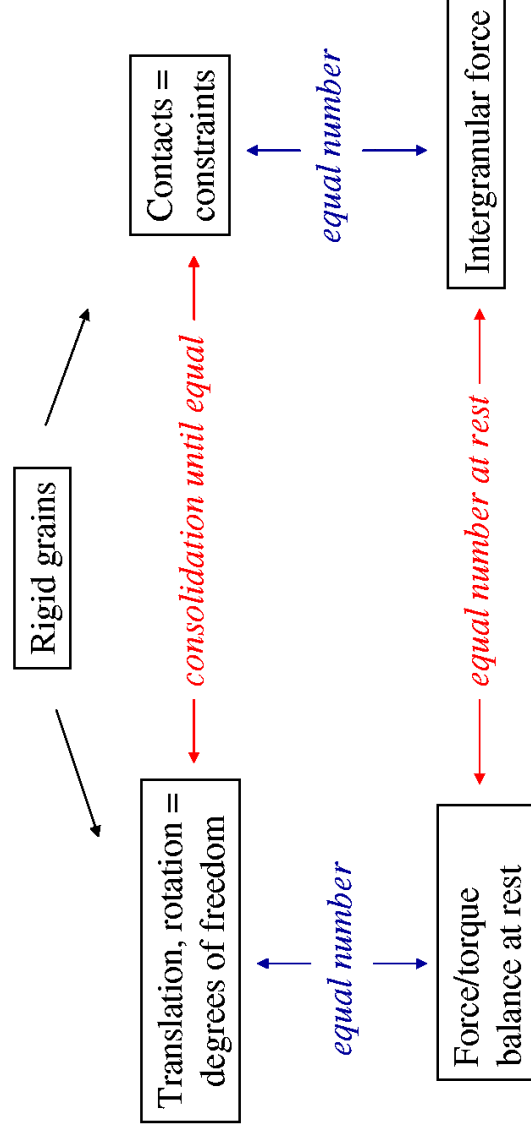
Stress in Granular Materials



- Soil Mechanics
design of foundations
dams, embankments
landslides
- Materials handling,
silos
- Counterintuitive Physics
load distribution
under pile



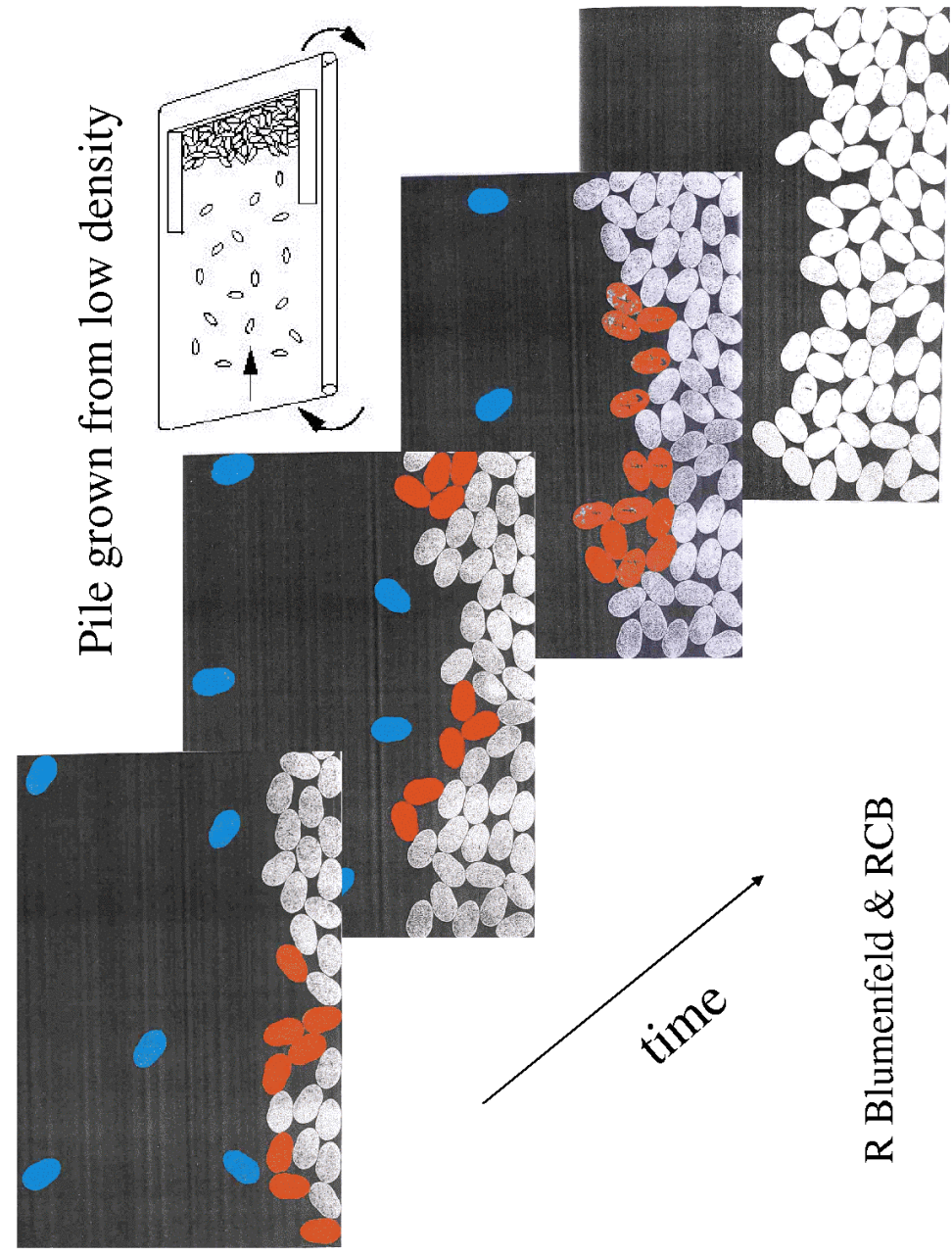
Marginally Rigid State



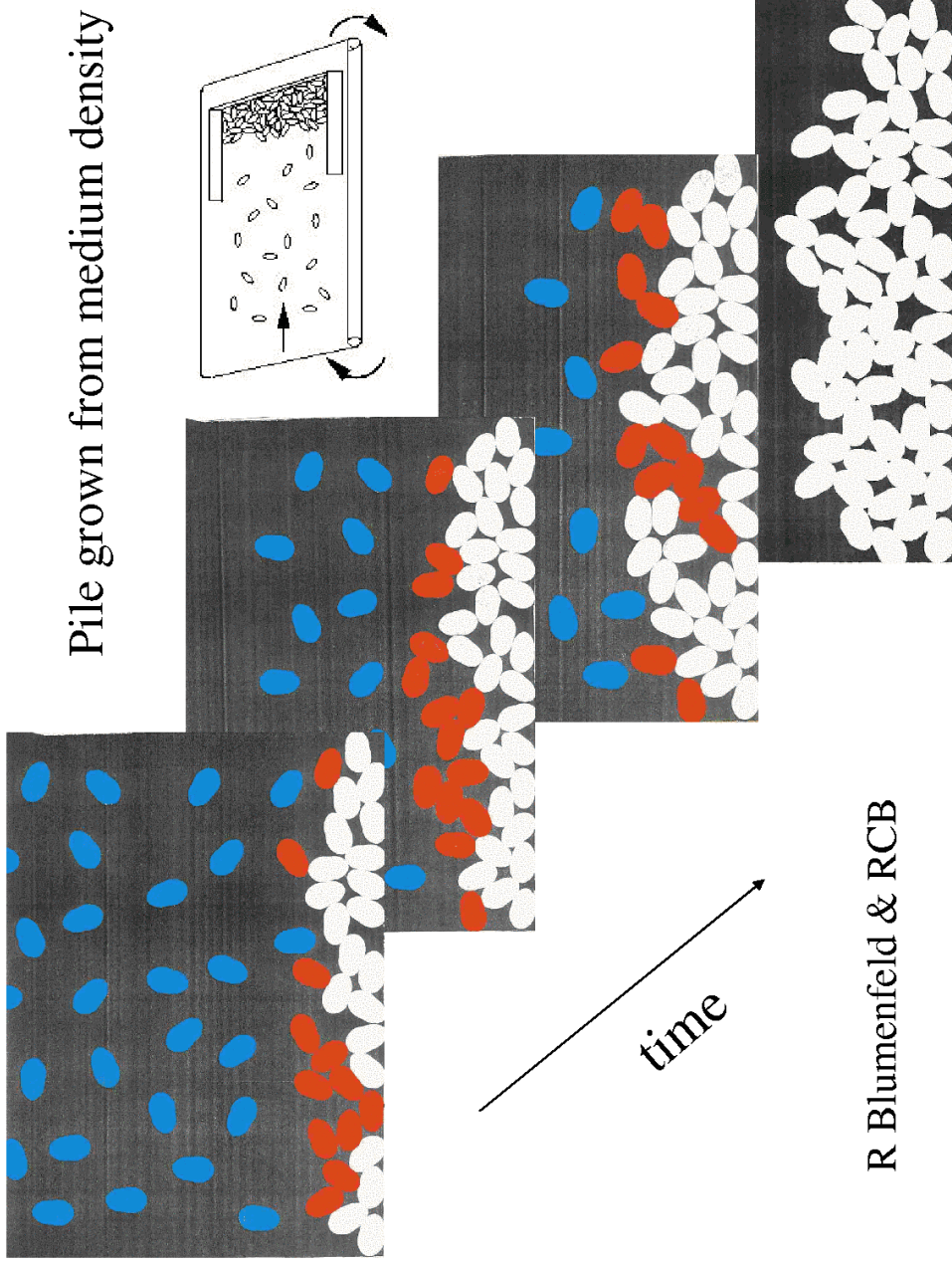
- Static equilibrium determines all the intergranular forces
and hence the transmission of stress

Test of Marginal Rigidity: Critical Coordination Number

$d=2$	per grain per contact	3 d.o.f 2 constraints, with friction	$\Rightarrow z_c = 3$
			<i>checked by experiment</i>
	no friction " & discs	$z_c = 6$ $z_c = 4$	<i>topological maximum</i> <i>both match sequential (disordered) packing</i>
	friction	$z_c = 4$	
$d=3$	no friction " & discs	$z_c = 12$ $z_c = 6$	<i>hard to exceed (but possible: Donev et al)</i> <i>both match sequential (disordered) packing</i>

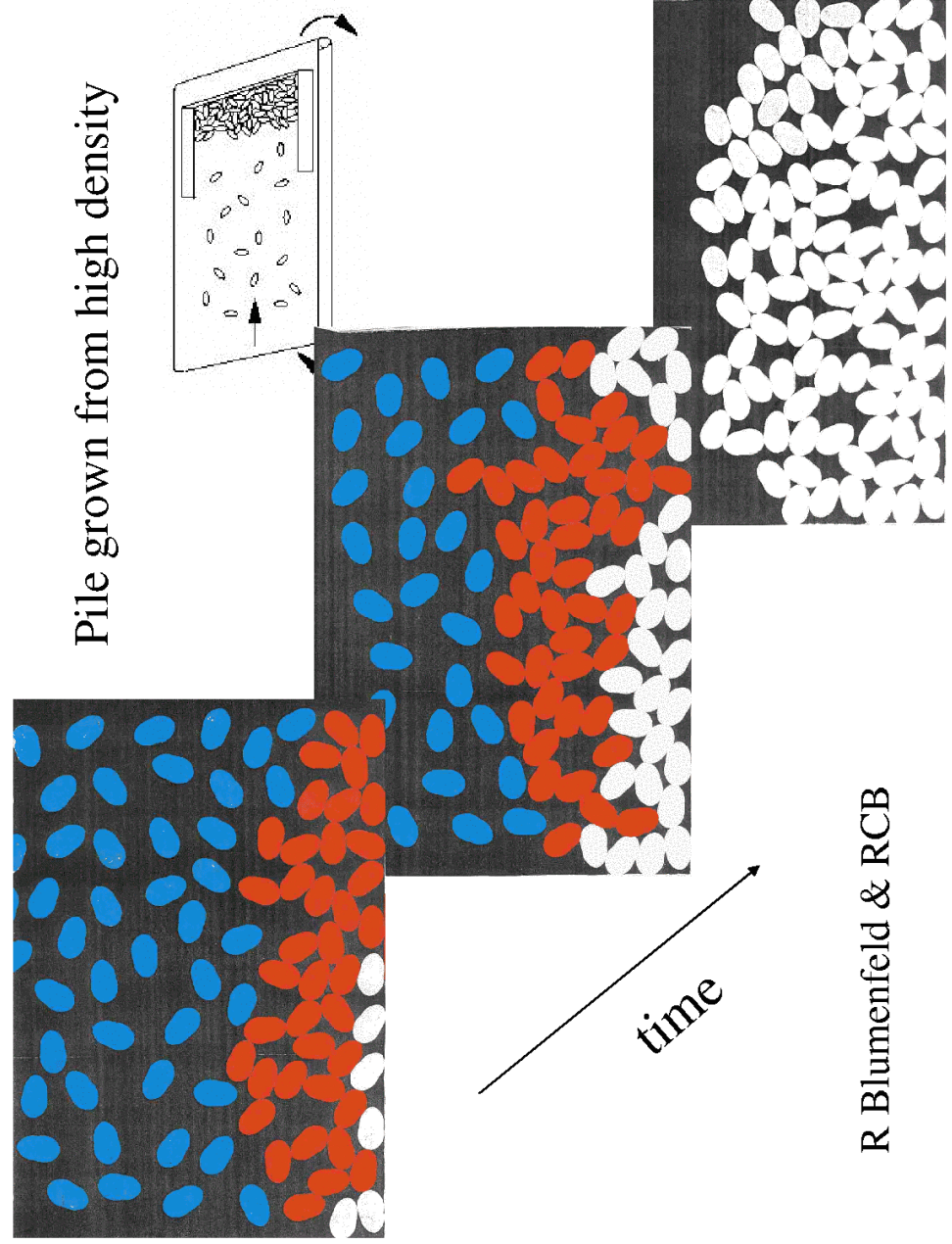


Pile grown from medium density



R Blumenfeld & RCB

Pile grown from high density



R Blumenfeld & RCB

Final density
vs initial density

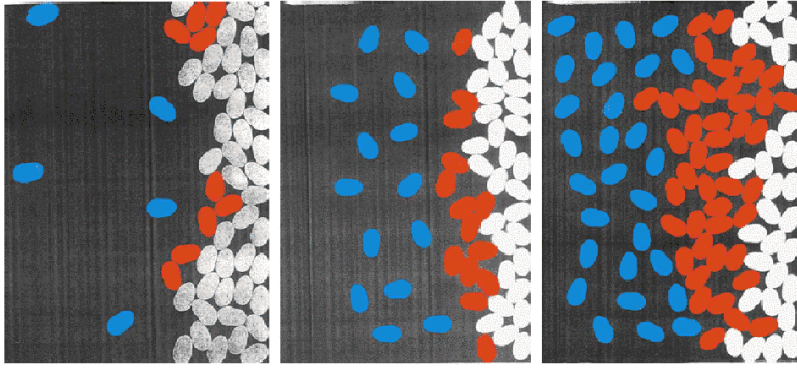
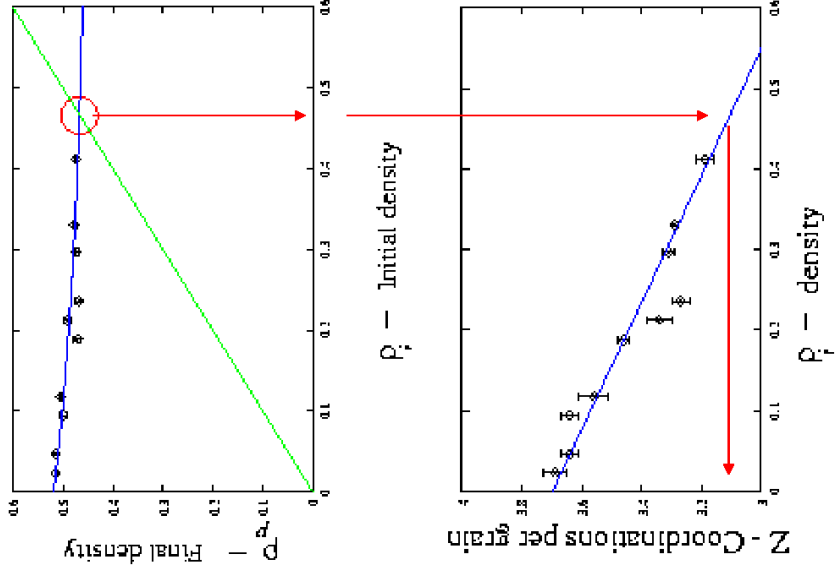
limiting value $\rho_c = 0.47$

Experimental
results

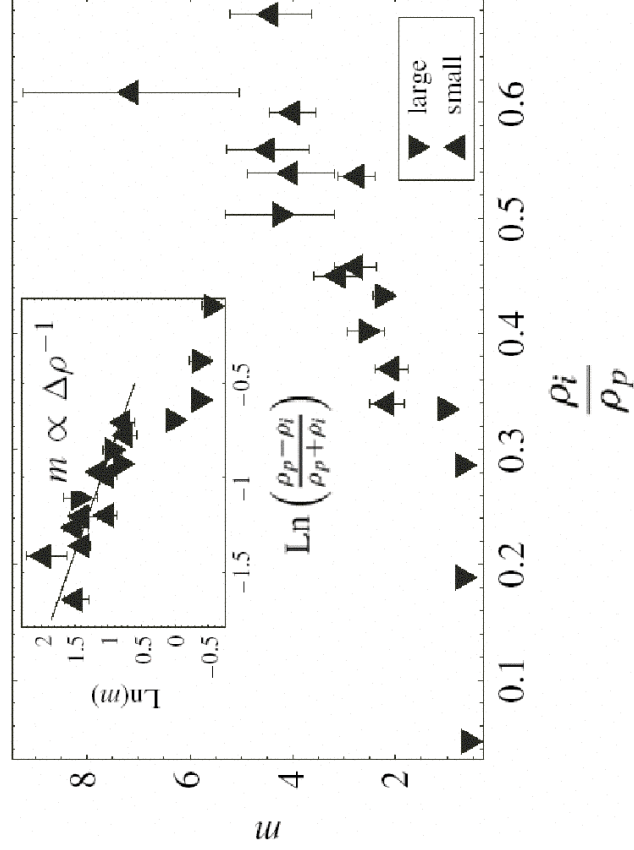
Coordination number

limiting value $z_c = 3.1$

RB & RB



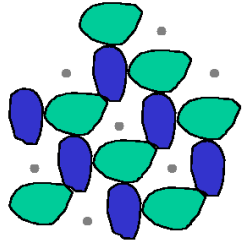
The Consolidation Front



Stress transmission calculations

$$P_{ijkl} \sigma_{kl} = 0 \quad (*)$$

antisymmetric \swarrow \searrow symmetric indices



Simplest periodic lattice at z_c :
 2 grains per unit cell \Rightarrow
 2 conditions balancing torque:

$$\sigma_{kl} = \sigma_{lk} \quad \text{and } (*) \text{ above}$$

$$P_{ijkl} = \epsilon_{ij} \epsilon_{kp} \sum_{\text{round grain}} (r_p R_q + R_p r_q) \epsilon_{ql}$$

$d = 2$

- Generalises off-lattice
- **Spatial average is zero**
- Anti-correlation between neighbours

Local linear constitutive equations

$$P_{ijkl} \sigma_{kl} = 0$$

“OSL” - *Cates et al* “Isostatic” “Null-stress”

FPA: $p_{ijkk} = 0$ for all ij

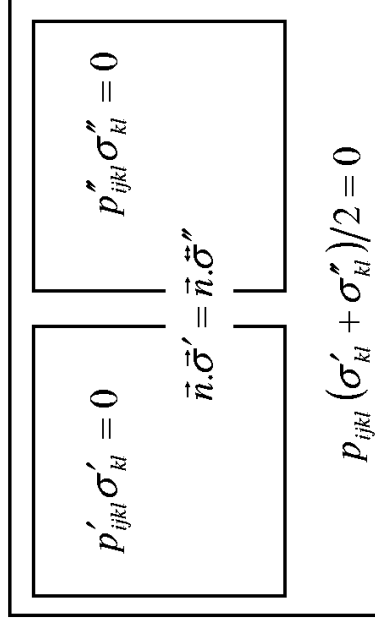
$$\vec{\sigma} = \sigma_{aa} \vec{a}\vec{a} + \sigma_{bb} \vec{b}\vec{b} + \sigma_{cc} \vec{c}\vec{c}, \quad \vec{a}, \vec{b}, \vec{c} \text{ set by sample history}$$

d=2: $P_{kl} \sigma_{kl} = 0$ degenerate case
Cates et al, Edwards & Grinev

$$\sigma_{ij} = f_1 \tau_{1ij} + f_2 \tau_{2ij} \quad \text{Tkachenko \& Witten}$$

Renormalisation of Static Equations

Simple model



•simple solution in $d=2$: $p_{ij} = B \left(p'^{-1}_{ij} p'_{ij} + p''^{-1}_{ij} p''_{ij} \right)$ $t \Rightarrow$ transverse to n

(left, antisymmetric indices are redundant in $d=2$)

•generalises to all d but B , p_t^{-1} not scalar; right indices of p' , p'' preserved.

Fixed Points

1. *Constant Fixed Points*: $p = \text{any constant}$
 - Sensible for smooth grains.
 - Significance unclear for rough grains: average(p) is zero at grain level, but RG transformation not linear.
2. *Pressureless fixed points*
 - Where (one of) the p_{ij} , constrains the stress to be traceless, clearly transitive under the stress combination.
 - Linearly stable to fluctuations in $d=2$: analytic calculations seen numerically by R Farr
 - Pressureless stress incompatible with cohesionless granular matter.
3. *Fixed Principle Axes fixed points*
 - All p traceless wrt rh indices, i.e. $p_{ijkk} = 0$ for all i, j ; clearly transitive under renormalisation of p
 - Random axes fixed point neutrally stable wrt small isotropic parts
analytic calculations in $d=2$
numerical indications in $d=3$

Higher derivative cases

$$p_{ijkl} \sigma_{kl} + q_{ijklm} \partial_m \sigma_{kl} + r_{ijklmn} \partial_m \partial_n \sigma_{kl} + \dots = 0$$

Rough Grains :

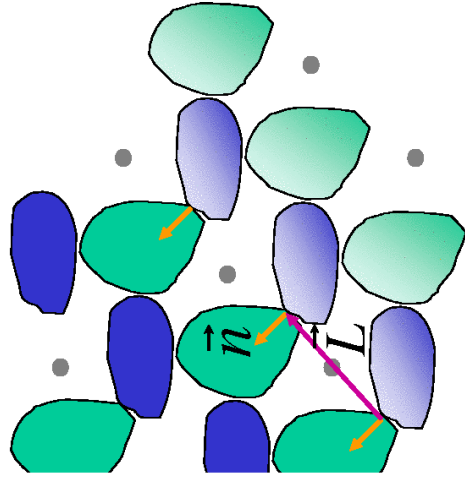
periodic, aspherical: $p \neq 0$ (but staggered)

periodic, spherical: $p = 0, q \neq 0$

statistically isotropic: $p = 0, q = 0, r \neq 0$

Smooth Grains :

periodic, aspherical: $p \neq 0$ (no stagger).



Yield Equations

Condition for sliding at plane:

$$|\vec{n} \times \vec{\sigma} \times \vec{L}| > \mu (-\vec{n} \cdot \vec{\sigma} \times \vec{L})$$

shear force

coeff friction

normal force

$$Y(\vec{\sigma}) = \prod_{\text{sliding planes}} (|\vec{n} \times \vec{\sigma} \times \vec{L}| + \mu \vec{n} \cdot \vec{\sigma} \times \vec{L}) > 0 \quad \text{yielding}$$

$$Y(\vec{\sigma}) = 0 \quad \text{creeping yield} \Rightarrow \text{plastic flow, direction} = g(\vec{\sigma})$$

$$Y(\vec{\sigma}) < 0 \quad \text{unyielding}$$

Plastic Flow

conventional plasticity grain rolling

$$\partial_k u_l + \partial_l u_k = A(\vec{x}, t) g_{kl}(\vec{\sigma}) + p^T_{kl ij}(\vec{x}) \omega_{ij}(\vec{x}, t)$$

local sliding rate new (?) coefficients grain angular velocity

•supplemented by:

plastic yield condition $Y(\vec{\sigma}) = 0$

force balance $\vec{\nabla} \cdot \vec{\sigma} + \vec{f} = 0$

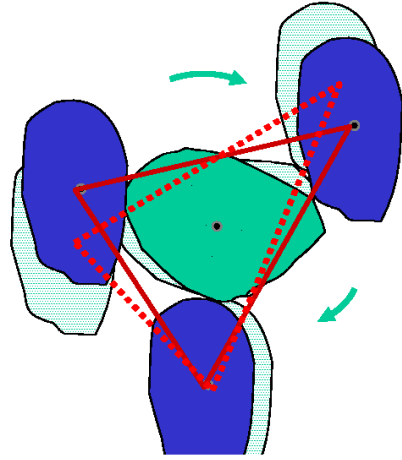
'constitutive equation' $P_{ij kl} \sigma_{kl} = 0$

•grain rolling is required for the equations to close correctly

•Galilean invariance requires average of $p^T = 0$

A
 \vec{u}
 ω_{ij}

Grain Rolling



Rotate central grain & translate neighbours, maintaining rolling contact

Explicit calculation of shear rate in triangle:

$$\partial_k u_l + \partial_l u_k = \frac{1}{\text{area}} \int (u_k dS_l + u_l dS_k) = \omega_{ij} p_{ij kl}$$

i.e. p^T really is the transpose of p , (for $d=2$, periodic lattice)

Dissipation rate & Interpretation

$$2\dot{\Delta} = (\partial_k u_l + \partial_l u_k) \sigma_{kl} = A g_{kl} (\vec{\sigma}) \sigma_{kl} + \omega_{ij} p_{ij} \sigma_{kl}$$

↑ sliding
↑ rolling

• The force conjugate to rolling is $p_{ij} \sigma_{kl}$

where p can quite generally be defined from the plastic flow equation.

• Rolling is not (directly) dissipative $\Rightarrow p_{ij} \sigma_{kl} = 0$

hence this force must be balanced - explaining the 'constitutive eqn'.

Cf Tkachenko & Witten PRE 62 2510 (2000)

Microscopic views of Rolling/Sliding

What happens when we load a sample, incompatibly with $p_{ijkl} \sigma_{kl} = 0$?

Change in geometry \rightarrow local changes in p

- \rightarrow changes in σ which could be much less local
- what do these look like?

SUGGESTION:

Smooth grains: imposed deformations penetrate bulk, so the bulk deforms

Rough grains: any strain disproportionately concentrated at walls

\rightarrow wall rearrangement

\rightarrow loads transmitted to bulk are compatible with $p_{ijkl} \sigma_{kl} = 0$

Simple Cases

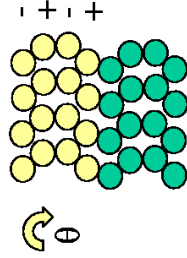
Smooth grains



Rough grains



Grain rotation \rightarrow overshoot of affine at walls



Local shear $\gamma \sim a \nabla \theta$
 wall shear $\gamma_{\text{wall}} \sim \theta$

Wall shears \gg bulk shears for rough grains
 \rightarrow rearrangement preferentially at walls

Summary and Outlook

- Direct experimental evidence for marginally rigid state
 - $d=2$ yes, $d=3$ outstanding
- Theory of stress transmission ... empirical theory
 - non-lattice calculations required in $d=3$
- RG indicates FPA may be stable
- RG in $d=3$; systematic/improvements ?
- Gradient terms and order of equations ?
- New hydrodynamics: Yield = plasticity + rolling
 - interpretation of dilatancy: shear \Rightarrow roll \Rightarrow dilate
- “missing” static equations relate to Rolling Force
- Closure: evolution of P_{ijkl} under shear ?
 - are rough and smooth systems fundamentally different ?
- Impact engineering soil mechanics?

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